TETRA TECH

January 10, 2023

Mr. Brock Bowles
Division of Reclamation, Mining and Safety
1313 Sherman Street, Room 215
Denver, CO 80203

RE: Adequacy Questions 2, DRMS AM-4 application – Home Office Mine, Permit M1977-439

Dear Mr. Bowles:

This letter is in response to your Adequacy Questions 2 letter, received via electronic copy on December 9, 2022 regarding Martin Marietta's 112c Permit Amendment Application (AM4) for the Home Office Mine (File No. M-1977-439) that was submitted to you on November 18, 2022. Please find below our responses to the Adequacy Questions 2:

6.4.7 EXHIBIT G – Water Information

39. The adequacy response stated that "mining and reclamation are completed in accordance with mining and reclamation plans in effect at the time of mining" but did not address how each pond will safely convey the expected 100-year flood event throughout the life of the mine including final reclamation.

    The analysis should quantify the velocity and volume of flows expected, the elevation of the event, and its relation to the elevation of any proposed spillways and reservoir embankments. The flood control plan should propose mitigation measures such as inflow and outflow channels and/or other appropriate measures. If specific measures are not known at this time, please commit to submitting them later in a technical revision.

    Response
    Martin Marietta agrees to evaluate the previously mined ponds and will submit a technical revision, if necessary.

40. The following questions are for the water information submitted with the AM4 application and refer to Stage G:

    a. Commitment to monthly water level monitoring of all wells the applicant has access to, along with a map of well locations to be monitored and a concise presentation of historic data for those wells with explanations of observed impacts to those wells shown in historic data (i.e. pumping unlined pits, installation of clay liners, underdrains, etc.) Monthly monitoring data should be presented quarterly to DRMS with historic trends shown for at least the last 12 months for each well.

    Response
    Available historic water level data has been previously submitted in Exhibit G. Moving forward, Martin Marietta will submit ground water level monitoring reports on a quarterly basis. The reports will include a map of showing the wells that Martin Marietta has legal access to along with information about the water levels measured in those wells. Where available, the data reports will include the historic water level measurements. The data will be provided in tabular format and as hydrographs of water level elevation vs time. We have updated the hydrographs in Exhibit G (see Attachment A to this letter) to include the most current water level data.

    b. Discussion of why the two wells shown in Exhibit G downgradient of phases G1 and G2 (identified as 246541, and 49917-F) are not likely to be impacted by shadowing from lined reservoirs.

Response
These well have been operated throughout the historic mining process without complaint from the well owners. This is because they are near the Cache la Poudre River and the river largely controls the water levels in the wells. The installation of the pit liners will have less effect on water levels than the dewatering. Consequently, these wells will not be impacted by the installation of the liners.

c. Presentation of historic groundwater level data in the form of GW contour maps to show, where possible, historic GW flow directions and levels/depth below ground surface, along with impacts to the historic patterns due to mining and reclamation. These should place an emphasis on current data to demonstrate minimization of mounding and shadowing impacts.

Response
Tetra Tech is preparing water level contour maps and hydrographs (Attachment A) for the area adjacent to the Phase G pits to characterize the general conditions for: pre-mining, dewatered, and the lined reservoir. The water level contour maps will be submitted to the DRMS as soon as practical.

d. Section 2.3 Mitigation – of the provided Exhibit G states that "in the event of a well owner complaint within 600' of the affected area" MM will submit a report to DRMS within 30 days. DRMS does not restrict the radius of impact to 600' and will require MM to commit to reporting any complaints by well owners to DRMS within 48 hrs or less. MM will be required to initiate an investigation into the complaint immediately and submit the results to DRMS for evaluation within 30 days.

Response
Martin Marietta will commit to reporting to the DRMS any complaints received from well owners within 48 hours, to investigating the complaint as soon as practical, and to submitting the results to the DRMS for evaluation within 30 days.

For the investigation, the first level of response will be to review water level data from the monitoring well network and, if available, a measurement of the water level in the plaintiff's well. The information will be evaluated to determine if there is a reason to believe the plaintiff's complaint may be tied to the lined reservoirs. If the data indicates that there is no reason to believe the plaintiff's well was impacted by the lined reservoirs, that will conclude the action taken by Martin Marietta. If the data does not clearly show there is no impact, as a second level of response, Martin Marietta will present a contract to the well owner that requests access to the well to perform a mechanical and electrical inspection and testing of the well and associated system, e.g. pressure tank. The agreement will explain that if the problem with the well is not due to a lower water level and is instead due to a mechanical or electrical issue, the well owner will be responsible for the repairs. If the well is determined to be in good working order and the problem is due to a lower water level, then the mitigation steps outlined in the previously submitted Groundwater Mitigation Plan will be implemented.

Martin Marietta is regularly collecting and tracking the water level data from the wells that they have legal access to. The existing data collected to date (see Table 1 of Attachment A) demonstrates that the water levels in surrounding wells have not been materially impacted by dewatering or the installation of the Stage G1 liner. Consequently, if a complaint is received and the water level data are in the range of the existing data set, the complaint will be judged as unreasonable, and this conclusion will be reported to the DRMS. If the water levels are two feet lower than the existing data set, the mitigation plan will be activated and that plan of action will be reported to the DRMS.

e. Section 2.3 also states that "if a well goes dry, MM will implement mitigation measures within 7 days." In the event that a well owner reports that their well has become unusable, MM should commit to notify DRMS and implement mitigation measures immediately (as soon as practically possible). MM will need to concurrently commence an investigation into the status of the complaint. The results of

this investigation as well as any proposed remediation or rationale for discontinuing mitigation will be submitted to DRMS for approval within 30 days.

Response

In the instance of a complaints that a well has gone dry, if the well monitoring data available indicates that it is reasonable to think the cause of the well going dry may be related to the lining of the reservoirs, Martin Marietta commits to implementing the mitigation measures outlined in the Groundwater Mitigation Plan within 7 days of receipt of the complaint or as soon as is practically possible. (For more details on the process, see response, 40d, above.) Martin Marietta will also commit to investigating the status of the complaint and to report any corrective actions and the results of this investigation, as well as any proposed remediation or rationale for discontinuing mitigation, to the DRMS for approval within 30 days.

f. Due to the mounding impacts already observed and at least partly mitigated along the west side of phase G1, the permittee should provide advance designs for additional underdrains that could be installed if excessive groundwater mounding is observed on the north side of G1 and/or the west side of G2. These additional underdrains could then be rapidly installed (to commence within no more than 30 days) if excessive mounding is identified in these areas during monthly monitoring. Trigger levels may be identified (for example, GW rising to within 3' of the ground surface, or other adverse mounding impacts observed) to trigger implementation of additional corrective actions such as underdrain installation.

Response

Martin Marietta is aware that the flood irrigation of the parcel to the north of phase G1 causes ponding of water on the surface/flooding. This is likely due to surface water drainage challenges, not groundwater because the flooding occurred before the liners were installed.

If it turns out that there is a groundwater mounding issue causing flooding, Martin Marietta will prepare a design and construct a drain on the north side of phase G1. See response to 40g, below for more information.

Martin Marietta does not believe that a drain will be necessary on the west side of Phase G2. The reasons for this conclusion are as follows:

1. *There is already a drain on the north side of Phase G2.*
2. *The west side of Phase G2 is very close to the river.*
3. *The presence of unlined ponds west of Phase G2.*

Furthermore, in the unlikely event that water does mound up west of Phase G2, the presence of surface water drainage ditches along North Taft Hill Road will route water to the river.

g. Some discussion should be provided to justify the depths of the underdrains, both installed and proposed.

Response

The as-constructed drawings for the west side of Phase G1 and north side of Phase G2 are included as Attachment B to this letter.

Charts 2, 3 and 6 of Exhibit G (Attachment A to this letter) have all been updated. They present the ground water level data along the north edge of Phase G1. The ground surface along the north edge of Phase G1 is approximately 5025 feet. The highest ground water elevation after the installation of the liner in November of 2020, is approximately 5021 feet in the northwest corner of Phase G1 (see Chart 2 well HO-1 in Attachment A). The highest water elevation in the northeast corner of the reservoir is approximately 5017 feet (see Chart 3 Well HO-11 in Attachment A). The data indicate that the depth to water is approximately four feet after liner installation; therefore, Martin Marietta believes that a drain is not necessary at this location. However,

in the unlikely event that flooding is due to ground water mounding along the north side of Phase G1, Martin Marietta will install an underdrain.

If an underdrain is necessary, the design will intercept groundwater on the north side (groundwater mound side) of Phase G1 and discharge the intercepted ground water below grade on the east side (groundwater shadow side) of Phase G1. Based on preliminary review, the design may include the following:

a. Four- or six-inch diameter perforated pipe along the north side of Phase G1 from the northwest corner to the northeast corner. The slope drain will follow the ground surface, flowing from west to east.

b. At the northeast corner, the drain will turn south along the east side of Phase G1. Along the east side, the underdrain will be perforated pipe for approximately 50 feet, then transition to solid pipe for about 200 feet.

c. South of the solid section of pipe the drain will again be perforated pipe for about 500 feet. The 500-foot length of perforated pipe will allow the water in the pipe to discharge to the water table in the shadow zone of Phase G1. The design of the drain will include cleanouts at regular intervals to facilitate maintenance of the drain.

6.4.7 EXHIBIT L – Reclamation Costs

41. A cost estimate was not completed at this time because a significant amount of information needed to complete an estimate was requested in this adequacy review. A cost estimate will be completed when the information is received, and Martin Marietta will have an opportunity to review/comment on it.

Response
Noted

As of this letter, there is still one objection on file from Mr. Seaworth.

Response
Since the time the letter was sent to us, we understand that Mr. Seaworth withdrew his objection.

Please note that the decision date for this application is December 14, 2022. If you are unable to provide satisfactory responses to any inadequacies prior to this date, it will be your responsibility to request an extension of time to allow for continued review of this application. Also, the review time may not exceed 365 days from when the application was filed, which was January 10, 2022 (Rule 1.4.1(9)). If more time beyond the 365th day is needed to adequately address the above issues, the matter can be set for a Board hearing at which time the Board may deny, approve with or without conditions (Rule 1.4.1(9)).

Response
We extended the decision date to January 9, 2022, and Martin Marietta has asked the Board to further extend the decision date to February 27, 2023, so we can have a little more time to resolve the few remaining issues. With this letter we have attempted to respond to all your comments with the exception of mapping the groundwater, per comment 40c. We estimate that we will need until the end of January to complete this exercise and then we will submit it to the DRMS for review.

All corrected pages must also be provided to Larimer County Clerk & Recorder.

Response
A copy of this letter and Attachments A and B have been submitted to the Larimer County Clerk and Recorder. Documentation that it was submitted to the County is attached to this letter as Attachment C.

Thank you for your consideration. If you have any questions or need additional information, please contact me at 720-864-4507, pam.hora@tetratech.com.

Sincerely,

TETRA TECH

Pamela Franch Hora

Pamela Franch Hora, AICP
Senior Planner

Attachment A: Exhibit G, revised January 2023
Attachment B: As-Constructed Drawings
Attachment C: Documentation showing Cover Letter and Attachments A and B are on file at Larimer County

cc: Julie Mikulas, Martin Marietta

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TETRA TECH

January 16, 2023

Mr. Brock Bowles
Division of Reclamation, Mining and Safety
1313 Sherman Street, Room 215
Denver, CO 80203

RE: Adequacy Review 2 follow-up, DRMS AM-4 application – Home Office Mine, Permit M1977-439

Dear Mr. Bowles:

As you know when we submitted our response to your Adequacy Review 2 comments regarding Martin Marietta's 112c Permit Amendment Application (AM4) for the Home Office Mine (File No. M-1977-439), we still owed you groundwater contour mapping. Below is the comment you had made, our revised response, and attached are the contour maps you requested.

6.4.7 EXHIBIT G – Water Information

39. The following questions are for the water information submitted with the AM4 application and refer to Stage G:

    c. Presentation of historic groundwater level data in the form of GW contour maps to show, where possible, historic GW flow directions and levels/depth below ground surface, along with impacts to the historic patterns due to mining and reclamation. These should place an emphasis on current data to demonstrate minimization of mounding and shadowing impacts.

    Response
    Tetra Tech has prepared water level contour maps for the area adjacent to the Phase G pits to characterize the general conditions for: pre-mining, dewatered, and the lined reservoir. The three figures are attached.

Thank you for your consideration. If you have any questions or need additional information, please contact me at 720-864-4507, pam.hora@tetratech.com.

Sincerely,

TETRA TECH

Pamela Franch Hora

Pamela Franch Hora, AICP
Senior Planner

Enclosures: Figure A: Pre-Mining Groundwater Contours
Figure B: Dewatering Groundwater Contours
Figure C: Lined Reservoir Groundwater Contours

cc: Julie Mikulas, Martin Marietta

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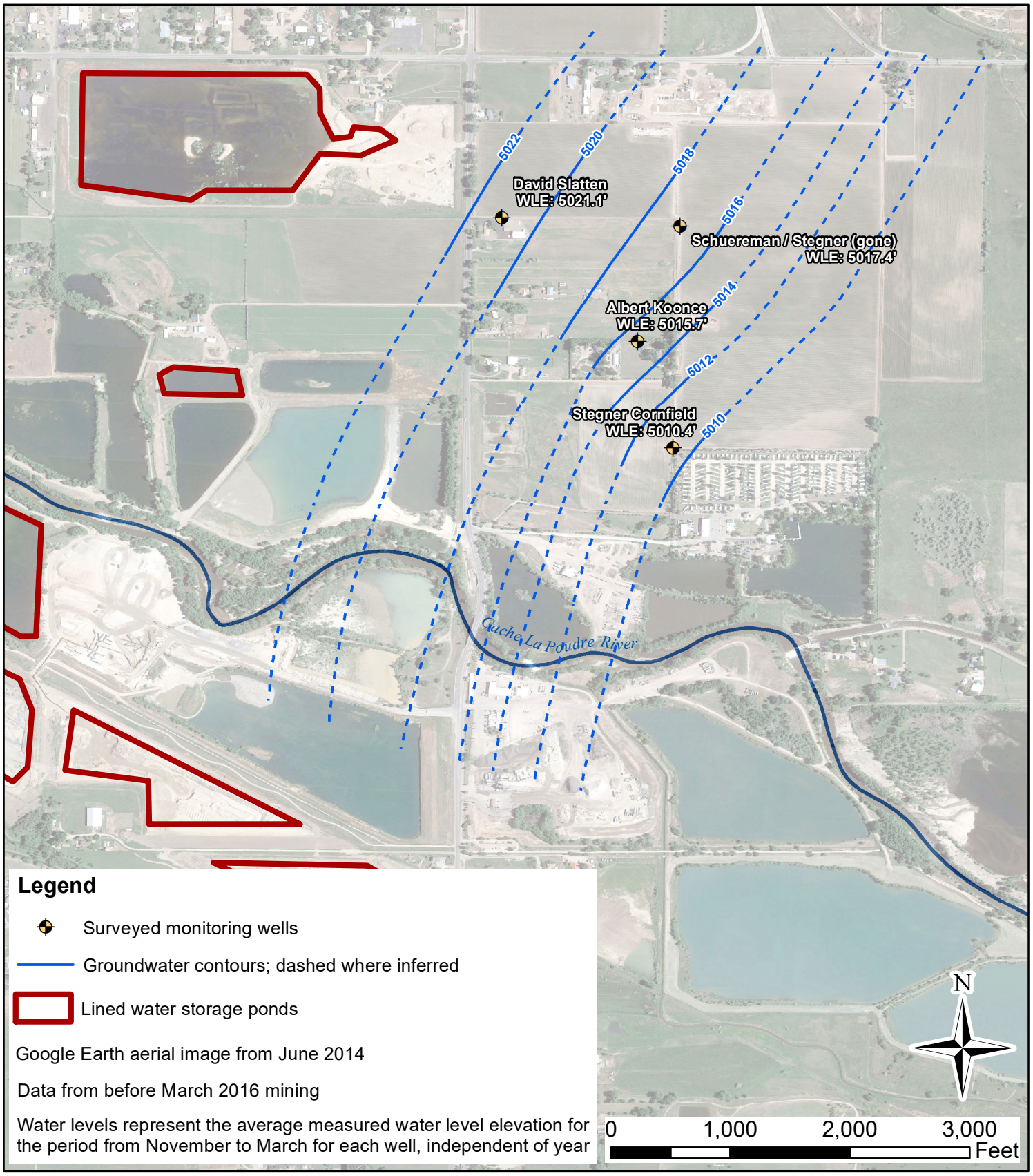

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TETRA TECH

351 Coffman St., Suite 200
Longmont, CO 80501
PH: (303) 772-5282

MARTIN MARIETTA

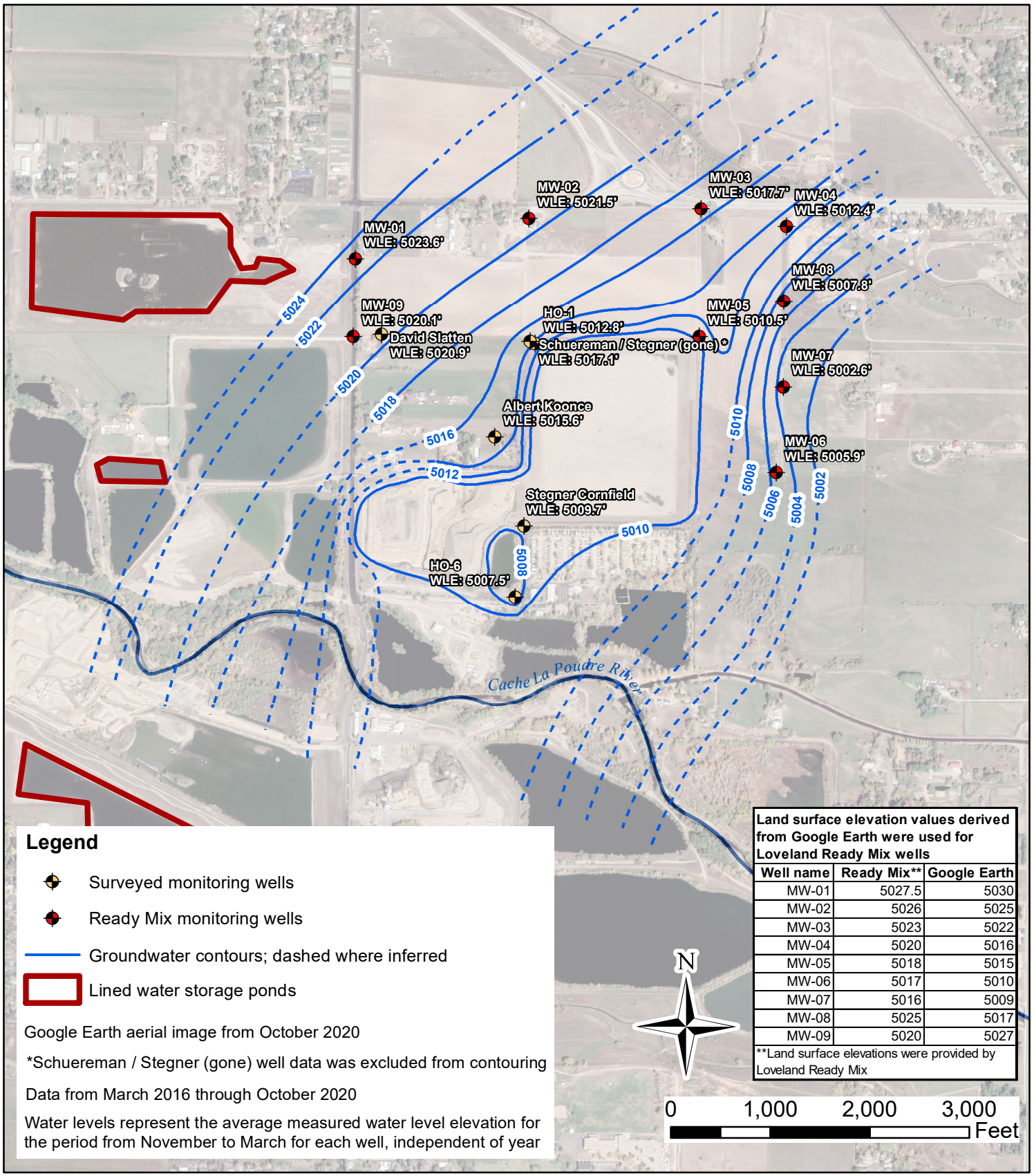
Taft Hill,
Fort Collins, CO

PRE-MINING
GROUNDWATER CONTOURS

Project No.: 117-8741002

Date: January 11, 2023

Figure
A



Legend

- Surveyed monitoring wells
- Ready Mix monitoring wells
- Groundwater contours; dashed where inferred
- Lined water storage ponds

Google Earth aerial image from October 2020

*Schuereman / Stegner (gone) well data was excluded from contouring

Data from March 2016 through October 2020

Water levels represent the average measured water level elevation for the period from November to March for each well, independent of year

Land surface elevation values derived from Google Earth were used for Loveland Ready Mix wells

Well name	Ready Mix**	Google Earth
MW-01	5027.5	5030
MW-02	5026	5025
MW-03	5023	5022
MW-04	5020	5016
MW-05	5018	5015
MW-06	5017	5010
MW-07	5016	5009
MW-08	5025	5017
MW-09	5020	5027

**Land surface elevations were provided by Loveland Ready Mix

TETRA TECH

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PH: (303) 772-5282

MARTIN MARIETTA

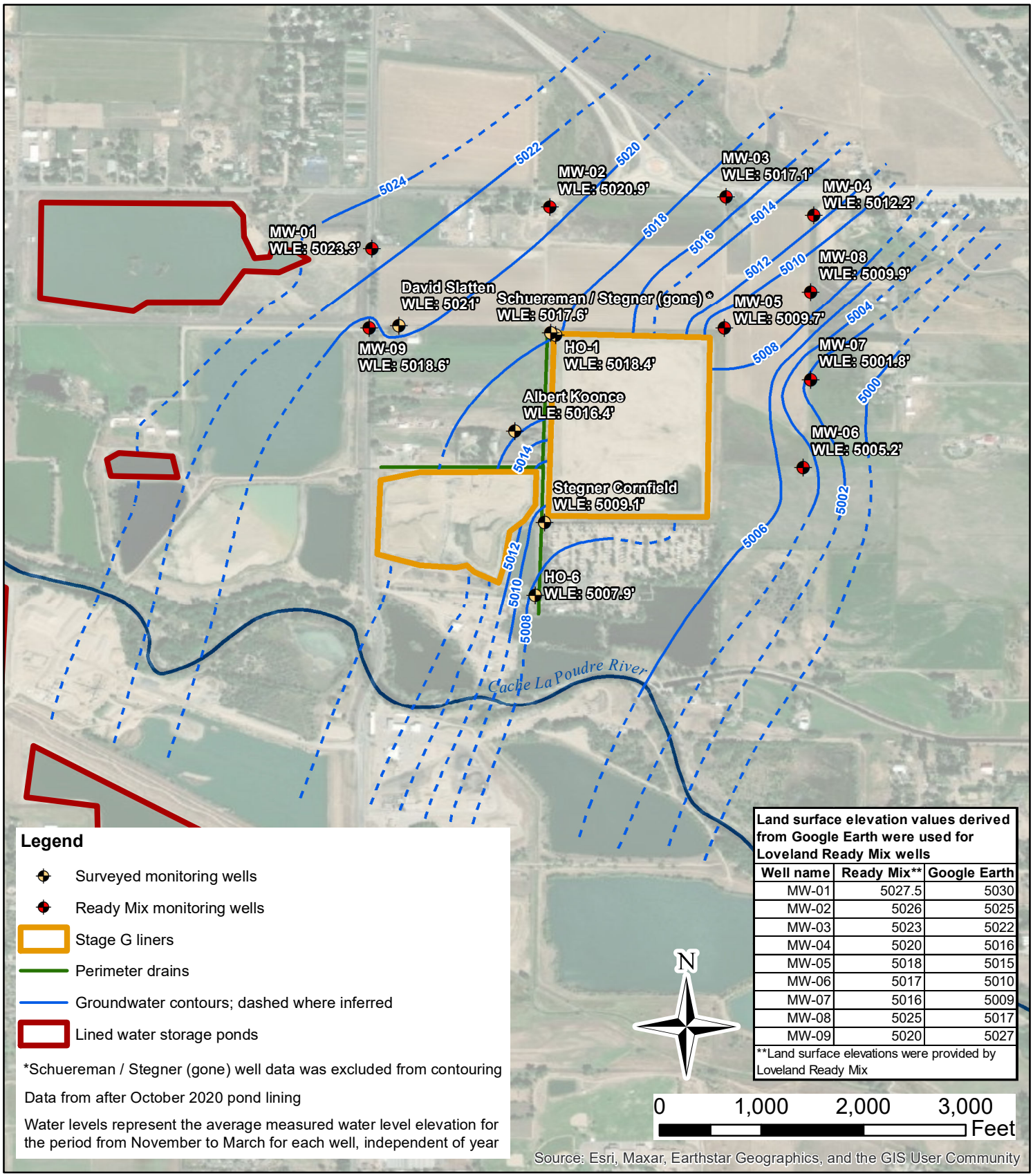
Taft Hill,
Fort Collins, CO

DEWATERING
GROUNDWATER CONTOURS

Project No.: 117-8741002

Date: January 11, 2023

Figure
B



Legend

- Surveyed monitoring wells
- Ready Mix monitoring wells
- Stage G liners
- Perimeter drains
- Groundwater contours; dashed where inferred
- Lined water storage ponds

*Schuereman / Stegner (gone) well data was excluded from contouring

Data from after October 2020 pond lining

Water levels represent the average measured water level elevation for the period from November to March for each well, independent of year

Land surface elevation values derived from Google Earth were used for Loveland Ready Mix wells

Well name	Ready Mix**	Google Earth
MW-01	5027.5	5030
MW-02	5026	5025
MW-03	5023	5022
MW-04	5020	5016
MW-05	5018	5015
MW-06	5017	5010
MW-07	5016	5009
MW-08	5025	5017
MW-09	5020	5027

**Land surface elevations were provided by Loveland Ready Mix

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TETRA TECH

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MARTIN MARIETTA

Taft Hill,
Fort Collins, CO

LINED RESERVOIR
GROUNDWATER CONTOURS

Project No.: 117-8741002

Date: January 11, 2023

Figure
C

Julie Mikulas
Regional Land Manager

January 16, 2023

Ms. Angela Myers
Larimer County Clerk and Recorder
200 West Oak Street
Fort Collins, CO 80521

RE: Home Office Pits, additional pages for 112 Regular Construction Materials Reclamation Permit Amendment Application, County Copy of Public Notice Documents

Dear Ms. Myers:

Attached are additional pages to the 112(c) application to the Colorado Division of Reclamation, Mining, and Safety for the operation known as the Home Office Pits. This information has been provided to the Colorado Division of Reclamation, Mining, and Safety as part of the permit application process and are to be available for public review until the amendment is approved.

If you have any questions or concerns, please contact me at (970) 407-3661.

Sincerely,

Julie Mikulas
Regional Land Manager

The public notice documents were received on the following date: 1/17/2023

By: _____

LARIMER COUNTY CLERK
SEAL
COLORADO

Rocky Mountain Division – Northern Office
1800 N Taft Hill Road, Fort Collins, CO 80534
julie.mikulas@martinmarietta.com
www.martinmarietta.com

EXHIBIT G: WATER INFORMATION

Martin Marietta is amending the existing 112 Reclamation Permit No. M-1977-439 to change the final reclamation for Area G of the Home Office site from one open water lake to two sealed water storage reservoirs using compacted clay embankment liners.

1.0 INTRODUCTION AND BACKGROUND

Martin Marietta owns properties known as the "Home Office" site in Larimer County, Colorado. The properties are located on the west and east sides of North Taft Hill Road, approximately ½ mile south of Larimer County Road 54G, in Sections 33 and 34 of Township 8 North, Range 69 West of the 6th Principal Meridian, and Sections 3 and 4 of Township 7 North, Range 69 West of the 6th Principal Meridian.

This Groundwater Monitoring and Mitigation Plan presents the methods for monitoring groundwater during mining and reclamation, and for mitigating any potential groundwater impacts associated with permitted mining at the site. Martin Marietta is applying to the Colorado Division of Reclamation, Mining and Safety (DRMS) for an Amendment to the existing 112 Reclamation Permit No. N-1977-439 to change the final reclamation for Area G of this site from one open water lake to two sealed water storage reservoirs using a compacted clay embankment liner.

Exhibit B shows the location of the Affected Area and Area G. Exhibits F1 and F2 show all the Affected Area in the permit. Exhibit F3 details Area G, which is located in the northern portion of the Affected Area. The changes within the Affected Area are limited to Area G. Consequently, this discussion is limited to potential changes in the hydrologic balance as a result of the installation of compacted clay embankment liners in Area G-I and Area G-II. Figure G-1, enclosed, shows the Affected Area; Area G; adjacent parcels to Area G and property owners; and conceptual groundwater flows before and after the installation of the compacted clay liners.

1.1 HISTORIC USE

Area G mining and associated dewatering began in Spring 2016 and is currently underway. A compacted clay liner was installed in Area G-I in September 2020 upon completion of mining. A compacted clay liner will be installed in Area G-II upon completion of mining.

Figure G-1 shows the adjacent properties to Area G, lined cells, unlined cells, cells that will be lined in the future, and irrigation ditches in the vicinity of the Affected Area. Agricultural land uses are located north and east of Area G. Loveland Ready Mix owns properties to the north and east of Area G. Irrigation ditches in the area include the New Mercer Canal, the Larimer County Canal No. 2 Ditch, the Arthur Canal, and Taylor & Gill Lateral (owned by Martin Marietta).

Loveland Ready Mix irrigates fields north and east of Area G outside of the permit area. Irrigation field tiles are reported to have been installed in the properties owned by Constance A Fredman located southeast of Area G of the Affected Area (Figure G-2). After reports of poor drainage, the field tiles were repaired by Loveland Ready Mix. Flooding was reported to Martin Marietta on the PKR Farm LLC property located east of the Affected Area (Figure G-2) as part of the field tile issue. In response, Martin Marietta worked with Loveland Ready Mix and PKR Farm LLC and installed drainage ditches between Loveland Ready Mix and PKR Farms LLC and along the east side of Area G-I. The drainage ditches convey irrigation runoff from the area east of the Affected Area to the south to the Cache La Poudre River, similar to the historic drainage patterns. Since the installation of the ditches, there have not been further reports of flooding.

Based on discussions with Seaworth Properties LLC, water is rising on the east side of a parcel owned by Seaworth Properties LLC, on the west side of Area G-I during the irrigation season. Martin Marietta is working with Seaworth Properties LLC and installed a groundwater perimeter drain between January 10, 2022 and

February 4, 2022 to address the groundwater rise and irrigation return flow to the river. Figure F-3 presents the location and design drawings for the perimeter drain (See Section 2.1 Mining Plan). An additional section of perimeter drain is proposed along the north side of Area G-II. This section will connect to the installed perimeter drain at the northeast corner of Area G-II. A swale was installed on Martin Marietta's property on the west side of Area G-I to facilitate surface drainage for landowners located west of Area G-I. Figure G-3 shows the approximate locations of the perimeter drain and swale.

Chart 1 illustrates the mining cells in the vicinity of the Affected Area and the liner status of each cell.

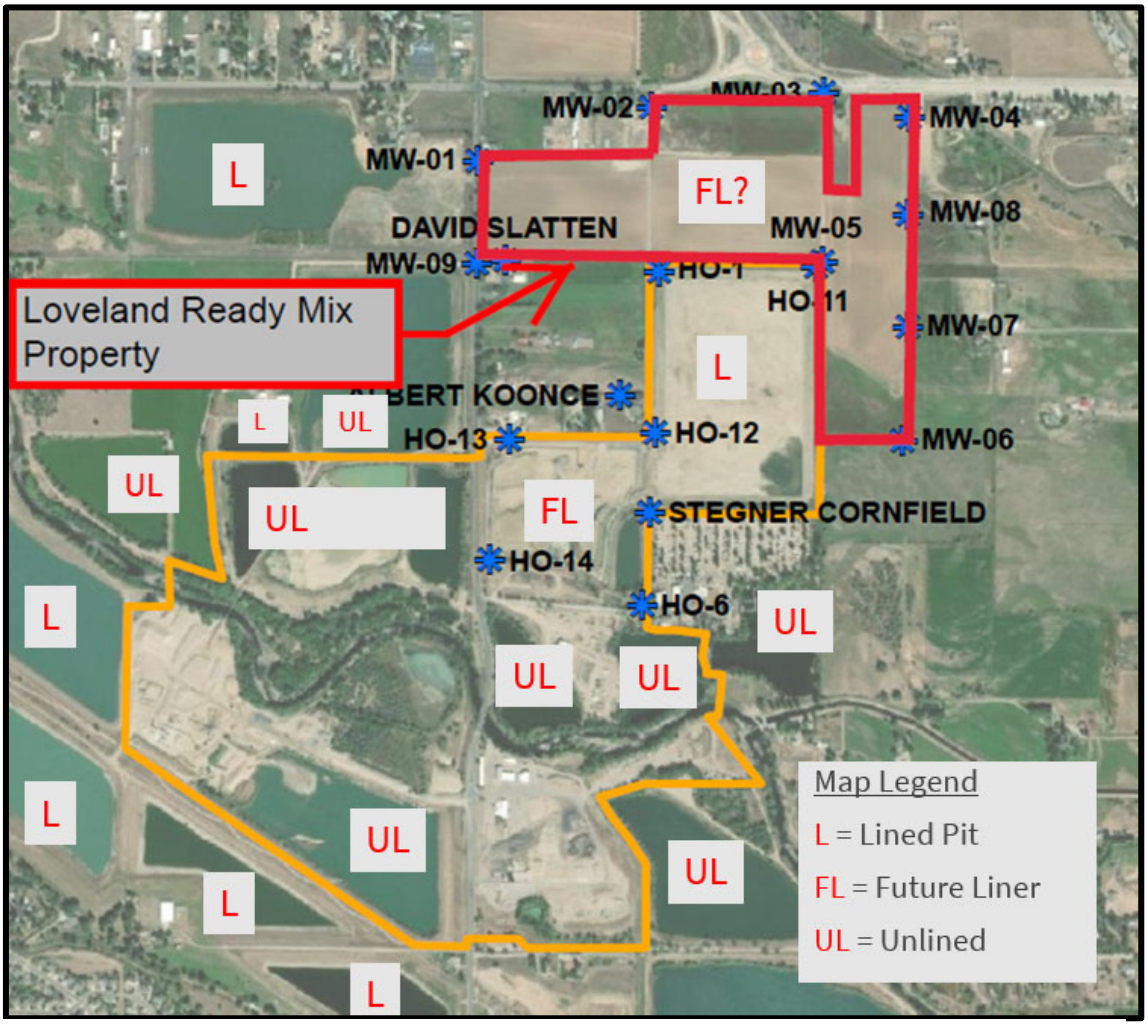


Chart 1: Mining Cells in the Vicinity & Liner Status

1.2 EXISTING WELLS

1.2.1 Monitoring Wells

Six monitoring wells (HO-1, 6, 11, 12, 13 and 14) were installed outside the limits of proposed mining, allowing for groundwater monitoring to occur during and after mining.

Monitoring well HO-1 was installed in the northwest corner of Area G-I. Monitoring well HO-6 was installed in the southeast corner of Area G-II. Both HO-1 and HO-6 were installed in 2018. Monthly groundwater monitoring for

wells HO-1 and HO-6 began in May 2018. The well monitoring program documented pre-mining groundwater levels near the project area and seasonable fluctuations.

The following monitoring wells were installed in Area G in 2021:

- HO-11, in the northeast corner of Area G-I
- HO-12, near the southern portion of Area G-I, and near the northeast corner of Area G-II
- HO-13, near the northwest corner of Area G-II
- HO-14, near the southwest corner of Area G-II

Monthly groundwater monitoring for wells HO-11, HO-12, HO-13, and HO-14 began in May 2021. Exhibit F3, dated August 2021, and Figure G-3 show the locations of the wells.

Chart 1 shows water level measurements for HO-1 and HO-6. Chart 2 shows water level measurements for HO-11, HO-12, HO-13, and HO-14. Exhibit G, Attachment 1 shares the raw data for wells HO-1, HO-6, HO-11, HO-12, HO-13, and HO-14.

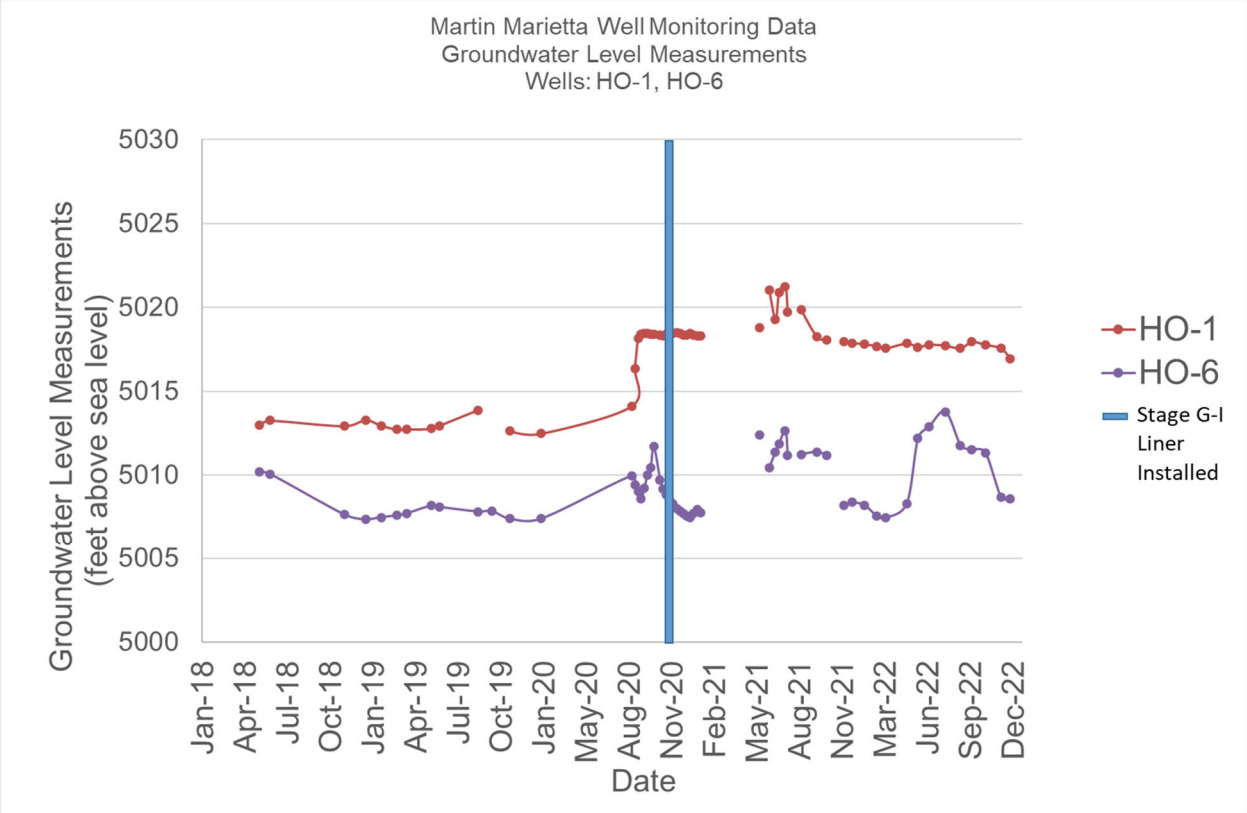


Chart 2: Martin Marietta Monitoring Data (HO-1 and HO-6)

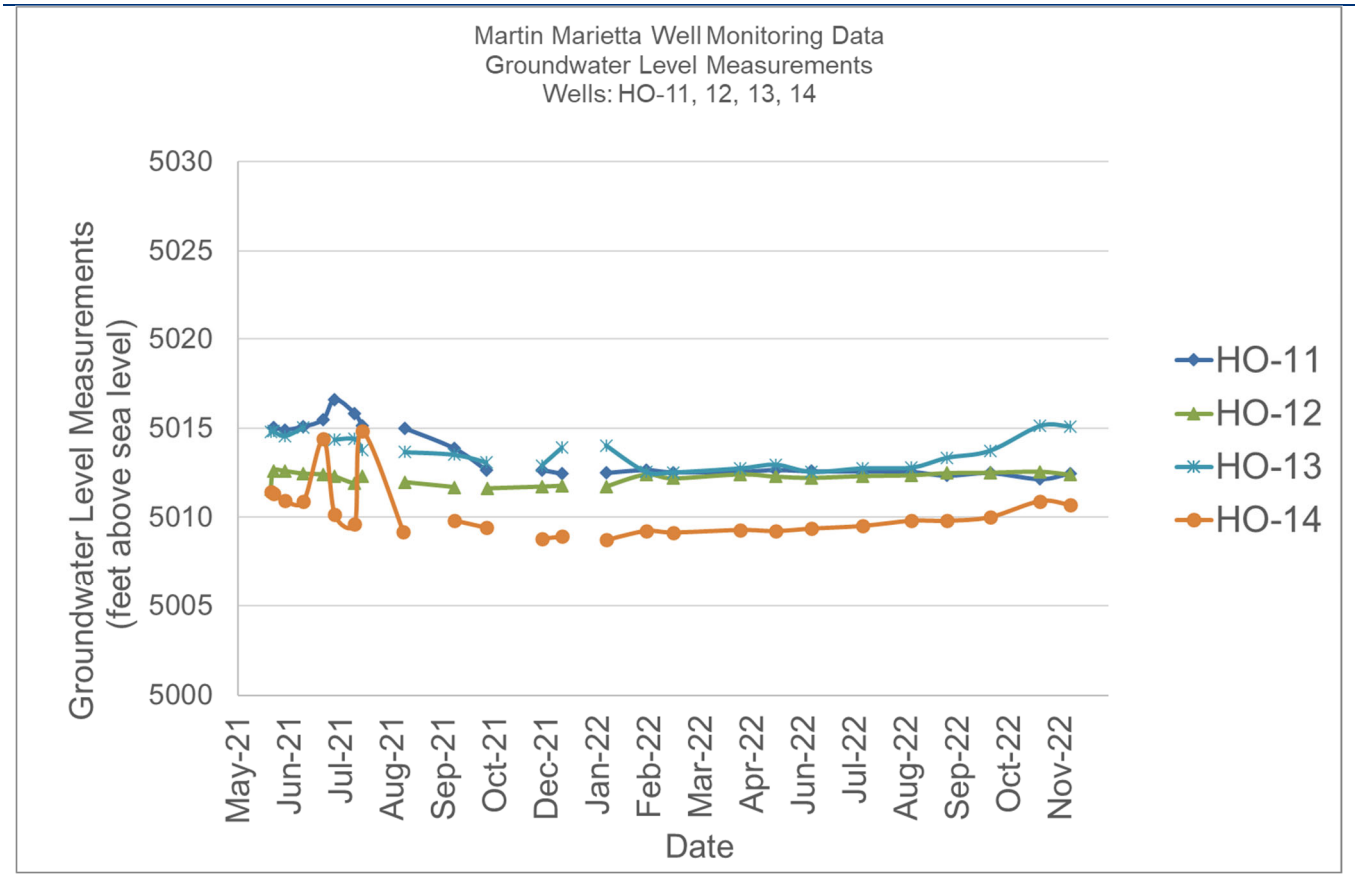


Chart 3: Well Monitoring Data (HO-11, 12, 13, 14)

Loveland Ready Mix installed monitoring wells on their lands in July 2016. Chart 4 and 5 show water level measurements that have been collected monthly since the wells were installed. Monitoring wells MW-02, MW-03 and MW-04 are located north of Area G-I. Monitoring wells MW-05, MW-06, MW-07 are located east of Area G-I. Groundwater monitoring for monitoring wells MW-02 through MW-07 began in July 2016. Groundwater levels fluctuate throughout the seasons. Exhibit G, Attachment 1 presents data provided by Loveland Ready Mix.

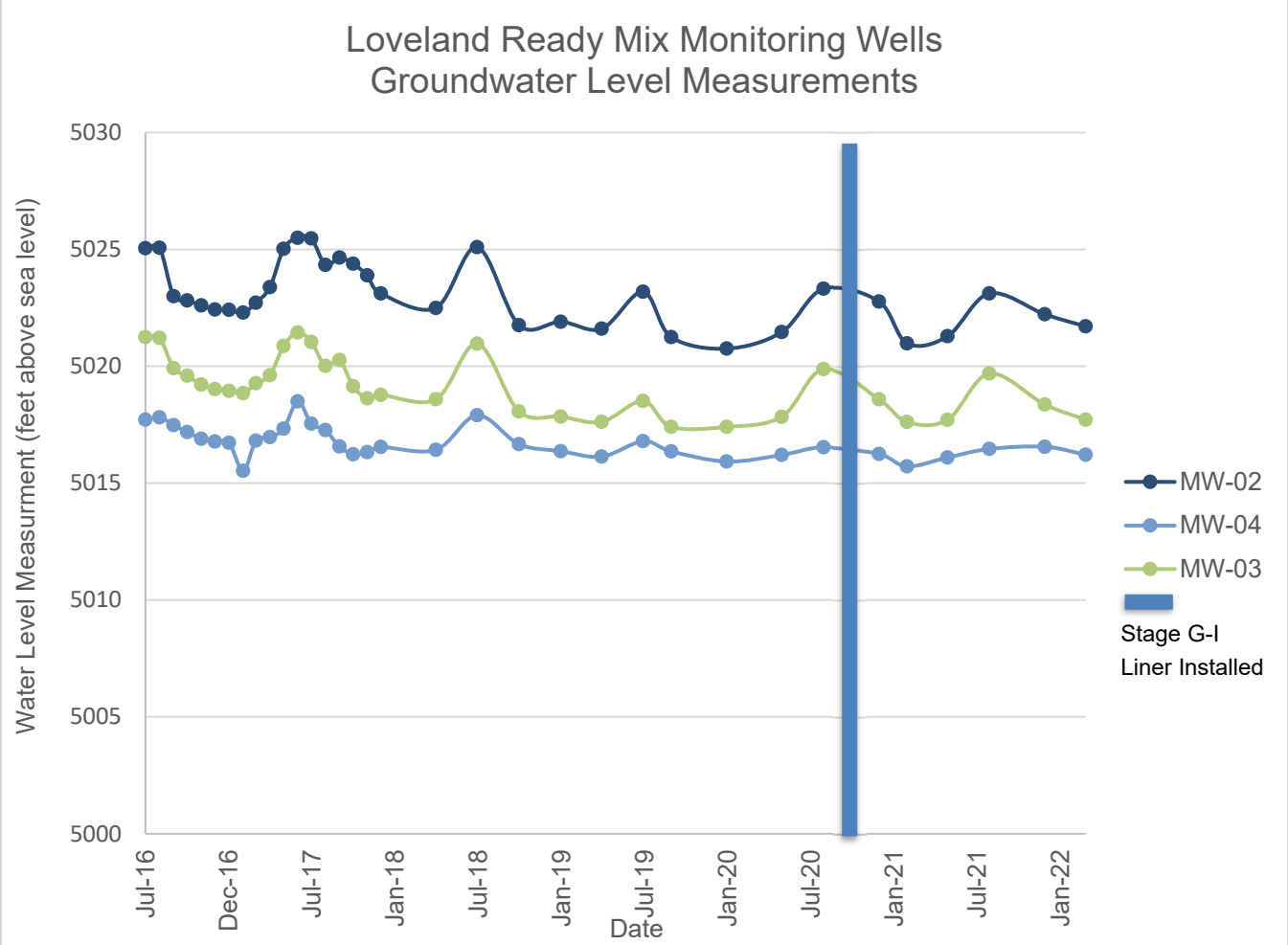


Chart 4: Loveland Ready Mix Monitoring Wells, MW-02, 03, 04

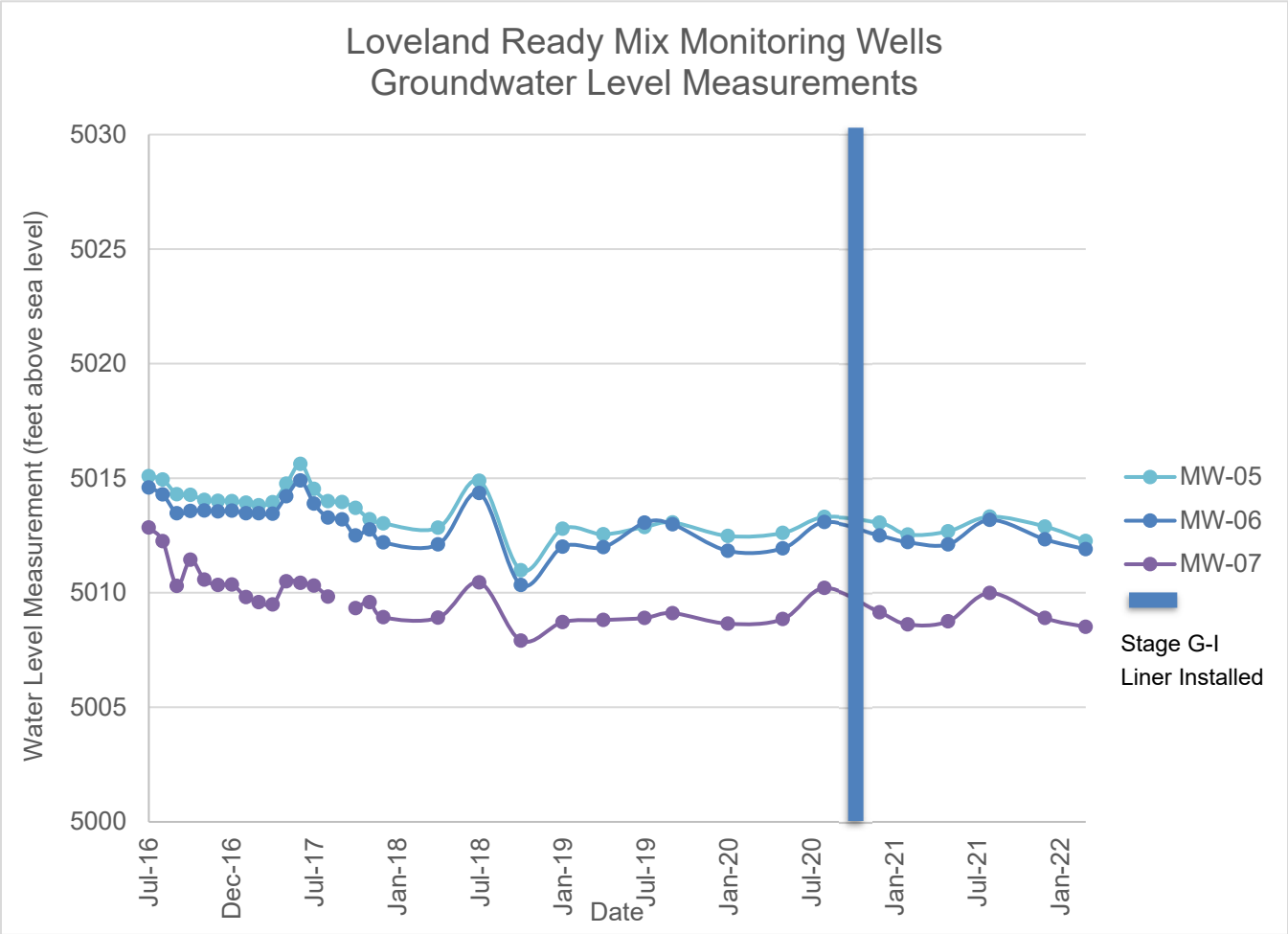


Chart 5: Loveland Ready Mix Monitoring Wells, MW-05, 06, 07

1.2.2 Nearby Wells

Numerous wells were installed in 2001 to establish groundwater baselines to monitor the groundwater conditions before, during, and after mining per the 2005 Well Monitoring Program Interim Report for Home Office. Water level measurements from 2004 and 2005 are included in the 2005 Well Monitoring Program Interim Report Well locations are shown on Figure G-3. Well owners, David L and Virginia S Slatten's property is located west of Area G-I at the northernmost portion of Area G-I (Figure G-1). Well Owners, Albert R and Sharon E Koonce's property is located west of Area G-I, just north of Area G-II. A well owned by Martin Marietta, referred to as "MM near KOA" is located east of Area G-II and south of Area G-I. Groundwater monitoring for the Slatten, Koonce and MM near KOA wells began in April 2007. Groundwater levels fluctuate throughout the seasons. The range of water level measurements for the Slatten well is 6.6 feet. The range of water level measurements for the Koonce well is 8.5 feet. The range of water level measurements for the MM near KOA well is 7.7 feet. Table 1 presents the maximum, minimum and average saturated thickness for the monitoring well data set. Mining and associated dewatering began in Spring 2016. A shift in water level measurements is shown in Chart 3, which presents historical well monitoring data from the Slatten, Koonce and MM near KOA wells. Exhibit G, Attachment 1 presents raw data for the nearby wells.

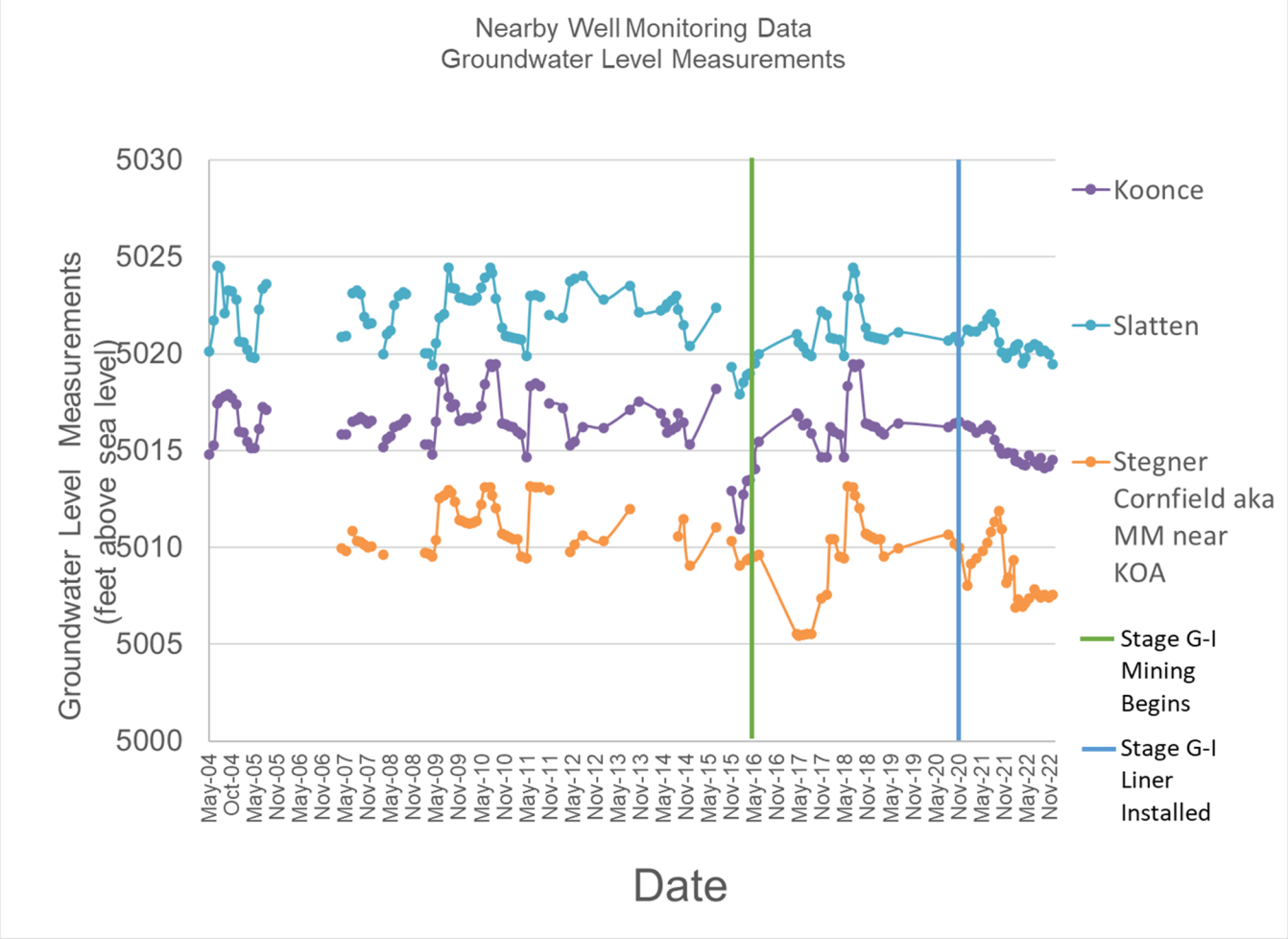


Chart 6: Nearby Well Monitoring Data

1.3 WELL INVENTORY

In December 2021, a well inventory of the Affected Area and adjacent areas was conducted to identify domestic wells near the project site. The inventory involved a review of constructed well records on file with the Colorado Division of Water Resources, located within ½ mile plus 200 feet of the Affected Area. The well inventory identified 98 constructed wells within ½ mile plus 200 feet, of the Affected Area. Figure G-2 enclosed shows the Affected Area and the constructed well locations on file with the Colorado Division of Water Resources.

1.4 HISTORIC AND FUTURE GROUND WATER LEVELS, AND IMPACTS TO WELLS

The presence of lined and unlined cells near Area G and previous dewatering of Area G demonstrates that the revised reclamation plan, i.e. lining of Area G cells will result in changes, but manageable changes, to the hydrologic balance.

The principal change to the hydrologic balance will be mounding on the upgradient (west) side of Area G. Martin Marietta has been preemptive in addressing the mounding on the east side by installing a perimeter drain. Exhibit G, Attachment 2 presents the location and design of the perimeter drain.

Regarding the down gradient impacts, monitoring well data demonstrates minimal impacts to wells will occur on the down gradient shadow effect (east and southeast) sides of Area G. Table 1 presents the historic range of

saturated thickness in the monitoring wells. The period or record for the data includes the time when the Area G cells were being actively dewatered. The impacts from dewatering are greater than the shadow effect of the lined cells. Monitoring wells HO-6 and Stenger Cornfield/MM Near KOA are adjacent to the dewater cells yet they had minimum statured thickness of 6.9 feet and 4.6 feet respectively. Chart 7 presents the hydrograph of wells HO-6 and the Stegner Cornfield/MM Near KOA wells. The impacts of dewatering and subsequent recovery of the water levels after installation of the liner in the northern Area G cell.

The saturated thickness in the shadow zone after installation of the liner will be on the order of five to six feet. Water wells completed in sand and gravel aquifers typically provide approximately 25 to 30 gallons per minute per foot of drawdown or saturated thickness in the well. The wells on the down gradient side of Area G are domestic with permitted maximum pumping rates of 15 gallons per minute (gpm). Consequently, five to six feet of saturated thickness will provide the allowed pumping rates of 15 gpm. Wells located further from Area G will have even more saturated thickness and hence will be able to pump the permitted rates.

Table 1: Saturated Thickness

Saturated Thickness (feet)				
Well	**Minimum**	**Maximum**	**Delta**	**Average**
HO-1	5.8	14.6	8.8	10.4
HO-6	6.9	13.3	6.4	8.9
HO-11	9.7	14.2	4.5	11.1
HO-12	6.4	7.5	1.0	7.0
HO-13	5.8	8.4	2.6	7.0
HO-14	3.2	9.3	6.1	4.7
Koonce	5.5	14.0	8.51	10.7
Slaten	7.9	14.5	6.57	11.5
Stegner Cornfield/MM Near KOA	4.6	12.4	7.7	9.3
MW-01	9.1	16.8	7.7	11.4
MW-02	11.8	16.5	4.7	14.0
MW-03	10.4	14.4	4.0	12.1
MW-04	11.5	14.5	3.0	12.7
MW-05	7.5	12.1	4.6	10.0
MW-06	6.3	10.9	4.6	9.0
MW-07	8.9	13.9	4.9	10.7
MW-08	6.8	16.0	9.2	10.5
MW-09	6.0	12.8	6.8	8.2

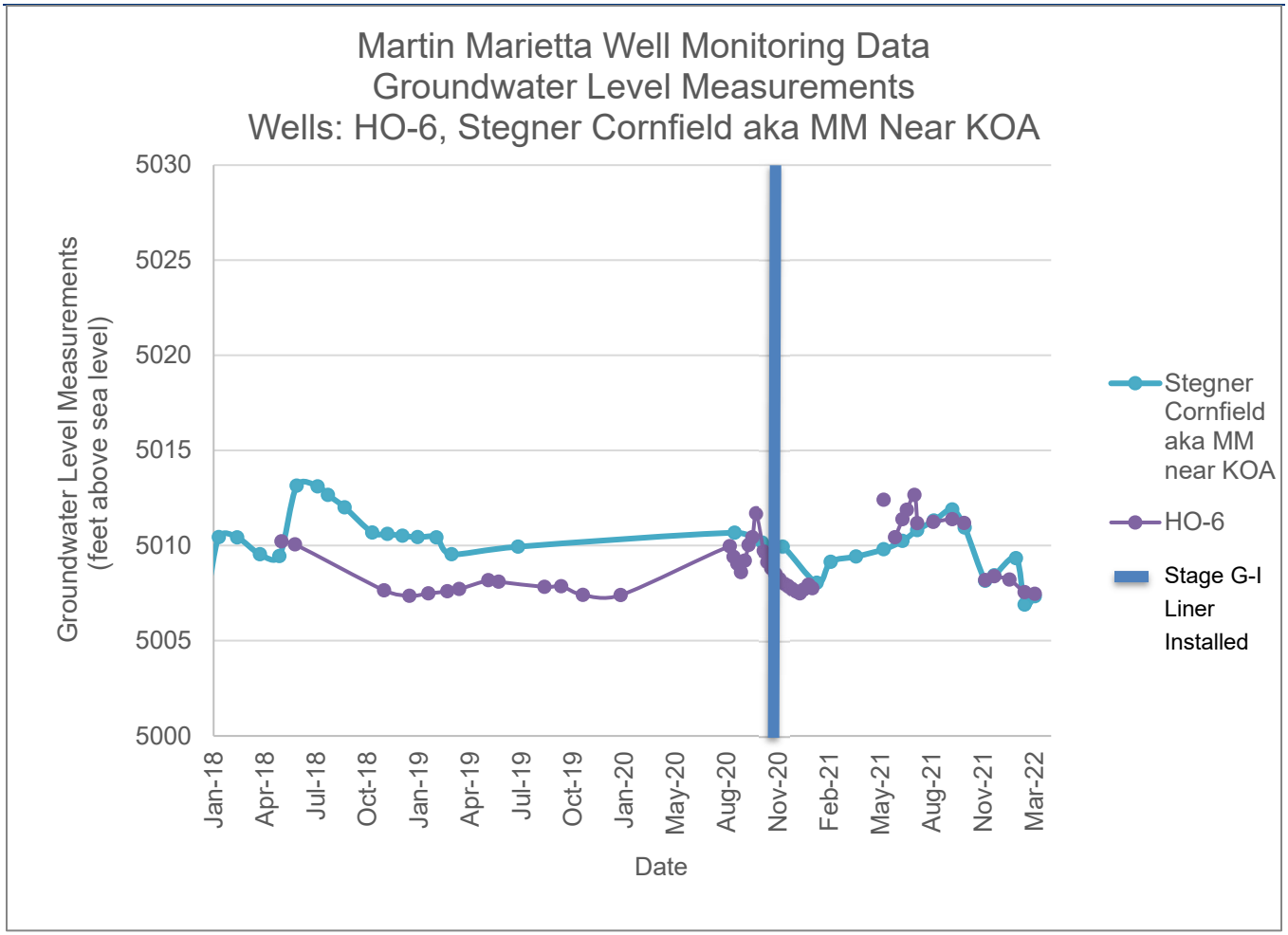


Chart 7: Monitoring Well Data (HO-6, Stegner Cornfield aka MM Near KOA)

2.0 MONITORING AND MANAGEMENT

2.1 MINING PLAN

The Reclamation Plan has been designed to reduce potential groundwater impacts to adjacent properties. A perimeter drain was designed by Deere and Ault to mitigate groundwater rise and to allow irrigation return flow off of the properties being irrigated west of Area G-I. The perimeter drain is installed along the west Area G-I. An additional portion of perimeter drain is proposed along the north side of Area G-II. Mining is expected to be completed in Area G-II by the end of 2022. The portion of the perimeter drain north of Area G-II will be installed upon completion of mining. The perimeter drain discharges through the berm between Area G-II and II to an unlined pond that is tributary to the Cache la Poudre River.

2.2 MONITORING

2.2.1 Martin Marietta Monitoring Wells

Monthly water level monitoring at HO-1, HO-6, HO-11, HO-12, HO-13, and HO-14 will continue during mining. When mining is complete, quarterly water level monitoring at HO-1, HO-6, HO-11, HO-12, HO-13, and HO-14 will continue until reclamation is complete and the DRMS releases the financial warranty bond.

2.2.2 Domestic Water Wells

Monthly water level monitoring at the Slatten and Koonce wells will continue during mining. When mining is complete, quarterly water level monitoring at the Slatten and Koonce wells will continue until reclamation is complete and the DRMS releases the financial warranty bond.

2.3 MITIGATION

The available monitoring well data will be used to identify changes in alluvial groundwater flow associated with mining and reclamation activities. Baseline data collected from the monitoring program will provide a range of relative water levels associated with pre-mining groundwater conditions. These data will be utilized to evaluate the nature and extent of the change to the prevailing hydrologic balance and if necessary, provide for the development of corrective actions.

In the event of a well owner complaint within 600 feet of the Affected Area, Martin Marietta will review the available monitoring information and submit a report to the DRMS within 30 days. The report will include discussions with any well owner who has contacted Martin Marietta regarding a concern and a review of baseline data from the well and vicinity to evaluate whether changes may be due to seasonal variations, climate, mining, or other factors. The report will identify the extent of potential or actual impacts associated with the changes. If the extent of groundwater changes due to mining or reclamation activities is determined to be a significant contributing factor that has or may create adverse impacts, the mining-associated impacts will be addressed to the satisfaction of the DRMS.

If the DRMS determines that the impact on a well for which temporary mitigation has been initiated is not a result of Martin Marietta's activities, or is not solely a result of Martin Marietta's activities, Martin Marietta will reduce or cease mitigation accordingly.

If a well goes dry due to mining or reclamation activities, Martin Marietta will implement mitigation measures within 7 days. Mitigation measures would include providing a temporary alternative water supply that meets the documented historic well production or need, until further investigation can be conducted to determine if the well condition is due to the mining operation.

Martin Marietta will begin to implement one or more mitigation measures if mining or reclamation activity is determined to be a significant contributing factor to groundwater changes requiring mitigation.

Temporary mitigation measures may include, but are not limited to:

- Compensation for well owners to use their existing treated water system to replace the well production loss.
- Provide a water tank and deliver water as necessary to meet documented historic well production or need.
- Other means acceptable to both the well owner and Martin Marietta.

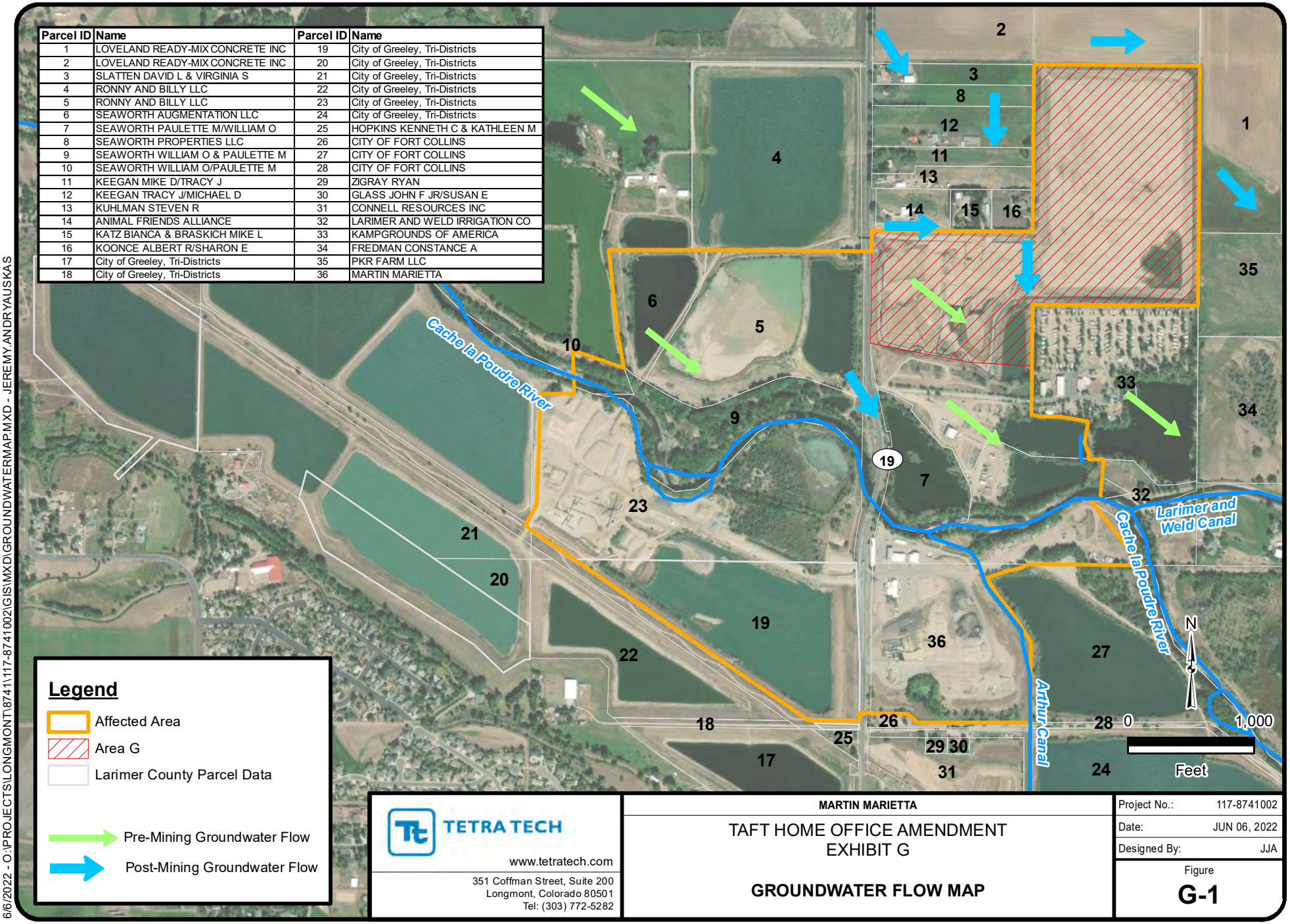
Long-term mitigation measures may include, but are not limited to:

- Cleaning a well to improve efficiency.
- Providing an alternative source of water or purchasing additional water to support historic well use with respect to water quantity and quality. If needed, water quality parameters will be checked in affected wells to ensure alternative sources support the historic use.
- Modifying a well to operate under lower groundwater conditions. This could include deepening existing wells or lowering the pumps. All work would be done at Martin Marietta's expense with the exception of replacing equipment that was non-functional prior to mining.
- If existing wells cannot be retrofitted or repaired, replacing the impacted well with a new replacement well.
- Design and installation of a cistern.

If a groundwater mitigation action is required, Martin Marietta will notify the DRMS of the condition, action taken and report the results and present a plan for monitoring the mitigation.

O:\Projects\Longmont\8741\117-8741002\Docs\Permits\DRMS Adequacy Review 1\ExhG\Adequacy Response 2\ExhGWaterInformation_2023 0110.docx

FIGURES



Parcel ID	Name	Parcel ID	Name
1	LOVELAND READY-MIX CONCRETE INC	19	City of Greeley, Tri-Districts
2	LOVELAND READY-MIX CONCRETE INC	20	City of Greeley, Tri-Districts
3	SLATTEN DAVID L & VIRGINIA S	21	City of Greeley, Tri-Districts
4	RONNY AND BILLY LLC	22	City of Greeley, Tri-Districts
5	RONNY AND BILLY LLC	23	City of Greeley, Tri-Districts
6	SEAWORTH AUGMENTATION LLC	24	City of Greeley, Tri-Districts
7	SEAWORTH PAULETTE M/WILLIAM O	25	HOPKINS KENNETH C & KATHLEEN M
8	SEAWORTH PROPERTIES LLC	26	CITY OF FORT COLLINS
9	SEAWORTH WILLIAM O & PAULETTE M	27	CITY OF FORT COLLINS
10	SEAWORTH WILLIAM O/PAULETTE M	28	CITY OF FORT COLLINS
11	KEEGAN MIKE D/TRACY J	29	ZIGRAY RYAN
12	KEEGAN TRACY J/MICHAEL D	30	GLASS JOHN F JR/SUSAN E
13	KUHLMAN STEVEN R	31	CONNELL RESOURCES INC
14	ANIMAL FRIENDS ALLIANCE	32	LARIMER AND WELD IRRIGATION CO
15	KATZ BIANCA & BRASKICH MIKE L	33	KAMPGROUNDS OF AMERICA
16	KOONCE ALBERT R/SHARON E	34	FREDMAN CONSTANCE A
17	City of Greeley, Tri-Districts	35	PKR FARM LLC
18	City of Greeley, Tri-Districts	36	MARTIN MARIETTA

TETRA TECH
www.tetratech.com
351 Coffman Street, Suite 200
Longmont, Colorado 80501
Tel: (303) 772-5282

MARTIN MARIETTA

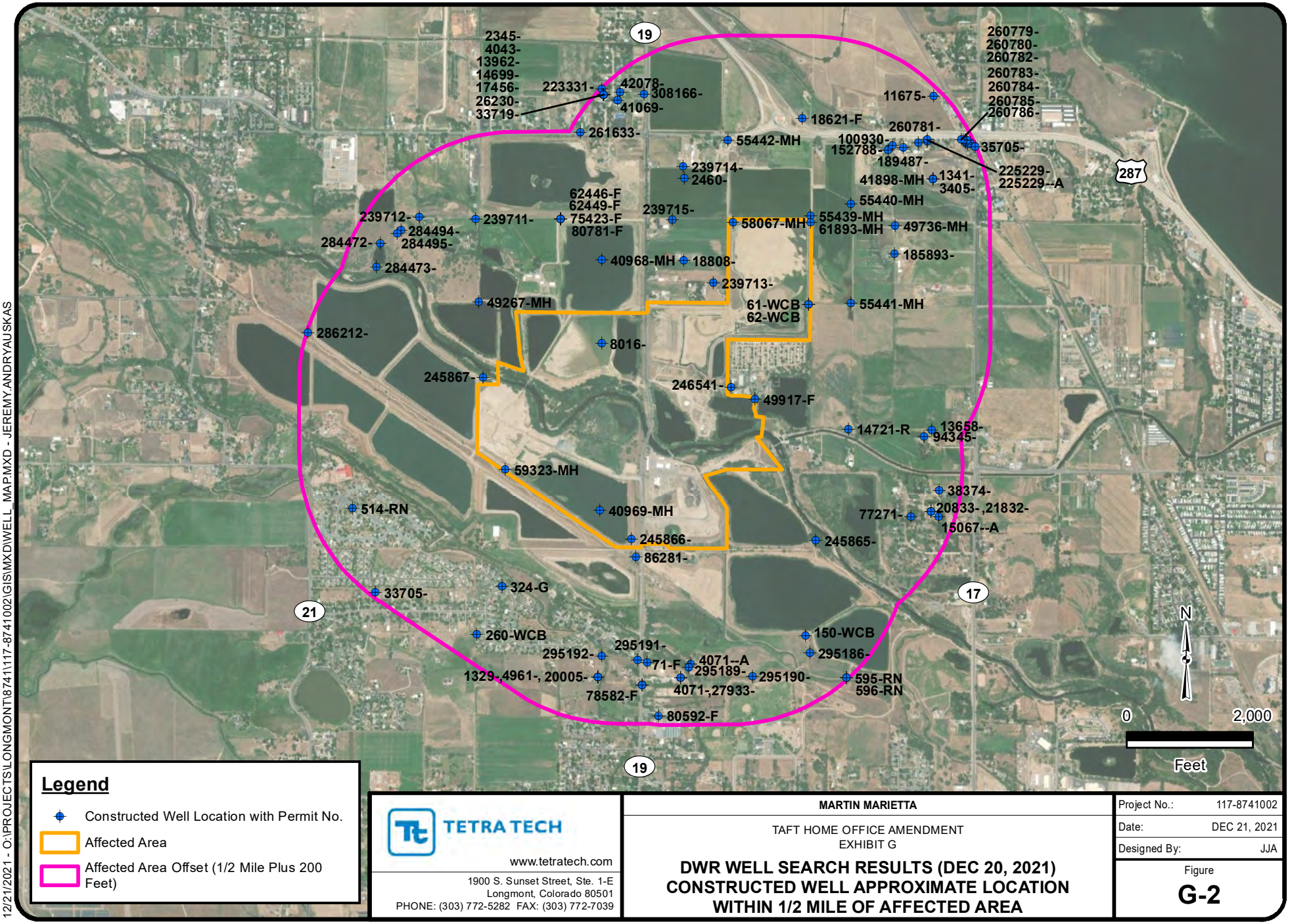
TAFT HOME OFFICE AMENDMENT
EXHIBIT G

GROUNDWATER FLOW MAP

Project No.:	117-8741002
Date:	JUN 06, 2022
Designed By:	JJA

Figure
G-1

6/6/2022 - O:\PROJECTS\LONGMONT\8741\117-8741002\GIS\MXD\GROUNDWATERMAP.MXD - JEREMY.ANDRYAUSKAS



Legend

- Constructed Well Location with Permit No.
- Affected Area
- Affected Area Offset (1/2 Mile Plus 200 Feet)

TETRA TECH
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MARTIN MARIETTA

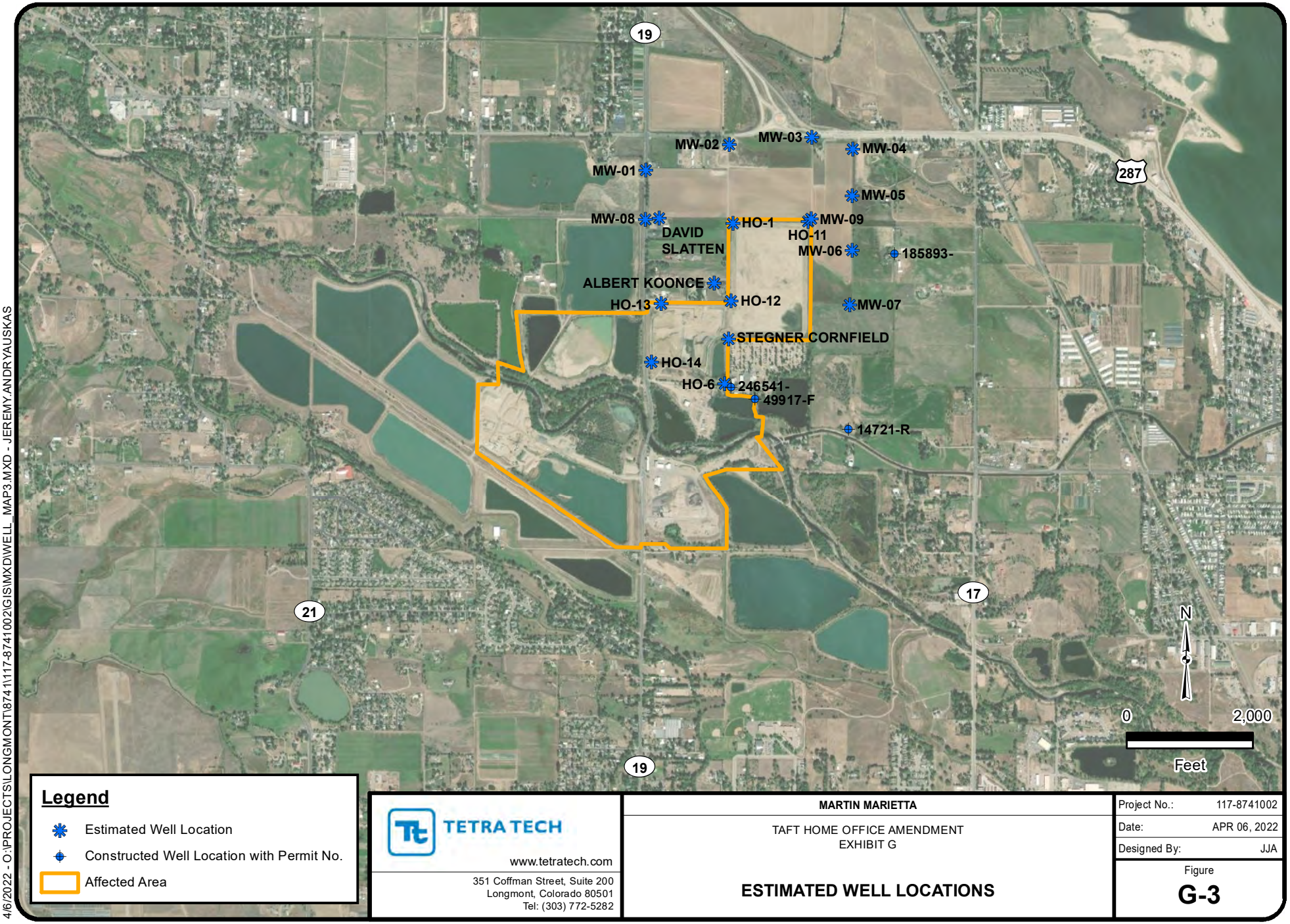
TAFT HOME OFFICE AMENDMENT
EXHIBIT G

DWR WELL SEARCH RESULTS (DEC 20, 2021)
CONSTRUCTED WELL APPROXIMATE LOCATION
WITHIN 1/2 MILE OF AFFECTED AREA

Project No.:	117-8741002
Date:	DEC 21, 2021
Designed By:	JJA

Figure
G-2

12/21/2021 - O:\PROJECTS\LONGMONT\8741\117-8741002\GIS\MXD\WELL_MAP.MXD - JEREMY.ANDRYAUSKAS



4/6/2022 - O:\PROJECTS\LONGMONT\874 1\117-8741002\GIS\MXD\WELL_MAP3.MXD - JEREMY.ANDRYAUSKAS

Legend

- Estimated Well Location
- Constructed Well Location with Permit No.
- Affected Area

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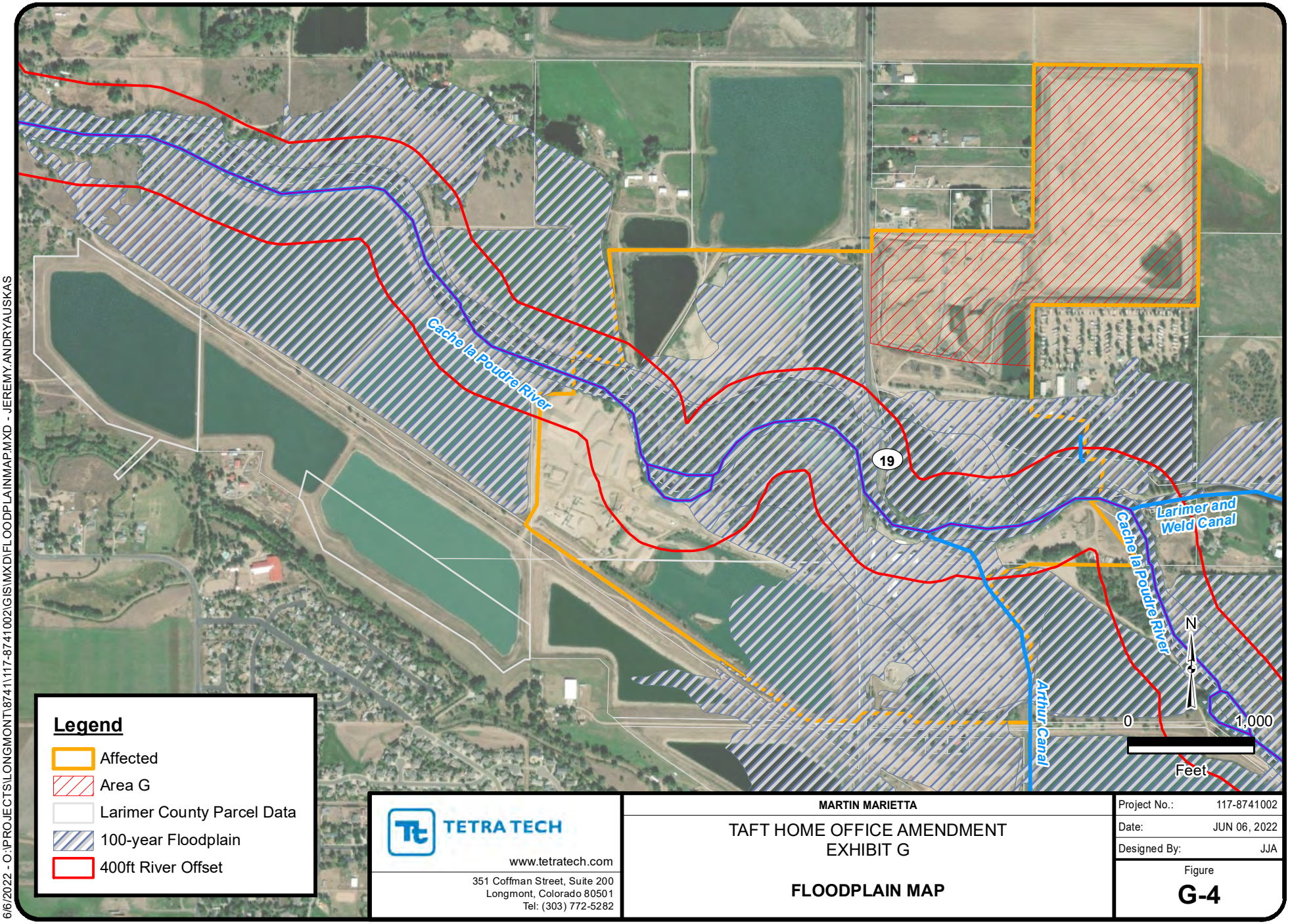
MARTIN MARIETTA

TAFT HOME OFFICE AMENDMENT
EXHIBIT G

ESTIMATED WELL LOCATIONS

Project No.:	117-8741002
Date:	APR 06, 2022
Designed By:	JJA

Figure
G-3



6/6/2022 - O:\PROJECTS\LONGMONT\8741\117-8741002\GIS\MXD\FLOODPLAINMAP.MXD - JEREMY.ANDRYAUSKAS

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MARTIN MARIETTA

TAFT HOME OFFICE AMENDMENT
EXHIBIT G

FLOODPLAIN MAP

Project No.:	117-8741002
Date:	JUN 06, 2022
Designed By:	JJA

Figure
G-4

ATTACHMENT 1
RAW MONITORING WELL DATA

HOME OFFICE MONITORING WELLS (COMBINED MEASUREMENTS PROVIDED BY DEERE & AULT AND MARTIN MARIETTA)																		
Monitoring Well Name	HO-1			HO-6			HO-11			HO-12			HO-13			HO-14*		
Well Location	1469960, 3109282			1467379, 3109131			1469954, 3110451			1468699, 3109239			1468646, 3108123			1467725, 3107980		
Top of PVC Casing Elevation (ft.)	*5024.08*			*5019.39*			*5021.36*			*5023.53*			*5026.66*			*5023.84*		
Ground Elevation of Well (ft.)	*5021.21*			*5016.45*			*5018.43*			*5021.17*			*5023.74*			*5021.58*		
PCV Stickup (approx.)	*2.87*			*2.94*			*2.93*			*2.36*			*2.92*			*2.20*		
Bottom Elevation of Well (ft.)	4996.21			4987.45			5004.43			5004.17			5008.74			5004.53		
Estimated Bedrock Elevation (ft)	5006.71			5000.45			5002.43			5005.17			5006.74			5005.58		
Total Well Depth Ground-Bottom (ft.)	25.00			29.00			14.00			17.00			15.00			17.00		
Date	Water Depth from Top of PVC (ft.)	Saturated Thickness	Ground Water Elevation (ft.)	Water Depth from Top of PVC (ft.)	Saturated Thickness	Ground Water Elevation (ft.)	Water Depth from Top of PVC (ft.)	Saturated Thickness	Ground Water Elevation (ft.)	Water Depth from Top of PVC (ft.)	Saturated Thickness	Ground Water Elevation (ft.)	Water Depth from Top of PVC (ft.)	Saturated Thickness	Ground Water Elevation (ft.)	Water Depth from Top of PVC (ft.)	Saturated Thickness	Ground Water Elevation (ft.)
Monday, May 7, 2018	11.08	6.29	5013.00	9.17	9.77	5010.22												
Friday, June 1, 2018	10.82	6.55	5013.26	9.33	9.61	5010.06												
Tuesday, November 13, 2018	11.15	6.22	5012.93	11.74	7.20	5007.65												
Sunday, December 30, 2018	10.78	6.59	5013.30	12.03	6.91	5007.36												
Sunday, February 3, 2019	11.12	6.25	5012.96	11.90	7.04	5007.49												
Sunday, March 10, 2019	11.35	6.02	5012.73	11.79	7.15	5007.60												
Monday, April 1, 2019	11.35	6.02	5012.73	11.67	7.27	5007.72												
Saturday, May 25, 2019	11.31	6.06	5012.77	11.21	7.73	5008.18												
Thursday, June 13, 2019	11.12	6.25	5012.96	11.29	7.65	5008.10												
Friday, September 6, 2019	10.20	7.17	5013.88	11.56	7.38	5007.83												
Monday, October 7, 2019				11.53	7.41	5007.86												
Saturday, November 16, 2019	11.45	5.92	5012.63	11.98	6.96	5007.41												
Saturday, January 25, 2020	11.60	5.77	5012.48	11.99	6.95	5007.40												
Friday, August 14, 2020	9.96	7.41	5014.12	9.42	9.52	5009.97												
Friday, August 21, 2020	7.71	9.66	5016.37	9.98	8.96	5009.41												
Friday, August 28, 2020	5.91	11.46	5018.17	10.34	8.60	5009.05												
Friday, September 4, 2020	5.70	11.67	5018.38	10.79	8.15	5008.60												
Friday, September 11, 2020	5.63	11.74	5018.45	10.18	8.76	5009.21												
Friday, September 18, 2020	5.64	11.73	5018.44	9.36	9.58	5010.03												
Friday, September 25, 2020	5.67	11.70	5018.41	8.95	9.99	5010.44												
Friday, October 2, 2020	5.70	11.67	5018.38	7.69	11.25	5011.70												
Friday, October 16, 2020	5.73	11.64	5018.35	9.66	9.28	5009.73												
Friday, October 23, 2020	5.79	11.58	5018.29	10.22	8.72	5009.17												
Friday, October 30, 2020	5.68	11.69	5018.40	10.55	8.39	5008.84												
Friday, November 6, 2020	5.61	11.76	5018.47	10.83	8.11	5008.56												
Friday, November 13, 2020	5.61	11.76	5018.47	11.10	7.84	5008.29												
Monday, November 23, 2020	5.60	11.77	5018.48	11.37	7.57	5008.02												
Monday, November 30, 2020	5.64	11.73	5018.44	11.51	7.43	5007.88												
Monday, December 7, 2020	5.73	11.64	5018.35	11.66	7.28	5007.73												
Tuesday, December 15, 2020	5.73	11.64	5018.35	11.80	7.14	5007.59												
Tuesday, December 22, 2020	5.62	11.75	5018.46	11.90	7.04	5007.49												
Tuesday, December 29, 2020	5.73	11.64	5018.35	11.70	7.24	5007.69												
Thursday, January 7, 2021	5.79	11.58	5018.29	11.45	7.49	5007.94												
Thursday, January 14, 2021	5.78	11.59	5018.30	11.63	7.31	5007.76												
Monday, May 24, 2021										11.93	6.43	5011.60	11.85	8.07	5014.81	12.30	5.86	5011.43
Wednesday, May 26, 2021	5.3	12.07	5018.78	6.98	11.96	5012.41	6.3	12.63	5015.06	10.90	7.46	5012.63	11.83	8.09	5014.83	12.40	5.76	5011.33
Thursday, June 3, 2021							6.45	12.48	5014.91	10.91	7.45	5012.62	12.10	7.82	5014.56	12.79	5.36	5010.94
Wednesday, June 16, 2021	3.04	14.33	5021.04	8.95	9.99	5010.44	6.28	12.65	5015.08	11.05	7.31	5012.48	11.62	8.30	5015.04	12.84	5.32	5010.89
Wednesday, June 30, 2021	4.80	12.57	5019.28	8.00	10.94	5011.39	5.87	13.06	5015.49	11.10	7.26	5012.43				9.30	8.85	5014.43
Thursday, July 8, 2021	3.17	14.20	5020.91	7.51	11.43	5011.88	4.76	14.17	5016.60	11.22	7.14	5012.31	12.30	7.62	5014.36	13.58	4.57	5010.15
Thursday, July 22, 2021	2.82	14.55	5021.26	6.73	12.21	5012.66	5.54	13.39	5015.82	11.60	6.76	5011.93	12.26	7.66	5014.40	14.08	4.07	5009.65
Tuesday, July 27, 2021	4.35	13.02	5019.73	8.21	10.73	5011.18	6.21	12.72	5015.15	11.2	7.16	5012.33	12.9	7.02	5013.76	8.97	9.29	5014.87
Wednesday, August 25, 2021																14.63	3.63	5009.21
Thursday, August 26, 2021	4.22	13.15	5019.86	8.15	10.79	5011.24	6.37	12.56	5014.99	11.54	6.82	5011.99	13	6.92	5013.66			
Thursday, September 30, 2021	5.83	11.54	5018.25	8	10.94	5011.39	7.5	11.43	5013.86	11.83	6.53	5011.70	13.15	6.77	5013.51	14	4.26	5009.84
Friday, October 22, 2021	6.01	11.36	5018.07	8.2	10.74	5011.19	8.7	10.23	5012.66				13.57	6.35	5013.09	14.4	3.86	5009.44
Saturday, October 23, 2021										11.9	6.46	5011.63						
Tuesday, November 30, 2021	6.1	11.27	5017.98	11.2	7.74	5008.19	8.7	10.23	5012.66	11.8	6.56	5011.73	13.74	6.18	5012.92	15.02	3.24	5008.82

HOME OFFICE MONITORING WELLS (COMBINED MEASUREMENTS PROVIDED BY DEERE & AULT AND MARTIN MARIETTA)																		
Monitoring Well Name	HO-1			HO-6			HO-11			HO-12			HO-13			HO-14*		
Well Location	1469960, 3109282			1467379, 3109131			1469954, 3110451			1468699, 3109239			1468646, 3108123			1467725, 3107980		
Top of PVC Casing Elevation (ft.)	*5024.08*			*5019.39*			*5021.36*			*5023.53*			*5026.66*			*5023.84*		
Ground Elevation of Well (ft.)	*5021.21*			*5016.45*			*5018.43*			*5021.17*			*5023.74*			*5021.58*		
PCV Stickup (approx.)	*2.87*			*2.94*			*2.93*			*2.36*			*2.92*			*2.20*		
Bottom Elevation of Well (ft.)	4996.21			4987.45			5004.43			5004.17			5008.74			5004.53		
Estimated Bedrock Elevation (ft)	5006.71			5000.45			5002.43			5005.17			5006.74			5005.58		
Total Well Depth Ground-Bottom (ft.)	25.00			29.00			14.00			17.00			15.00			17.00		
Date	Water Depth from Top of PVC (ft.)	Saturated Thickness	Ground Water Elevation (ft.)	Water Depth from Top of PVC (ft.)	Saturated Thickness	Ground Water Elevation (ft.)	Water Depth from Top of PVC (ft.)	Saturated Thickness	Ground Water Elevation (ft.)	Water Depth from Top of PVC (ft.)	Saturated Thickness	Ground Water Elevation (ft.)	Water Depth from Top of PVC (ft.)	Saturated Thickness	Ground Water Elevation (ft.)	Water Depth from Top of PVC (ft.)	Saturated Thickness	Ground Water Elevation (ft.)
Tuesday, December 14, 2021							8.9	10.03	5012.46	11.75	6.61	5011.78	12.75	7.17	5013.91	14.9	3.36	5008.94
Friday, December 17, 2021	6.21	11.16	5017.87	11	7.94	5008.39												
Friday, January 14, 2022	6.26	11.11	5017.82	11.18	7.76	5008.21	8.87	10.06	5012.49	11.8	6.56	5011.73	12.62	7.30	5014.04	15.1	3.16	5008.74
Friday, February 11, 2022	6.4	10.97	5017.68	11.84	7.10	5007.55	8.7	10.23	5012.66	11.1	7.26	5012.43	14.1	5.82	5012.56	14.58	3.68	5009.26
Wednesday, March 2, 2022	6.52	10.85	5017.56	11.93	7.01	5007.46	8.83	10.10	5012.53	11.33	7.03	5012.20	14.13	5.79	5012.53	14.67	3.59	5009.17
Monday, April 18, 2022	6.22	11.15	5017.86	11.10	7.84	5008.29	8.80	10.13	5012.56	11.10	7.26	5012.43	13.90	6.02	5012.76	14.53	3.73	5009.31
Friday, May 13, 2022	6.44	10.93	5017.64	7.20	11.74	5012.19	8.70	10.23	5012.66	11.23	7.13	5012.30	13.70	6.22	5012.96	14.59	3.67	5009.25
Tuesday, June 7, 2022	6.30	11.07	5017.78	6.50	12.44	5012.89	8.77	10.16	5012.59	11.30	7.06	5012.23	14.10	5.82	5012.56	14.45	3.81	5009.39
Wednesday, July 13, 2022	6.35	11.02	5017.73	5.60	13.34	5013.79	8.79	10.14	5012.57	11.20	7.16	5012.33	13.90	6.02	5012.76	14.30	3.96	5009.54
Tuesday, August 16, 2022	6.52	10.85	5017.56	7.61	11.33	5011.78	8.81	10.12	5012.55	11.15	7.21	5012.38	13.86	6.06	5012.80	14.01	4.25	5009.83
Saturday, September 10, 2022	6.12	11.25	5017.96	7.85	11.09	5011.54	9.02	9.91	5012.34	11.03	7.33	5012.50	13.31	6.61	5013.35	14.01	4.25	5009.83
Monday, October 10, 2022	6.31	11.06	5017.77	8.05	10.89	5011.34	8.87	10.06	5012.49	11.00	7.36	5012.53	12.92	7.00	5013.74	13.81	4.45	5010.03
Monday, November 14, 2022	6.51	10.86	5017.57	10.71	8.23	5008.68	9.21	9.72	5012.15	10.95	7.41	5012.58	11.53	8.39	5015.13	12.91	5.35	5010.93
Monday, December 5, 2022	7.15	10.22	5016.93	10.81	8.13	5008.58	8.90	10.03	5012.46	11.12	7.24	5012.41	11.55	8.37	5015.11	13.11	5.15	5010.73

Home Office Nearby Wells Water Level Monitoring									
LOCATION	ALBERT KOONCE			DAVID SLATTEN			STEGNER CORNFIELD/MM Near KOA		
LATTITUDE	40 37'08.6 N			40 37'18.8 N			40 36'59.8 N		
LONGTITUDE	105 06'37.4 W			105 06'48.6 W			105 06'34.5 W		
DESCRIPTION	2" CASE-BY NORTH FENCELINE			2"CASE-CENTER NORTH FENCE			PVC BY RV PARK		
ELEVATION OF BENCHMARK	5023.4			5028.0			5018.5		
ELEVATION OF GROUND SURFACE	5022.4			5027.0			5017.8		
ESTIMATED BEDROCK ELEVATION	5005.4			5010.0			5000.8		
DATE	**READING (Measurement from Benchmark to Water Level, ft)**	**WATER ELEV**	**SATURATED THICKNESS (ft)**	**READING (Measurement from Benchmark to Water Level, ft)**	**WATER ELEV**	**SATURATED THICKNESS (ft)**	**READING (Measurement from Benchmark to Water Level, ft)**	**WATER ELEV**	**SATURATED THICKNESS (ft)**
05/04/04	8.62	5014.82	9.4	7.88	5020.12	10.1			
06/10/04	8.15	5015.29	9.9	6.29	5021.71	11.7			
07/08/04	5.98	5017.46	12.0	3.45	5024.55	14.6			
08/03/04	5.75	5017.69	12.3	3.55	5024.45	14.4			
09/07/04	5.62	5017.82	12.4	5.91	5022.09	12.1			
10/04/04	5.55	5017.89	12.4	4.74	5023.26	13.3			
11/01/04	5.73	5017.71	12.3	4.77	5023.23	13.2			
12/06/04	6.04	5017.40	12.0	5.18	5022.82	12.8			
01/03/05	7.46	5015.98	10.5	7.36	5020.64	10.6			
02/07/05	7.53	5015.91	10.5	7.42	5020.58	10.6			
03/07/05	7.96	5015.48	10.0	7.77	5020.23	10.2			
04/04/05	8.31	5015.13	9.7	8.16	5019.84	9.8			
05/02/05	8.3	5015.14	9.7	8.22	5019.78	9.8			
06/07/05	7.33	5016.11	10.7	5.7	5022.3	12.3			
07/06/05	6.21	5017.23	11.8	4.62	5023.38	13.4			
08/10/05	6.33	5017.11	11.7	4.42	5023.58	13.6			
01/01/07									
02/01/07									
03/01/07									
04/02/07	7.62	5015.82	10.4	7.12	5020.88	10.9	8.6	5009.95	9.1
05/09/07	7.58	5015.86	10.4	7.1	5020.9	10.9	8.71	5009.84	9.0
06/01/07									
07/02/07	6.94	5016.50	11.1	4.89	5023.11	13.1	7.7	5010.85	10.1
08/06/07	6.87	5016.57	11.1	4.74	5023.26	13.3	8.21	5010.34	9.5
09/05/07	6.72	5016.72	11.3	4.92	5023.08	13.1	8.28	5010.27	9.5
10/08/07	6.84	5016.60	11.2	6.1	5021.9	11.9	8.41	5010.14	9.3
11/05/07	7.04	5016.40	11.0	6.48	5021.52	11.5	8.52	5010.03	9.2
12/03/07	6.92	5016.52	11.1	6.42	5021.58	11.6	8.5	5010.05	9.3
01/01/08									
02/01/08									
03/03/08	8.26	5015.18	9.7	8	5020	10.0	8.92	5009.63	8.8
04/07/08	7.85	5015.59	10.1	6.99	5021.01	11.0			
05/05/08	7.71	5015.73	10.3	6.82	5021.18	11.2			
06/01/08	7.23	5016.21	10.8	5.5	5022.5	12.5			
07/07/08	7.11	5016.33	10.9	4.99	5023.01	13.0			
08/11/08	6.98	5016.46	11.0	4.84	5023.16	13.2			
09/08/09	6.82	5016.62	11.2	4.92	5023.08	13.1			
10/01/08									
11/01/08									
12/01/08									
01/01/09									
02/03/09	8.1	5015.34	9.9	7.98	5020.02	10.0	8.82	5009.73	8.9
03/03/09	8.13	5015.31	9.9	7.98	5020.02	10.0	8.85	5009.70	8.9
04/07/09	8.62	5014.82	9.4	8.57	5019.43	9.4	9	5009.55	8.8
05/05/09	6.93	5016.51	11.1	7.48	5020.52	10.5	8.18	5010.37	9.6
06/02/09	4.88	5018.56	13.1	6.15	5021.85	11.9	6	5012.55	11.8
07/07/09	4.21	5019.23	13.8	5.94	5022.06	12.1	5.86	5012.69	11.9
08/12/09	5.67	5017.77	12.3	3.53	5024.47	14.5	5.6	5012.95	12.1
09/08/09	6.21	5017.23	11.8	4.58	5023.42	13.4	5.71	5012.84	12.0
10/06/09	6.05	5017.39	11.9	4.62	5023.38	13.4	6.21	5012.34	11.5
11/11/09	6.91	5016.53	11.1	5.1	5022.9	12.9	7.12	5011.43	10.6
12/01/09	6.89	5016.55	11.1	5.11	5022.89	12.9	7.19	5011.36	10.6
01/01/10	6.77	5016.67	11.2	5.18	5022.82	12.8	7.25	5011.30	10.5
2/1/2010	6.74	5016.70	11.3	5.24	5022.76	12.8	7.32	5011.23	10.4
3/1/2010	6.78	5016.66	11.2	5.26	5022.74	12.7	7.28	5011.27	10.5
4/1/2010	6.72	5016.72	11.3	5.11	5022.89	12.9	7.2	5011.35	10.6
5/1/2010	6.15	5017.29	11.9	4.59	5023.41	13.4	6.31	5012.24	11.4
6/1/2010	5	5018.44	13.0	4.05	5023.95	13.9	5.42	5013.13	12.3

Home Office Nearby Wells Water Level Monitoring									
LOCATION	ALBERT KOONCE			DAVID SLATTEN			STEGNER CORNFIELD/MM Near KOA		
LATTITUDE	40 37'08.6 N			40 37'18.8 N			40 36'59.8 N		
LONGTITUDE	105 06'37.4 W			105 06'48.6 W			105 06'34.5 W		
DESCRIPTION	2" CASE-BY NORTH FENCELINE			2"CASE-CENTER NORTH FENCE			PVC BY RV PARK		
ELEVATION OF BENCHMARK	5023.4			5028.0			5018.5		
ELEVATION OF GROUND SURFACE	5022.4			5027.0			5017.8		
ESTIMATED BEDROCK ELEVATION	5005.4			5010.0			5000.8		
DATE	**READING (Measurement from Benchmark to Water Level, ft)**	**WATER ELEV**	**SATURATED THICKNESS (ft)**	**READING (Measurement from Benchmark to Water Level, ft)**	**WATER ELEV**	**SATURATED THICKNESS (ft)**	**READING (Measurement from Benchmark to Water Level, ft)**	**WATER ELEV**	**SATURATED THICKNESS (ft)**
7/13/2010	3.99	5019.45	14.0	3.56	5024.44	14.4	5.44	5013.11	12.3
8/1/2010	4.12	5019.32	13.9	3.84	5024.16	14.2	5.88	5012.67	11.9
9/1/2010	4	5019.44	14.0	5.14	5022.86	12.9	6.54	5012.01	11.2
10/22/2010	7.03	5016.41	11.0	6.66	5021.34	11.3	7.86	5010.69	9.9
11/19/2010	7.1	5016.34	10.9	7.09	5020.91	10.9	7.93	5010.62	9.8
12/17/2010	7.18	5016.26	10.8	7.14	5020.86	10.9	8.03	5010.52	9.7
1/14/2011	7.21	5016.23	10.8	7.18	5020.82	10.8	8.1	5010.45	9.6
2/18/2011	7.48	5015.96	10.5	7.21	5020.79	10.8	8.12	5010.43	9.6
3/18/2011	7.61	5015.83	10.4	7.28	5020.72	10.7	9	5009.55	8.8
5/3/2011	8.76	5014.68	9.2	8.1	5019.9	9.9	9.1	5009.45	8.6
6/6/2011	5.1	5018.34	12.9	5	5023	13.0	5.4	5013.15	12.4
7/16/2011	4.99	5018.45	13.0	4.96	5023.04	13.0	5.42	5013.13	12.3
8/19/2011	5.12	5018.32	12.9	5.04	5022.96	13.0	5.45	5013.10	12.3
9/30/2011									
11/4/2011	6	5017.44	12.0	5.98	5022.02	12.0	5.58	5012.97	12.2
2/17/2012	6.25	5017.19	11.8	6.14	5021.86	11.9			
4/20/2012	8.17	5015.27	9.8	4.25	5023.75	13.8	8.8	5009.75	8.9
5/25/2012	8	5015.44	10.0	4.14	5023.86	13.9	8.42	5010.13	9.3
7/30/2012	7.21	5016.23	10.8	4	5024	14.0	7.93	5010.62	9.8
1/11/2013	7.25	5016.19	10.8	5.2	5022.8	12.8	8.22	5010.33	9.5
8/13/2013	6.34	5017.10	11.7	4.5	5023.5	13.5	6.55	5012.00	11.2
10/23/2013	5.9	5017.54	12.1	5.85	5022.15	12.1			
4/17/2014	6.5	5016.94	11.5	5.75	5022.25	12.3			
5/23/2014	7	5016.44	11.0	5.62	5022.38	12.4			
6/11/2014	7.5	5015.94	10.5	5.44	5022.56	12.6			
7/16/2014	7.38	5016.06	10.6	5.24	5022.76	12.8			
8/20/2014	7.21	5016.23	10.8	5	5023	13.0			
9/8/2014	6.5	5016.94	11.5	5.7	5022.3	12.3	8	5010.55	9.8
10/16/2014	7	5016.44	11.0	6.5	5021.5	11.5	7.1	5011.45	10.6
12/9/2014	8.1	5015.34	9.9	7.6	5020.4	10.4	9.5	5009.05	8.3
7/7/2015	5.26	5018.18	12.7	5.63	5022.37	12.4	7.5	5011.05	10.3
8/18/2015									
11/12/2015	10.5	5012.94	7.5	8.7	5019.3	9.3	8.2	5010.35	9.6
1/12/2016	12.5	5010.94	5.5	10.1	5017.9	7.9	9.5	5009.05	8.3
2/12/2016	10.7	5012.74	7.3	9.5	5018.5	8.5			
3/12/2016	10	5013.44	8.0	9.1	5018.9	8.9	9.2	5009.35	8.6
4/5/2016	9.97	5013.47	8.0	9	5019	9.0	9.1	5009.45	8.6
5/15/2016	9.4	5014.04	8.6	8.5	5019.5	9.5	9	5009.55	8.8
6/16/2016	8	5015.44	10.0	8	5020	10.0	8.9	5009.65	8.9
4/15/2017	6.5	5016.94	11.5	7	5021	11.0	13	5005.55	4.8
5/2/2017	6.68	5016.76	11.3	7.43	5020.57	10.6	13.1	5005.45	4.6
6/12/2017	7.12	5016.32	10.9	7.67	5020.33	10.3	13.08	5005.47	4.7
7/7/2017	7.05	5016.39	10.9	7.99	5020.01	10.0	12.99	5005.56	4.8
8/10/2017	7.55	5015.89	10.4	8.12	5019.88	9.9	13	5005.55	4.8
11/1/2017	8.8	5014.64	9.2	5.8	5022.2	12.2	11.2	5007.35	6.6
12/17/2017	8.78	5014.66	9.2	6	5022	12.0	11	5007.55	6.8
1/11/2018	7.21	5016.23	10.8	7.18	5020.82	10.8	8.1	5010.45	9.6
2/14/2018	7.48	5015.96	10.5	7.21	5020.79	10.8	8.12	5010.43	9.6
3/28/2018	7.61	5015.83	10.4	7.28	5020.72	10.7	9	5009.55	8.8
5/3/2018	8.76	5014.68	9.2	8.1	5019.9	9.9	9.1	5009.45	8.6
6/4/2018	5.1	5018.34	12.9	5	5023	13.0	5.4	5013.15	12.4
7/13/2018	3.99	5019.45	14.0	3.56	5024.44	14.4	5.44	5013.11	12.3
8/1/2018	4.12	5019.32	13.9	3.84	5024.16	14.2	5.88	5012.67	11.9
9/1/2018	4	5019.44	14.0	5.14	5022.86	12.9	6.54	5012.01	11.2
10/22/2018	7.03	5016.41	11.0	6.66	5021.34	11.3	7.86	5010.69	9.9
11/19/2018	7.1	5016.34	10.9	7.09	5020.91	10.9	7.93	5010.62	9.8
12/17/2018	7.18	5016.26	10.8	7.14	5020.86	10.9	8.03	5010.52	9.7
1/14/2019	7.21	5016.23	10.8	7.18	5020.82	10.8	8.1	5010.45	9.6
2/18/2019	7.48	5015.96	10.5	7.21	5020.79	10.8	8.12	5010.43	9.6
3/18/2019	7.61	5015.83	10.4	7.28	5020.72	10.7	9	5009.55	8.8
7/19/2019	7.05	5016.39	10.9	6.89	5021.11	11.1	8.6	5009.95	9.1
8/23/2020	7.22	5016.22	10.8	7.32	5020.68	10.7	7.87	5010.68	9.9

Home Office Nearby Wells Water Level Monitoring									
LOCATION	ALBERT KOONCE			DAVID SLATTEN			STEGNER CORNFIELD/MM Near KOA		
LATTITUDE	40 37'08.6 N			40 37'18.8 N			40 36'59.8 N		
LONGTITUDE	105 06'37.4 W			105 06'48.6 W			105 06'34.5 W		
DESCRIPTION	2" CASE-BY NORTH FENCELINE			2"CASE-CENTER NORTH FENCE			PVC BY RV PARK		
ELEVATION OF BENCHMARK	5023.4			5028.0			5018.5		
ELEVATION OF GROUND SURFACE	5022.4			5027.0			5017.8		
ESTIMATED BEDROCK ELEVATION	5005.4			5010.0			5000.8		
DATE	**READING (Measurement from Benchmark to Water Level, ft)**	**WATER ELEV**	**SATURATED THICKNESS (ft)**	**READING (Measurement from Benchmark to Water Level, ft)**	**WATER ELEV**	**SATURATED THICKNESS (ft)**	**READING (Measurement from Benchmark to Water Level, ft)**	**WATER ELEV**	**SATURATED THICKNESS (ft)**
10/13/2020	7.02	5016.42	11.0	7.12	5020.88	10.9	8.35	5010.20	9.4
11/19/2020	6.93	5016.51	11.1	7.42	5020.58	10.6	8.56	5009.99	9.2
1/22/2021	7.13	5016.31	10.9	6.74	5021.26	11.3	10.5	5008.05	7.3
2/17/2021	7.21	5016.23	10.8	6.85	5021.15	11.1	9.4	5009.15	8.4
4/5/2021	7.5	5015.94	10.5	6.86	5021.14	11.1	9.12	5009.43	8.6
5/26/2021	7.34	5016.10	10.7	6.55	5021.45	11.4	8.74	5009.81	9.0
6/30/2021	7.13	5016.31	10.9	6.19	5021.81	11.8	8.3	5010.25	9.4
7/27/2021	7.34	5016.10	10.7	5.93	5022.07	12.1	7.73	5010.82	10.0
8/27/2021	7.87	5015.57	10.1	6.37	5021.63	11.6	7.23	5011.32	10.5
9/30/2021	8.32	5015.12	9.7	7.4	5020.6	10.6	6.65	5011.90	11.1
10/23/2021	8.6	5014.84	9.4	7.93	5020.07	10.1	7.6	5010.95	10.1
11/30/2021	8.6	5014.84	9.4	8.2	5019.8	9.8	10.4	5008.15	7.4
12/17/2021	8.53	5014.91	9.5	7.97	5020.03	10.0	10.11	5008.44	7.6
1/26/2022	8.61	5014.83	9.4	7.85	5020.15	10.1	9.21	5009.34	8.5
2/11/2022	8.96	5014.48	9.0	7.6	5020.4	10.4	11.65	5006.90	6.1
3/2/2022	9	5014.44	9.0	7.52	5020.48	10.5	11.21	5007.34	6.5
4/11/2022	9.15	5014.29	8.9	8.5	5019.5	9.5	11.6	5006.95	6.1
5/1/2022	9.2	5014.24	8.8	8.2	5019.8	9.8	11.41	5007.14	6.3
6/1/2022	8.7	5014.74	9.3	7.7	5020.3	10.3	11.2	5007.35	6.6
7/18/2022	9	5014.44	9.0	7.5	5020.5	10.5	10.71	5007.84	7.0
8/10/2022	9.21	5014.23	8.8	7.61	5020.39	10.4	11.01	5007.54	6.7
9/2/2022	8.81	5014.63	9.2	7.88	5020.12	10.1	11.15	5007.40	6.6
10/6/2022	9.32	5014.12	8.7	7.82	5020.18	10.2	10.99	5007.56	6.8
11/11/2022	9.25	5014.19	8.8	8.01	5019.99	10.0	11.12	5007.43	6.6
12/5/2022	8.91	5014.53	9.1	8.52	5019.48	9.5	11.01	5007.54	6.7

Data Source Notes:
Coordinates provided by Martin Marietta, 2021-08-19
Elevations provided by Martin Marietta, 2021-08-19
Water level measurements provided by Martin Marietta
Ground Surface for Koonce and Slatten approximated as 1 foot below BM.
Bedrock depth estimated as 17 feet below ground surface

Stegner - Monitoring Well Measurements									
Depth To Water from Surface (Feet)									
Date Measured	**MW-01**	**MW-02**	**MW-03**	**MW-04**	**MW-05**	**MW-06**	**MW-07**	**MW-08**	**MW-09**
July-16	2.62	0.95	1.75	2.28	2.91	2.41	3.15	6.41	2.92
August-16	4.15	0.93	1.78	2.18	3.06	2.71	3.75	7.38	3.56
September-16	6.89	3.00	3.08	2.52	3.70	3.53	5.70	9.28	5.09
October-16	-0.29	3.18	3.40	2.81	3.73	3.43	4.56	9.99	5.27
November-16	7.01	3.39	3.78	3.10	3.95	3.41	5.43	10.23	5.37
December-16	7.21	3.57	3.98	3.22	3.99	3.45	5.66	10.56	5.45
January-17	7.18	3.58	4.05	3.27	4.00	3.42	5.64	10.63	5.42
February-17	7.19	3.70	4.15	4.47	4.07	3.53	6.19	10.73	5.57
March-17	7.16	3.28	3.72	3.18	4.18	3.53	6.41	10.62	5.62
April-17	5.56	2.61	3.38	3.03	4.05	3.55	6.51	8.95	5.53
May-17	1.76	0.97	2.13	2.67	3.24	2.79	5.50	3.11	3.41
June-17	0.44	0.50	1.55	1.50	2.38	2.10	5.57	1.51	2.20
July-17	1.75	0.53	1.96	2.45	3.47	3.11	5.69	2.18	3.64
August-17	2.60	1.66	2.98	2.73	4.00	3.72	6.17	3.07	6.25
September-17	2.31	1.35	2.73	3.43	4.04	3.80		3.64	6.44
October-17	2.62	1.61	3.85	3.76	4.30	4.50	6.67	4.26	7.81
November-17	2.72	2.11	4.37	3.67	4.79	4.24	6.41	5.23	8.42
December-17	5.63	2.88	4.22	3.45	4.97	4.80	7.07	7.18	8.57
April-18	6.69	3.50	4.41	3.57	5.16	4.89	7.09	8.93	8.74
July-18	2.67	0.90	2.03	2.09	3.11	2.65	5.55	4.71	6.49
October-18	6.94	4.24	4.93	3.33	7.02	6.66	8.09	3.87	8.98
January-19	6.14	4.09	5.15	3.63	5.21	4.99	7.28	7.85	8.84
April-19	6.98	4.39	5.37	3.86	5.45	5.01	7.19	9.31	8.94
July-19	5.84	2.81	4.48	3.20	5.13	3.94	7.10	6.63	8.76
September-19	6.32	4.75	5.59	3.64	4.93	4.01	6.89	6.58	8.39
January-20	7.43	5.24	5.59	4.07	5.52	5.17	7.35	9.59	8.97
May-20	7.03	4.53	5.16	3.79	5.39	5.06	7.15	9.24	8.78
August-20	4.59	2.68	3.12	3.46	4.69	3.91	5.79	4.16	7.18
December-20	6.45	3.23	4.41	3.75	4.94	4.50	6.85	5.78	7.73
February-21	6.69	5.02	5.38	4.28	5.46	4.79	7.38	8.07	8.56
May-21	6.94	4.71	5.29	3.90	5.32	4.89	7.25	9.04	8.74
August-21	4.58	2.88	3.31	3.53	4.68	3.82	6.01	4.06	7.28
December-21	6.51	3.77	4.63	3.44	5.11	4.67	7.10	6.86	8.46
March-22	6.96	4.29	5.28	3.78	5.74	5.10	7.49	7.61	8.98

Legend

- Stegner Monitoring Wells
- Stegner Parcels

0 400

SCALE IN FEET

N

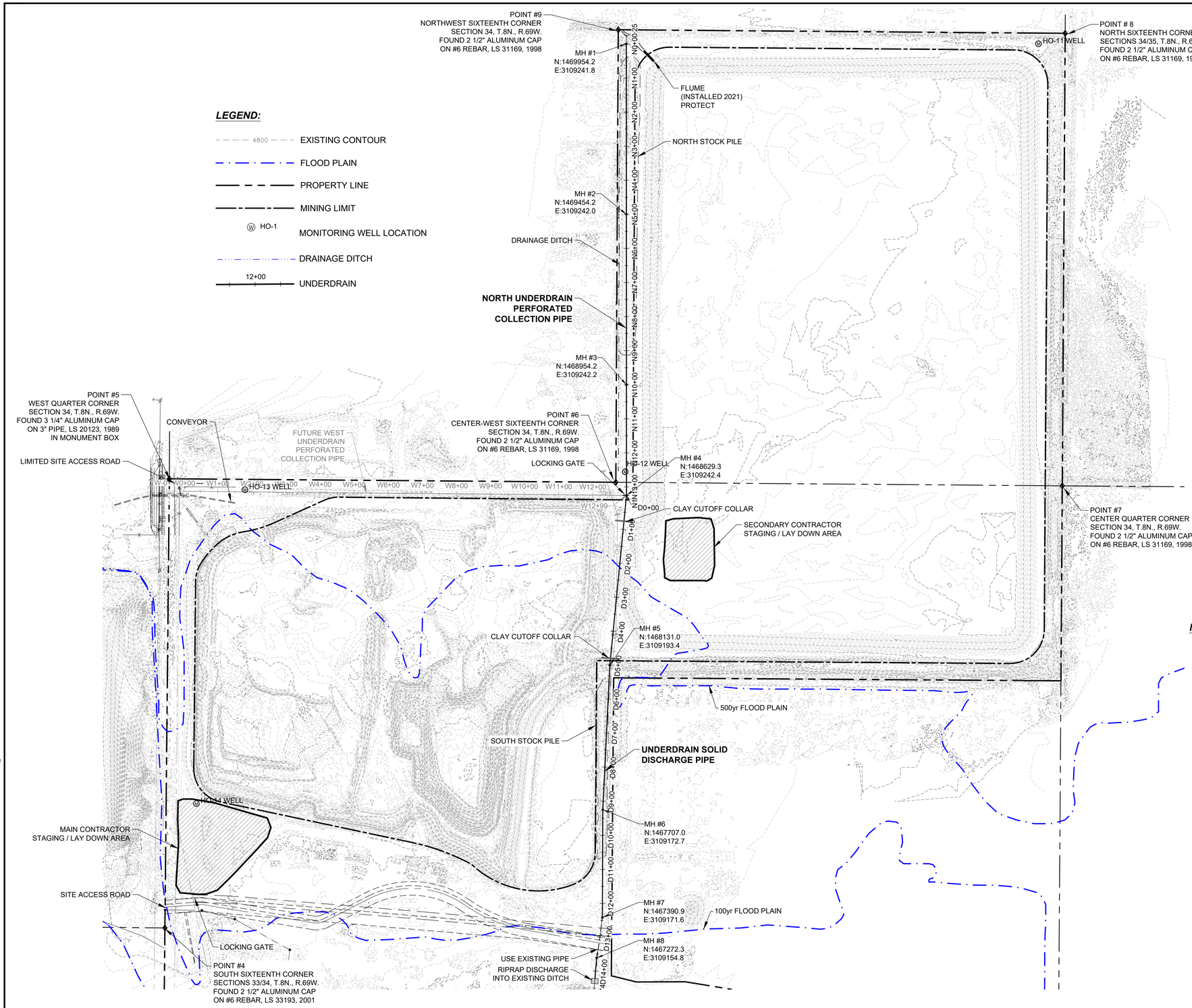
COORDINATE SYSTEM
NAD 83 COLORADO NORTH

STEGNER BASEMAP

TELESTO SOLUTIONS • INCORPORATED

LOVELAND READY-MIX CONCRETE

ATTACHMENT 2
UNDERDRAIN DESIGN



LEGEND:

- 4800 — EXISTING CONTOUR
- FLOOD PLAIN
- PROPERTY LINE
- MINING LIMIT
- HO-1 — MONITORING WELL LOCATION
- DRAINAGE DITCH
- 12+00 — UNDERDRAIN

0 150 300

SCALE IN FEET
CONTOUR INTERVAL=1'

NOTES:

1. ALL UTILITIES TO BE LOCATED, VERIFIED & POT HOLED AS NECESSARY BY CONTRACTOR.
2. TRAFFIC ON LIMITED SITE ACCESS ROAD TO BE COORDINATED WITH ADJACENT PROPERTY OWNERS & ONLY ALLOWED DURING STANDARD WORK HOURS.
3. SURVEY CONTROL PROVIDE BY KING SURVEYOR'S. COORDINATE VALUES ARE THAT OF THE COLORADO STATE PLANE COORDINATE SYSTEM, NORTH ZONE, NORTH AMERICAN DATUM 1983/92. TO CONVERT TO GROUND (MODIFIED) SCALE ABOUT POINT 0,0 AT A FACTOR OF 1.00026675 (0.99973332 CF) VERTICAL DATUM: NAVD 88.

EXISTING SURVEY POINT TABLE

POINT #	NORTHING	EASTING	DESCRIPTION
1	1466019.4650	3110502.4980	S. 1/4 COR, SEC
2	1466033.7797	3109189.4328	W. 1/16 COR, SEC34
3	1466042.3880	3107875.3510	SW. COR, SEC34
4	1467360.3495	3107888.3763	S. 1/16 COR, SEC 34
5	1468678.1520	3107901.4000	W. 1/4 COR, SEC 34
6	1468667.8170	3109210.9885	CENTER W. 1/16 COR, SEC 34
7	1468657.4820	3110520.5770	CENTER 1/4 COR, SEC 34
8	1469985.2060	3110529.0260	N. 1/16 COR, SEC 34
9	1469995.6070	3109218.1910	NW. 1/16 COR, SEC 34

NOTE: POINT # 1,2,3 ARE NOT SHOWN ON THIS PAGE. THEY ARE SOUTH OF THE SITE.

REV	DESCRIPTION	BY	DATE
1	60% DESIGN - NOT FOR CONSTRUCTION	WK	11-4-21
2	FOR CONSTRUCTION	SR	11-29-21

IRECK 11/29/2021 11:32:13 AM

DESIGNED BY: SAR
DRAWN BY: ITR
CHECKED BY: CAH

SUSAN A. RAINEY
STATE PROFESSIONAL ENGINEER NO. XXXXX
DATE:

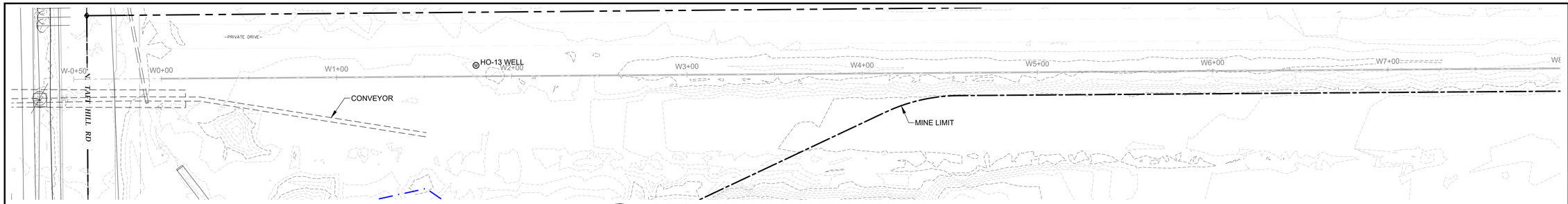
DEERE & AULT
A SCHNABEL ENGINEERING COMPANY
600 S. AIRPORT RD. BLDG. A, SUITE 205
LONGMONT, CO 80503
TEL 303.651.1468 FAX 303.651.1469

HOME OFFICE MINE
UNDERDRAIN CONSTRUCTION

GENERAL PLAN

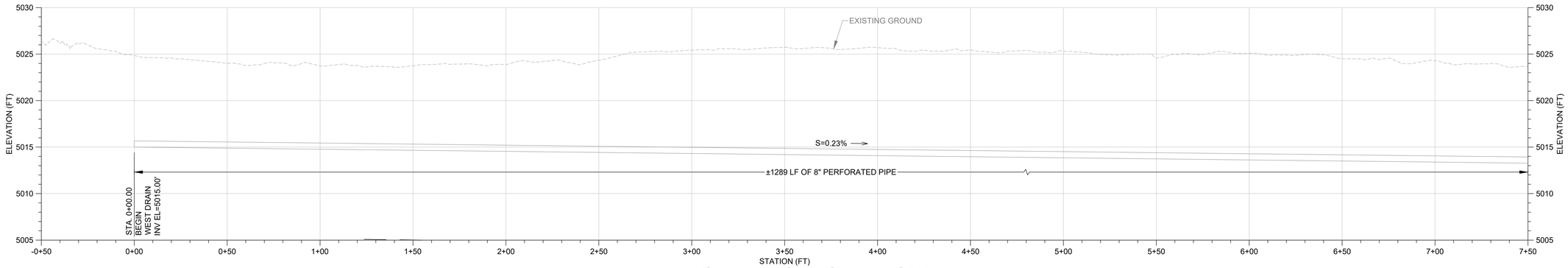
PROJECT: 0494.019.00
DATE: 11/29/21
SHEET 5 OF 9

U:\0494 MARTIN MARIETTA\0494.019 HOME OFFICE MINE\CAD\WORKING\UNDERDRAIN\HO UNDERDRAIN_PIPE PROFILES.DWG



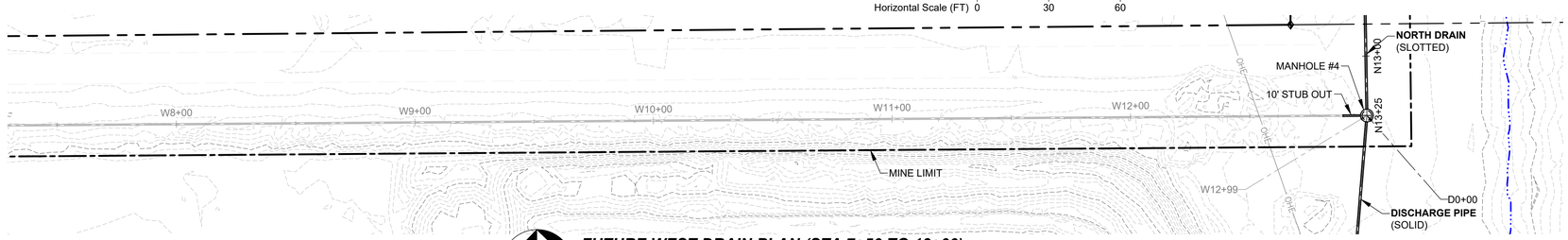
FUTURE WEST DRAIN PLAN (STA 0+00 TO 7+50)

0 30 60
SCALE IN FEET



FUTURE WEST DRAIN PROFILE (STA 0+00 TO 7+50)

Vertical Scale (FT) 0 6 12
Horizontal Scale (FT) 0 30 60



FUTURE WEST DRAIN PLAN (STA 7+50 TO 13+00)

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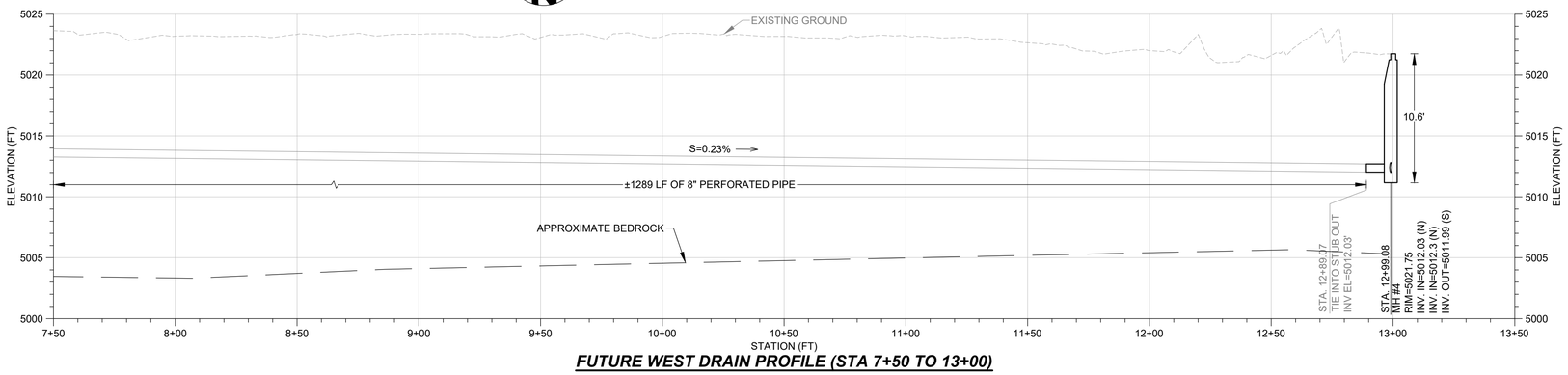
SCALE IN FEET



FUTURE WEST DRAIN PROFILE (STA 7+50 TO 13+00)

Vertical Scale (FT) 0 6 12
Horizontal Scale (FT) 0 30 60

REV	DESCRIPTION	BY	DATE
1	60% DESIGN - NOT FOR CONSTRUCTION	WK	11-4-21
2	FOR CONSTRUCTION	SR	11-29-21

DESIGNED BY: SAR
DRAWN BY: ITR
CHECKED BY: CAH

SUSAN A. RAINEY
DATE:
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HOME OFFICE MINE
UNDERDRAIN CONSTRUCTION

WEST DRAIN PLAN AND PROFILE

PROJECT: 0494.019.00
DATE: 11/29/21
SHEET 6 OF 9

U:\0494 MARTIN MARIETTA\0494.019 HOME OFFICE MINE\CAD\WORKING\UNDERDRAIN\HO UNDERDRAIN_PIPE PROFILES.DWG

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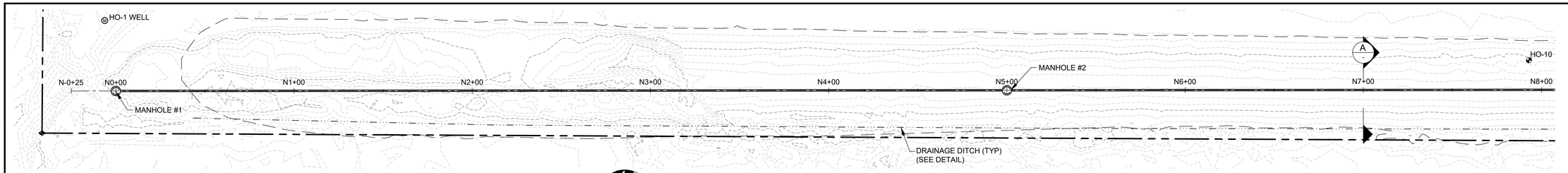

NORTH DRAIN PLAN (STA 0+00 TO 8+00)

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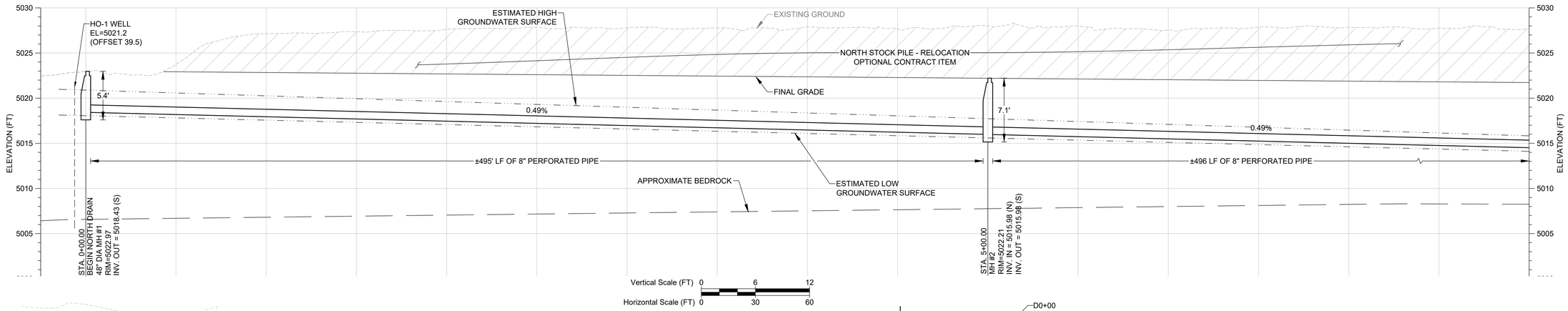
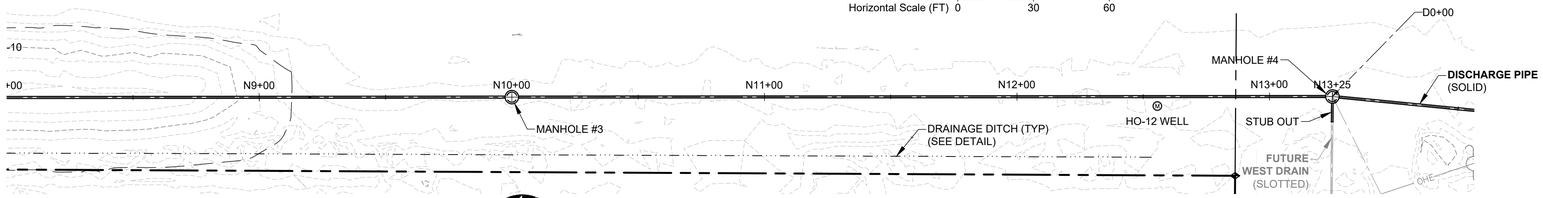


Vertical Scale (FT) 0 6 12

Horizontal Scale (FT) 0 30 60



NORTH DRAIN PLAN (STA 8+00 TO 14+00)

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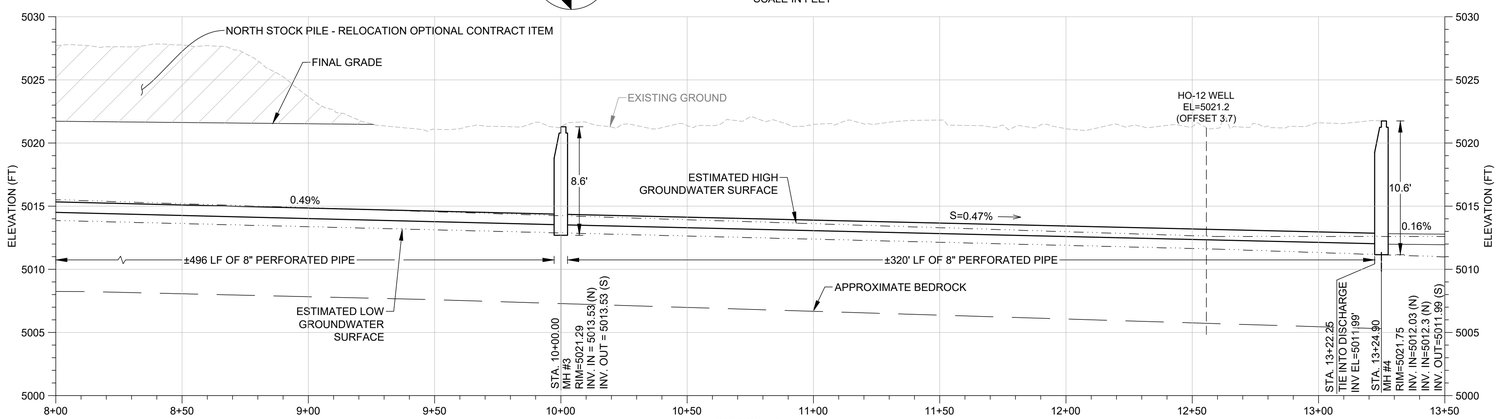

NORTH DRAIN PROFILE (STA 8+00 TO 13+50)

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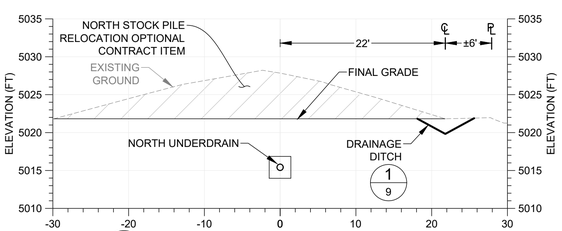
Horizontal Scale (FT) 0 30 60



DITCH SECTION STATION 7+00

SCALE IN FEET 0 10 20

REV	DESCRIPTION	BY	DATE
1	60% DESIGN - NOT FOR CONSTRUCTION	WK	11-4-21
2	FOR CONSTRUCTION	SR	11-29-21

IRECK 11/29/2021 11:32:13 AM

DESIGNED BY: SAR
DRAWN BY: ITR
CHECKED BY: CAH

SUSAN A. RAINEY
DATE:
STATE PROFESSIONAL ENGINEER NO. XXXXX

DEERE & AULT
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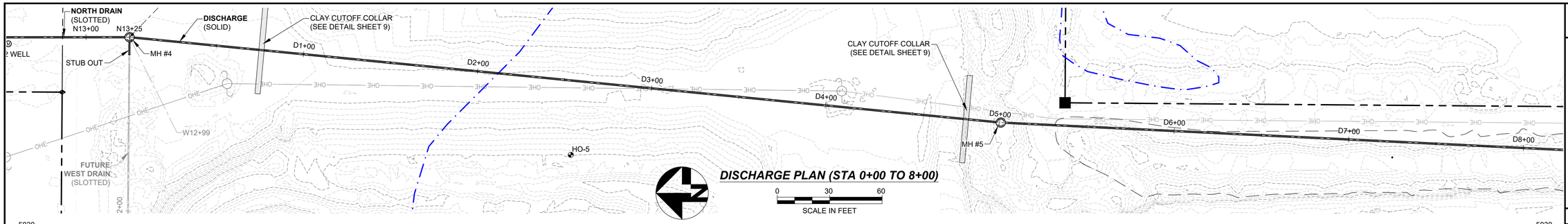
HOME OFFICE MINE
UNDERDRAIN CONSTRUCTION

NORTH DRAIN PLAN AND PROFILE

PROJECT: 0494.019.00
DATE: 11/29/21
SHEET 7 OF 9

U:\0494 MARTIN MARIETTA\0494.019 HOME OFFICE MINE\CAD\WORKING\UNDERDRAIN\HO UNDERDRAIN_PIPE PROFILES.DWG



DISCHARGE PLAN (STA 0+00 TO 8+00)

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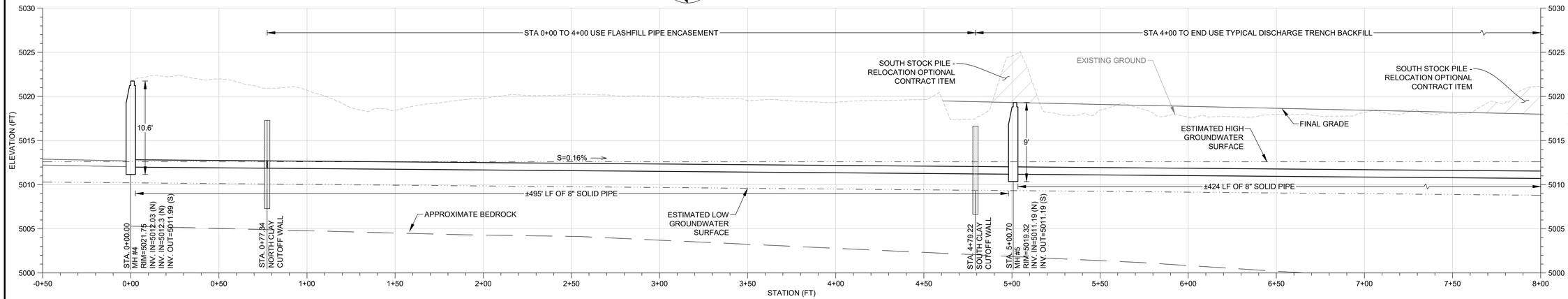
SCALE IN FEET



DISCHARGE PROFILE (STA 0+00 TO 8+00)

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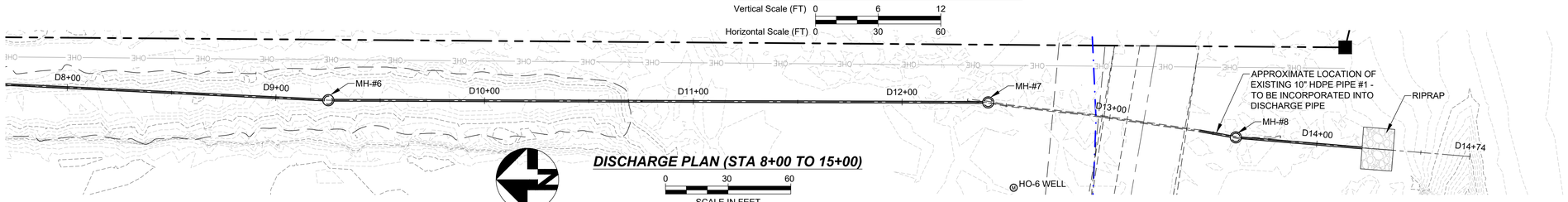
Horizontal Scale (FT) 0 30 60



DISCHARGE PLAN (STA 8+00 TO 15+00)

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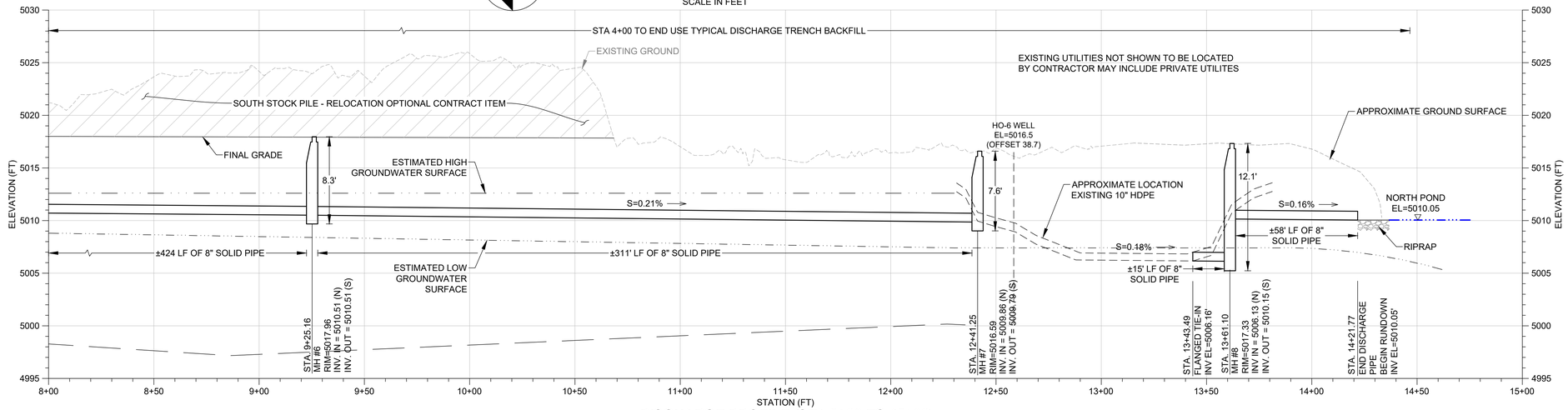
SCALE IN FEET



DISCHARGE PROFILE (STA 8+00 TO 15+00)

Vertical Scale (FT) 0 6 12
Horizontal Scale (FT) 0 30 60

REV	DESCRIPTION	BY	DATE
1	60% DESIGN - NOT FOR CONSTRUCTION	WK	11-4-21
2	FOR CONSTRUCTION	SR	11-29-21

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DESIGNED BY: SAR
DRAWN BY: ITR
CHECKED BY: CJH

SUSAN A. RAINEY
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DATE:

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HOME OFFICE MINE
UNDERDRAIN CONSTRUCTION

DISCHARGE PIPE PLAN AND PROFILE

PROJECT: 0494.019.00
DATE: 11/29/21
SHEET 8 OF 9

U:\0494 MARTIN MARIETTA\0494.019 HOME OFFICE MINE\CAD\WORKING\UNDERDRAIN\HO UNDERDRAIN\HO UNDERDRAIN_PIPE PROFILES.DWG

DEERE & AULT
A SCHNABEL ENGINEERING COMPANY

600 South Airport Road, Suite A-205
Longmont, Colorado 80503
T/ 303-651-1468
F/ 303-651-1469

TECHNICAL MEMORANDUM

TO:	Julie Mikulas Britney Guggisberg	**DATE:**	6/21/2022
COMPANY:	Martin Marietta	**SUBJECT:**	Underdrain Calculations
ADDRESS:	1800 North Taft Hill Road Fort Collins, CO 80521	**PROJECT NAME/NO.:**	Home Office DA494019.00
FROM:	Susan A. Rainey, PE	**CC:**	Pam Hora – Tetra Tech

This document presents the calculations performed as part of the groundwater underdrain design at the Home Office Mine.

PROJECT BACKGROUND

The Home Office Mine is located in Larimer County, Colorado in the northwest portion of the city of Fort Collins. As mining progressed at the site, Martin Marietta plans to construct a slope liner in stages around the perimeter of the mined area. The slope liner and berm in the center of the site will create two cells. Phase I, or the first cell and dividing berm, was constructed from August to October in 2020. Phase II is scheduled to be constructed in approximately the same time frame this year. Martin Marietta elected to construct a groundwater collection system or underdrain to deal with possible groundwater mounding that could occur due to the liner configuration. The first section of underdrain (the north drain and discharge) was constructed at the beginning of 2022. The remainder of the underdrain (the west drain) will be constructed as part of Phase II.

CALCULATIONS

Seepage analyses were performed using Seep/W, a finite element computer model software program, to estimate the possible groundwater flow into the underdrain. The seepage analyses were performed using two different K values for the native sand and gravel. The first value (2.54×10^{-3} cm/s) was selected using the NAVFAC DM 7.2 Table 1 typical coefficient of permeability for SW (well-graded sand) soil type as a guideline. This table lists a permeability of greater than 1×10^{-3} ft/min. We selected 5×10^{-3} ft/min or 2.54×10^{-3} cm/s for a possible lower end value, which is greater than the minimum (1×10^{-3} ft/min) typical permeability for well graded sand. This value was selected due to the presence of gravel and cobble on the site. A second seepage analysis was performed with a considerably higher permeability (1×10^{-2} cm/s) for a possible higher end value. This value was selected based off the typical permeability on the NAVFAC DM 7.2 Table 1 for GW (well-graded gravel) of 5×10^{-2} ft/min. Our value of 1.00×10^{-2} cm/s or 1.97×10^{-2} ft/min is less than this maximum permeability.

We performed analyses for each K value with two different groundwater heights for a range of possible flows into the underdrain system. The resulting flows were entered into FlowMaster, a general purpose 1D computational fluid dynamics simulation software, along with other design parameters (pipe size, slope, length, etc.) to calculate how full the pipe would be. An additional flow of 100 gallons per minute or more than double the highest Seep/W flow rate was entered into FlowMaster, as a high-end extreme flow. This was done for the two sections of perforated pipe (north and west drain). A cumulative flow of 200 gallons per minute for the discharge section was

also entered into FlowMaster. The resulting percent of pipe full of flow is shown on the attached calculations summary table.

A final check of water velocity through the slots in the perforated pipe was performed for the anticipated flows to confirm the open area of the perforated pipe was sufficient for the estimated possible, and high-end extreme flows.

Home Office Underdrain
Seep W Analysis
Drain pipe sizing.

North Drain

Inputs

Length:	1370	ft. - from drawings	
Slope	0.0047	ft./ft.	
Pipe Diame	8	in	
Manning's	0.0009		
Slots	1.5	in2/ft	specified minimum slot area

Outputs

Seep W Input						Seep W Results				Pipe	
Sand and Gravel (k)	Model Ground Surface	Model Groundwater Elevation	Groundwater depth below ground	Drain Elevation	Drain Depth	q	Total flow	Total flow	Flow height - solved in Flowmaster	% of diameter	Water velocity through slots
cm/sec	ft.	ft.	ft.	ft.	ft.	cfs/ft.	cfs	gpm	in		ft/sec
2.54E-03	5,024	5,019	5	5017	7	8.77E-06	1.20E-02	5.392287	0.6	7.1%	8.42E-04
2.54E-03	5,024	5,021	3	5017	7	1.85E-05	2.53E-02	11.37484	0.8	10.1%	1.78E-03
1.00E-02	5,024	5,019	5	5017	7	3.50E-05	4.80E-02	21.51996	1.1	13.7%	3.36E-03
1.00E-02	5,024	5,021	3	5017	7	7.39E-05	1.01E-01	45.43786	1.6	19.6%	7.09E-03
High flow check - 100 gpm, greater than 200% of model						1.63E-04	2.23E-01	100	2.3	29.3	1.56E-02

West Drain

Inputs

Length:	1326	ft. - from drawings	
Slope	0.0023	ft./ft.	
Pipe Diame	8	in	
Manning's	0.0009		
Slots	1.5	in2/ft	specified minimum slot area

Outputs

Seep W Input						Seep W Results				Pipe	
Sand and Gravel (k)	Model Ground Surface	Model Groundwater Elevation	Groundwater depth below ground	Drain Elevation	Drain Depth	q	Total flow	Total flow	Flow height - solved in Flowmaster	% of diameter	Water velocity through slots
cm/sec	ft.	ft.	ft.	ft.	ft.	cfs/ft.	cfs	gpm	in		ft/sec
2.54E-03	5,024	5,019	5	5017	7	8.77E-06	1.16E-02	5.219104	0.7	8.2%	8.42E-04
2.54E-03	5,024	5,021	3	5017	7	1.85E-05	2.45E-02	11.00951	0.9	11.8%	1.78E-03
1.00E-02	5,024	5,019	5	5017	7	3.50E-05	4.64E-02	20.82881	1.3	16.0%	3.36E-03
1.00E-02	5,024	5,021	3	5017	7	7.39E-05	9.80E-02	43.97854	1.8	23.1%	7.09E-03
High flow check - 100 gpm, greater than 200% of model						1.68E-04	2.23E-01	100	2.8	35.3%	1.61E-02

Discharge

Length:	1400	ft. - from drawings
Slope	0.0012	ft./ft.
Pipe Diame	8	in
Manning's	0.0009	

	Seep W Input					Seep W Results				Pipe
	Sand and Gravel (k)	Model Ground Surface	Model Groundwater Elevation	Groundwater depth below ground	Drain Elevation	Drain Depth	q	Total flow	Total flow	Flow height - solved in Flowmaster	% of diameter
	cm/sec	ft.	ft.	ft.	ft.	ft.	cfs/ft.	cfs	gpm	in	
	2.54E-03	5,024	5,019	5	5017	7		2.36E-02	10.61139	1.1	13.5%
	2.54E-03	5,024	5,021	3	5017	7		4.99E-02	22.38435	1.6	19.4%
	1.00E-02	5,024	5,019	5	5017	7		9.44E-02	42.34877	2.1	26.7%
	1.00E-02	5,024	5,021	3	5017	7		1.99E-01	89.4164	3.2	39.5%
	High flow check - 200 gpm, greater than 200% of model							4.46E-01	200	5.1	63.9%

Notes:

See USBR DS-15(5) - Filter Drsign. Paragraph 5.5.2 - "Drains sould be sized so that the depth of water in the drain pipe is less than 50% of the inside diameter.

Slot velocity not specified by design standard. Calculated to check, maintin below 0.06 ft/sec, which the maximum orfice velocity at 100 gpm as calculated in Flow Master

Schnabel ENGINEERING

CALCULATION COVER SHEET

Project	**Project Number**
Home Office Mine Underdrain	DA0494.019

Title

Toe drain filter selection and pipe selection

Computer Programs Used	**Version/Release No.**
Excel Flow Master Geostudio - SeepW	

Purpose and Objective

Select toe drain filter that is compatible with native soils at the depth of the toe drain. Using USBR Deign Standard No. 13, Embankment Dams, Chapter 5: Protective Filters
Select toe drain pipe slot size that is compatible with filter material.
Select pipe size based on anticipated toe drain flow and pipe slope that meets USBR criteria.
Check water velocity through toe drain slots based on area per foot in specifications.

Summary of Conclusions

Use of ASTM C33 #67 aggregate will be an acceptable filter for the native soils at this site.
A maximum slot size of 1/4 inch is acceptable for the selected filter material.
8 inch slotted pipe will not flow more than 50% full for drains at design slopes, meets USBR.
8 inch solid discharge pipe will discharge anticipated flow from both toe drains without pressurizing.
Acceptable velocity across slots using a minimum opening of 1.5 sq.in. per foot.

Originator

Print	Sign	Date
Walter Kramb		Dec 14, 2021

Checked

Print	Sign	Date

Base Material

Determine the gradation curves of the base soil. Use enough samples to define the range of grain size for the base soil. Design the filter gradation based on the base soil that requires the smallest D_{15F} size. If soil has particles larger than the #4 sieve, an adjusted gradation is calculated. Input values below for the base soil (original) gradation (in red):

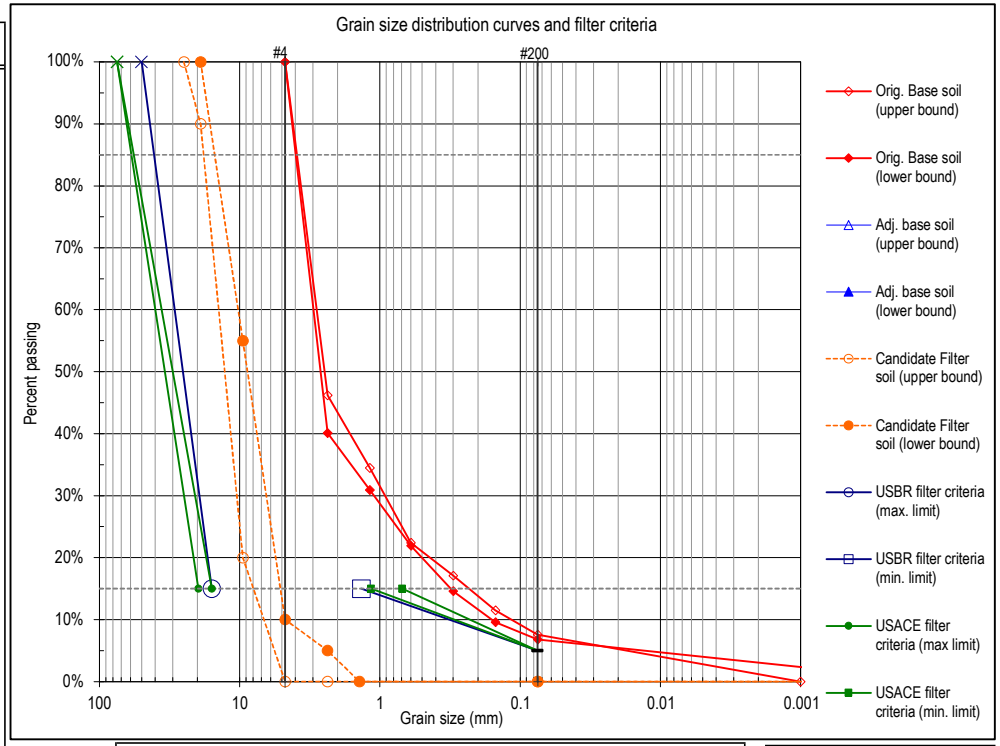
Project: Home Office Underdrain
ASTM #67 Gravel Filter

Name: W. Kramb
Date: 10/5/2021

Particle size (mm)	Sieve #	Base soil (original), % passing (upper bound)	(lower bound)	Adjusted gradation, % passing upper bound	lower bound
75	-				
37.5	-			*(No adjustment needed)*	
19.0	-				
9.5	-				
4.75	4	100.0%	100.0%		
4.00	5				
3.35	6				
2.80	7				
2.36	8	46.2%	40.1%		
2.00	10				
1.70	12				
1.40	14				
1.18	16	34.5%	30.9%		
1.00	18				
0.850	20				
0.710	25				
0.600	30	22.4%	21.9%		
0.500	35				
0.425	40				
0.300	50	17.1%	14.6%		
0.250	60				
0.212	70				
0.180	80				
0.150	100	11.5%	9.6%		
0.125	120				
0.106	140				
0.090	170				
0.075	200	7.6%	6.8%		
0.053	270				
0.037	-				
0.019	-				
0.009	-				
0.005	-				
0.002	-				
0.001	-	0.0%			
0.0001	-	0.0%	0.0%		

Required entry values are defined below. *

Average % passing #200 after regarding (if any) = A = 7.2%



Properties of base soil	Upper bound	Lower bound
D_{85B} of original base soil =	3.908 mm	3.987 mm
D_{85B} of adjusted base soil =	--- mm	--- mm
D_{15B} of original base soil =	0.2313 mm	0.3116 mm
D_{15B} of adjusted base soil =	--- mm	--- mm
D_{60B} of original base soil =	2.824 mm	2.977 mm
D_{10B} of original base soil =	0.1149 mm	0.1586 mm
C_u of original base soil =	24.6	18.8

Fixed points on graph:
- <5% passing #200
- 50mm max. grain size (USBR)
- 75mm max. grain size (USACE)

Filter Material

Filter criteria required by the USBR as published in *Design Standards - Embankment Dams No. 13 (1994):*

D_{85B} used in filter design	3.948
Average Passing #200 sieve of base soil	7.2%
Base soil category	4
Base soil description	Sands and gravels
Filter criteria (mm)	Maximum: $D_{15F} \leq$ 15.79 To ensure sufficient permeability: Minimum: $D_{15F} \geq$ 1.36
Maximum particle size of filter (mm)	50
Maximum % passing # 200 sieve	5%
PI of material passing #40	0 when tested in accordace with USBR 5360, Earth Manual, on material passing #40

Filter criteria required by the US Army Corps of Engineers as published in EM 1110-2-2300 (31 Jul 94):

D_{85B} used in filter design	3.948
Average Passing #200 sieve of base soil	7.2%
Base soil category	4**
Filter criteria (mm)	Maximum: $D_{15F} \leq$ 15.79 to 19.74 To ensure sufficient permeability: Minimum: $D_{15F} \geq$ 0.69 to 1.16
Maximum particle size of filter (mm)	75
Maximum % passing # 200 sieve	5%
PI of material passing #40	0 when tested in accordance with EM 1110-2-1906

**If the base soil is in category 4, use the lower of the two 'max. D_{15F}' values when the filter is beneath riprap subject to wave action or beneath drains which may be subject to violent surging and/or vibration.

*Required entry values for base soil & candidate filter gradations:

1. Particle size for 100% passing.
2. % Passing the #4 sieve.
3. % Passing the #200 sieve.
4. One point in the 85% - 90% range and another point in the 80% - 85% range, or the 85% point.
5. One point in the 15% - 20% range and another point in the 10% - 15% range, or the 15% point.
6. No duplicate entries; if D100<#4, enter 101% for #4 and 100% for appropriate size.

USBR filter gradation limits:

Maximum limit	
Grain size (mm)	% Passing
50.00	100.0%
15.79	15.0%

Minimum limit	
Grain size (mm)	% Passing
1.36	15.0%
0.075	5.0%

USACE filter gradation limits:

Maximum limit	
Grain size (mm)	% Passing
75.00	100.0%
19.74	15.0%
15.79	15.0%

Minimum limit	
Grain size (mm)	% Passing
0.69	15.0%
1.16	15.0%
0.075	5.0%

Candidate filter soil gradation. Values shown in red in the left column, and all values in the two right columns, can be changed.

Particle size *mm*	Sieve #	% Passing (upper bound)	% Passing (lower bound)
150.0	-		
100.0	-		
90.0	-		
75.0	-		
63.0	-		
50.0	-		
37.5	-		
25.0	-	100.0%	
19.0	-	90.0%	100.0%
12.5	-		
9.5	-	20.0%	55.0%
4.75	4	0.0%	10.0%
3.35	6		
2.36	8	0.0%	5.0%
2.00	10		
1.70	12		
1.40	14		(0.0%)
1.18	16		
0.850	20		
0.600	30		
0.425	40		
0.300	50		
0.250	60		
0.212	70		
0.180	80		
0.150	100		
0.125	120		
0.106	140		
0.090	170		
0.075	200	(0.0%)	(0.0%)
0.053	270		
0.037	-		
0.019	-		
0.009	-		
0.0001	-	0.0%	0.0%

Required entry values are defined above. *

Properties of candidate filter soil (CF). D sizes are in mm:

	D_{85CF}	D_{15CF}	D_{60CF}	D_{10CF}	C_u
upper bound	18.08	7.99	14.12	6.72	2.10
lower bound	15.08	5.13	10.26	4.75	2.16

Acceptibility of candidate filter (CF) soil:

USBR criteria	Upper bound	Lower bound
Max % passing #200:	OK	OK
Max particle size (mm):	OK	OK
Maximum D_{15CF}:	OK	OK
Minimum D_{15CF}:	OK	OK
To minimize segregation (from *Table 2*)***		
Max allowable D_{90CF} =	40	OK
Max D_{90CF} =	19.00	

USACE criteria	Upper bound	Lower bound
Max % passing #200:	OK	OK
Max particle size (mm):	OK	OK
Maximum D_{15CF}:	OK	OK
Minimum D_{15CF} ($3\times D_{15B}$):	OK	OK
Minimum D_{15CF} ($5\times D_{15B}$):	OK	OK
To minimize segregation (from *Table B-3*)***		
Max allowable D_{90CF} =	40	OK
Max D_{90CF} =	19.00	

Filters should be relatively uniform (see the C_U value of the candidate filter soil.). Also, filters should not be gap-graded.

*** Generally, this requirement is only necessary for coarse filters and gravel zones that serve as both filters and drains. For sand filters with D_{90} < ~20mm, these limitations are usually not necessary.

Home office underdrain
10-5-21

of D15F to D85B over that used for protecting a natural or unprocessed soil. The ratio can be as high as 9, but 5 is generally found to meet the practical requirements of the situation. This increase is sometimes possible because the first-stage filter: (1) is a material processed to stringent gradation requirements and placed and compacted under controlled conditions, (2) is inspected and tested to verify that material properties conform to those that are specified, (3) usually has seepage gradients that are much less than those of a foundation material or impervious zone that needs filter protection, and (4) has D_{85} particles in the first stage filter material that are larger than those in materials that are usually being protected and, therefore, less likely to move. However, this increase should be made with caution.

5.5.2 Drain Pipe Perforation Size

The maximum pipe perforation dimension[19] should be no larger than the finer side of the $D_{50}E$ where $D_{50}E$ is taken from the gradation of the envelope (drain) material that surrounds the drainpipe. That is:

D 50 min for Astm C33 #57 gravel – ~15mm = 0.59in
#67 gravel – ~12mm = 0.47in

$$\text{Max Perforation Dimension} \leq D_{50}E$$

It is emphasized that inaccessible drainpipes beneath embankment dams should be avoided. Drainpipes should be sized and located, and inspection wells should be provided so that access for inspection, maintenance, and repair, if necessary, is easy. It is recommended that each pipe segment be accessible from both ends. In order to provide a margin of safety for the pipe capacity, drains should be sized so that the depth of water in the drainpipe is less than 50 percent of the inside diameter of the drainpipe at the maximum expected discharge. If it is anticipated that the drainpipe will collect a large amount of flow from a pervious foundation or embankment, the maximum depth of water should not exceed 25 percent of the inside pipe diameter due to uncertainties in predicting the amount of flow.

Use ½" slots max
¼" more reasonable
also consider drilled pipe
Review size & slope

5.6 Laboratory Test Procedures

In the following section, test procedures for laboratory tests are presented. The procedures have been separated into two categories: particle retention and material quality. The particle retention tests evolved from the original test procedures used during research into particle movement. The material quality tests come mainly from industry standard tests, although one stems from research work.

---

[19] The maximum dimension as used in this standard is the width for a slot and the diameter for a hole.

Home Office Underdrain
Seep W Analysis
Drain pipe sizing.

North Drain

Inputs

Length:	1370	ft.	- from drawings
Slope	0.0047	ft./ft.	
Pipe Diame	8	in	
Manning's	0.0009		
Slots	1.5	in2/ft	specified minimum slot area

Outputs

	Seep W Input					Seep W Results				Pipe	
Sand and Gravel (k)	Model Ground Surface	Model Groundwater Elevation	Groundwater depth below ground	Drain Elevation	Drain Depth	q	Total flow	Total flow	Flow height - solved in Flowmaster	% of diameter	Water velocity through slots
cm/sec	ft.	ft.	ft.	ft.	ft.	cfs/ft.	cfs	gpm	in		ft/sec
2.54E-03	5,024	5,019	5	5017	7	8.77E-06	1.20E-02	5.392287	0.6	7.1%	8.42E-04
2.54E-03	5,024	5,021	3	5017	7	1.85E-05	2.53E-02	11.37484	0.8	10.1%	1.78E-03
1.00E-02	5,024	5,019	5	5017	7	3.50E-05	4.80E-02	21.51996	1.1	13.7%	3.36E-03
1.00E-02	5,024	5,021	3	5017	7	7.39E-05	1.01E-01	45.43786	1.6	19.6%	7.09E-03
High flow check - 100 gpm, greater than 200% of model						1.63E-04	2.23E-01	100	2.3	29.3	1.56E-02

West Drain

Inputs

Length:	1326	ft.	- from drawings
Slope	0.0023	ft./ft.	
Pipe Diame	8	in	
Manning's	0.0009		
Slots	1.5	in2/ft	specified minimum slot area

Outputs

	Seep W Input					Seep W Results				Pipe	
Sand and Gravel (k)	Model Ground Surface	Model Groundwater Elevation	Groundwater depth below ground	Drain Elevation	Drain Depth	q	Total flow	Total flow	Flow height - solved in Flowmaster	% of diameter	Water velocity through slots
cm/sec	ft.	ft.	ft.	ft.	ft.	cfs/ft.	cfs	gpm	in		ft/sec
2.54E-03	5,024	5,019	5	5017	7	8.77E-06	1.16E-02	5.219104	0.7	8.2%	8.42E-04
2.54E-03	5,024	5,021	3	5017	7	1.85E-05	2.45E-02	11.00951	0.9	11.8%	1.78E-03
1.00E-02	5,024	5,019	5	5017	7	3.50E-05	4.64E-02	20.82881	1.3	16.0%	3.36E-03
1.00E-02	5,024	5,021	3	5017	7	7.39E-05	9.80E-02	43.97854	1.8	23.1%	7.09E-03
High flow check - 100 gpm, greater than 200% of model						1.68E-04	2.23E-01	100	2.8	35.3%	1.61E-02

Discharge

Length:	1400	ft. - from drawings
Slope	0.0012	ft./ft.
Pipe Diame	8	in
Manning's	0.0009	

	Seep W Input					Seep W Results				Pipe
	Sand and Gravel (k)	Model Ground Surface	Model Groundwater Elevation	Groundwater depth below ground	Drain Elevation	Drain Depth	q	Total flow	Total flow	Flow height - solved in Flowmaster	% of diameter
	cm/sec	ft.	ft.	ft.	ft.	ft.	cfs/ft.	cfs	gpm	in	
	2.54E-03	5,024	5,019	5	5017	7		2.36E-02	10.61139	1.1	13.5%
	2.54E-03	5,024	5,021	3	5017	7		4.99E-02	22.38435	1.6	19.4%
	1.00E-02	5,024	5,019	5	5017	7		9.44E-02	42.34877	2.1	26.7%
	1.00E-02	5,024	5,021	3	5017	7		1.99E-01	89.4164	3.2	39.5%
	High flow check - 200 gpm, greater than 200% of model							4.46E-01	200	5.1	63.9%

Notes:

See USBR DS-15(5) - Filter Drsign. Paragraph 5.5.2 - "Drains sould be sized so that the depth of water in the drain pipe is less than 50% of the inside diameter.

Slot velocity not specified by design standard. Calculated to check, maintin below 0.06 ft/sec, which the maximum orfice velocity at 100 gpm as calculated in Flow Master

TABLE 5.5 Coarse Aggregate Grading Requirements for Concrete (Reprinted, with permission, from ASTM C33, Table 2, copyright ASTM International, 100 Barr Harbor Drive, West Conshohocken, PA 19428).

		Amounts Finer Than Each Laboratory Sieve (Square Openings), Weight Percent												
Size No.	**Nominal Size**	**4 in. (100 mm)**	**3 1/2 in. (90 mm)**	**3 in. (75 mm)**	**2 1/2 in. (63 mm)**	**2 in. (50 mm)**	**1 1/2 in. (37.5 mm)**	**1 in. (25.0 mm)**	**3/4 in. (19.0 mm)**	**1/2 in. (12.5 mm)**	**3/8 in. (9.5 mm)**	**No. 4 (4.75 mm)**	**No. 8 (2.36 mm)**	**No. 16 (1.18 mm)**
1	3 1/2 to 1 1/2 in. (90 to 37.5 mm)	100	90 to 100	...	25 to 60	...	0 to 15	...	0 to 5	...	...	...	...	...
2	2 1/2 to 1 1/2 in. (63 to 37.5 mm)	...	...	100	90 to 100	35 to 70	0 to 15	...	0 to 5	...	...	...	...	...
3	2 to 1 in. (50 to 25.0 mm)	...	...	...	100	90 to 100	35 to 70	0 to 15	...	0 to 5	...	...	...	...
357	2 in. to No. 4 (50 to 4.75 mm)	...	...	...	100	95 to 100	...	35 to 70	...	10 to 30	...	0 to 5	...	...
4	1 1/2 to 3/4 in. (37.5 to 19 mm)	...	...	...	...	100	90 to 100	20 to 55	0 to 15	...	0 to 5	...	...	...
467	1 1/2 in. to No. 4 (37.5 to 4.75 mm)	...	...	...	...	100	95 to 100	...	35 to 70	...	10 to 30	0 to 5	...	...
5	1 to 1/2 in. (25.0 to 12.5 mm)	...	...	...	...	...	100	90 to 100	20 to 55	0 to 10	0 to 5	...	...	...
56	1 to 3/8 in. (25.0 to 9.5 mm)	...	...	...	...	...	100	90 to 100	40 to 85	10 to 40	0 to 15	0 to 5	...	...
57	1 in. to No. 4 (25.0 to 4.75 mm)	...	...	...	...	...	100	95 to 100	...	25 to 60	...	0 to 10	0 to 5	...
6	3/4 in. to 3/8 in. (19.0 to 9.5 mm)	...	...	...	...	...	...	100	90 to 100	20 to 55	0 to 15	0 to 5	...	...
67	3/4 in. to No. 4 (19.0 to 4.75 mm)	...	...	...	...	...	...	100	90 to 100	...	20 to 55	0 to 10	0 to 5	...
7	1/2 in. to No. 4 (12.5 to 4.75 mm)	...	...	...	...	...	...	...	100	90 to 100	40 to 70	0 to 15	0 to 5	...
8	3/8 in. to No. 8 (9.5 to 2.36 mm)	...	...	...	...	...	...	...	...	100	85 to 100	10 to 30	0 to 10	0 to 5

CONSTRUCTION PLANS FOR

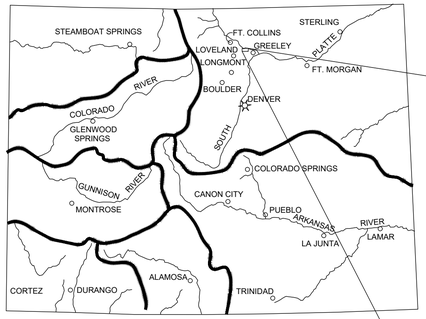
HOME OFFICE MINE
UNDERDRAIN AS-CONSTRUCTED

LARIMER COUNTY,COLORADO

PREPARED FOR:

MARTIN MARIETTA
1800 NORTH TAFT HILL ROAD
FORT COLLINS, COLORADO 80521

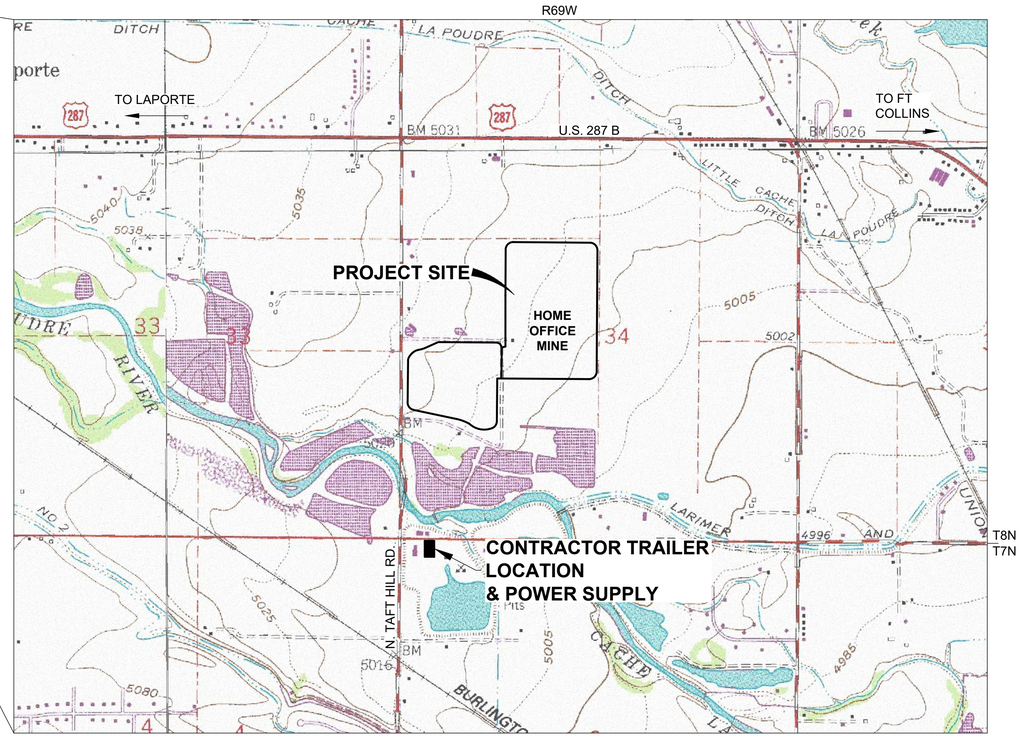
WATER DIVISION NO. 1, WATER DISTRICT NO. 3
LARIMER COUNTY, COLORADO



STATE OF COLORADO
PROJECT LOCATION MAP

NORTH

SCALE: NONE



PROJECT VICINITY MAP

NORTH

SCALE: 1"=1000'

NOTES:

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WRITTEN SCALES ON PLAN ARE FOR FULL SIZED 22" x 34" PLANS AND DO NOT APPLY TO REDUCED PLAN SETS.

SHEET INDEX:

811 Know what's below. Call before you dig.

CALL 2-BUSINESS DAYS IN ADVANCE BEFORE YOU DIG, GRADE, OR EXCAVATE FOR THE MARKING OF UNDERGROUND MEMBER UTILITIES.

DEERE & AULT CONSULTANTS, INC. ASSUMES NO RESPONSIBILITY FOR EXISTING UTILITY LOCATIONS (HORIZONTAL AND VERTICAL). THE EXISTING UTILITIES SHOWN ON THIS DRAWING HAVE BEEN PLOTTED FROM THE BEST AVAILABLE INFORMATION. IT IS, HOWEVER, THE RESPONSIBILITY OF THE CONTRACTOR TO FIELD VERIFY THE LOCATION OF ALL UTILITIES PRIOR TO THE COMMENCEMENT OF ANY CONSTRUCTION ACTIVITIES.

REV	DESCRIPTION	BY	DATE
1	60% DESIGN - NOT FOR CONSTRUCTION	WK	11-2-21
2	FOR CONSTRUCTION	SR	11-29-21
	AS-CONSTRUCTED	SR	5-23-22

DESIGNED BY: ----
DRAWN BY: ----
CHECKED BY: ----
DATE: ----
STATE PROFESSIONAL ENGINEER NO. XXXXX

DEERE & AULT
A SCHNABEL ENGINEERING COMPANY
600 SOUTH AIRPORT ROAD, BUILDING A, SUITE 205
LONGMONT, COLORADO 80503
PHONE: 303.651.1468

HOME OFFICE MINE
UNDERDRAIN AS-CONSTRUCTED

COVER SHEET

PROJECT: DA494019.00
DATE: ----
SHEET 1 OF 9

U:\0494 MARIETTA\0494.019 HOME OFFICE MINE\CAD\WORKING\UNDERDRAIN\HO UNDERDRAIN AS-CONSTRUCTEDASB_HOME OFFICE - COVER.DWG

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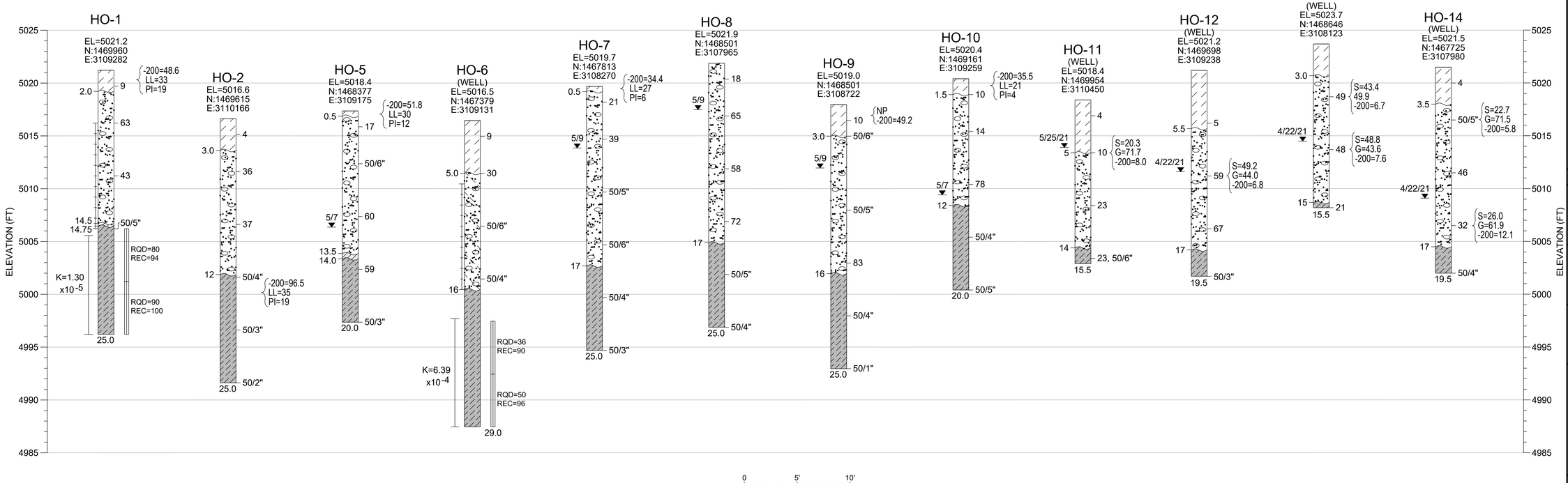


LEGEND:

- CLAY: MEDIUM STIFF, SILTY, LOCALLY SANDY, FINE TO MEDIUM GRAINED, MOIST TO WET, BROWN TO DARK BROWN , MODERATELY PLASTIC TO PLASTIC, LOCALLY ROOTS
- CLAYEY SAND & GRAVEL: LOOSE TO MEDIUM DENSE, SILTY, FINE TO MEDIUM GRAINED, LOCAL SMALL GRAVEL, DRY TO WET, LIGHT BROWN TO DARK BROWN, LOW TO MODERATELY PLASTIC, LOCAL PLANT ROOTS (SC)
- SILTY SAND & GRAVEL: LOOSE TO MEDIUM DENSE, CLAYEY, BROWN, DRY, NON TO VERY LOW PLASTIC, LOCAL ROOTS PRESENT
- SAND: LOOSE, POORLY GRADED, FINE TO MEDIUM GRAINED, MOIST, GRAYISH BROWN
- SAND & GRAVEL: MEDIUM DENSE TO VERY DENSE, SANDY GRAVEL TO GRAVELLY SAND, LOCAL COBBLES, WELL GRADED, FINE TO COARSE GRAINED SAND, SMALL GRAVEL UP TO 8 INCH GRAVEL, ROUNDED GRAVEL AND COBBLES, DRY TO WET, PINKISH BROWN TO LIGHT BROWN TO TANISH BROWN (SW, GW, GM)
- WEATHERED CLAYSTONE: STIFF (SOILS), VERY LOW TO LOW STRENGTH BEDROCK, DRY TO SLIGHTLY MOIST, VERY FINE SAND, MOTTLED BROWN TO BROWNISH TAN, LOCAL IRON STAINING, MIXTURE OF BEDROCK COLORS, MODERATELY PLASTIC, BLOCKY
- CLAYSTONE: HARD TO VERY HARD (SOILS), VERY LOW TO LOW STRENGTH BEDROCK, DRY TO SLIGHTLY MOIST, SANDY, VERY FINE SAND, GRAY TO DARK GRAY, LOW TO MODERATELY PLASTIC, LOCAL SHALE THINLY BEDDED, BLOCKY
- SANDSTONE: HARD (SOILS), LOW TO MODERATE STRENGTH BEDROCK, DRY TO SLIGHTLY MOIST, CLAYEY, VERY FINE SAND, LIGHT GRAY TO GRAY, LOCAL SHALE THINLY BEDDED, BLOCKY, FRESH

9.3 APPROXIMATE DEPTH OF GEOLOGIC CONTACT

5/7 WATER ENCOUNTERED DURING DRILLING AND DATE MEASURED

35 STANDARD SPLITSPOON BLOW COUNTS. NUMBER ("N" VALUE) INDICATES THE NUMBER OF BLOWS OF A 140 LB. HAMMER FREE FALLING 30 INCHES REQUIRED TO DRIVE THE SAMPLER 1 FOOT OR THE INDICATED INTERVAL. "WT" INDICATES THE SAMPLER ADVANCED WITH RESTING WEIGHT OF THE HAMMER

PIEZOMETER SCREENED PIPE INTERVAL

PACKER TEST INTERVALS & CALCULATED PERMEABILITY, K (CM/S), TL= TOO LOW TO MEASURE

CORE INTERVALS (HQ)
RQD = ROCK QUALITY DESIGNATION (%)
REC = RECOVERED CORE (%)

25.0 DEPTH OF BORING (FT)

NOTES:

1. EXPLORATORY BORINGS HO-1 THROUGH HO-10 WERE DRILLED 5/7/18 THROUGH 5/9/18, BORINGS HO-12 THROUGH HO-14 WERE DRILLED ON 4/22/21 AND HO-11 WAS DRILLED ON 5/25/21 USING A TRUCK-MOUNTED CME 75 DRILL RIG. BORINGS WERE DRILLED WITH 4.25-INCH I.D. HOLLOW STEM AUGERS.
2. EXPLORATORY TEST PITS WERE EXCAVATED ON 5/11/18 USING A 350LC HITACHI BACKHOE.
3. ALL EXPLORATORY BORING LOCATIONS AND ELEVATIONS WERE SURVEYED BY KING SURVEYORS FOR THE GEOTECHNICAL INVESTIGATION LOCATIONS AND ELEVATIONS FOR ALL OTHER TEST PITS WERE INTERPOLATED FROM THE SURVEYED GROUND SURFACE.
4. TP 1, 4, 7, 10 & 14 LOCATIONS AND ELEVATIONS WERE SURVEYED BY KING SURVEYORS ON 5/17/18
5. LINES BETWEEN MATERIALS REPRESENT APPROXIMATE BOUNDARIES BETWEEN TYPES AND TRANSITIONS MAY BE GRADUAL.
6. GROUNDWATER LEVELS WERE MEASURED AT THE TIME OF DRILLING. GROUNDWATER LEVELS MAY FLUCTUATE SEASONALLY AND DUE TO SITE MINING AND DEWATERING OPERATIONS.
7. LAB TESTING:
   - G = % GRAVEL BY WEIGHT
   - S = % SAND BY WEIGHT
   - -200 = % BY WEIGHT PASSING THE #200 SIEVE (FINES)
   - LL = LIQUID LIMIT
   - PI = PLASTICITY INDEX
   - NP = NON PLASTIC MATERIAL
8. STOCKPILE SAMPLE TESTING SUMMARY:

TP-2	TP-5	TP-11
G=0.6	G=5.5	G=15.3
S=50.3	S=41.2	S=46.7
-200=49.1	-200=53.3	-200=38.0
LL=27	LL=32	LL=25
PI=7	PI=15	PI=10
	%ORGANIC=2.3	

REV	DESCRIPTION	BY	DATE
1	60% DESIGN - NOT FOR CONSTRUCTION	WK	11-2-21
2	FOR CONSTRUCTION	SR	11-29-21
3	AS-CONSTRUCTED	SR	5-23-22

DESIGNED BY: ---- DRAWN BY: ---- CHECKED BY: ----

DATE: ----

STATE PROFESSIONAL ENGINEER NO. XXXXX

DEERE & AULT
A SCHNABEL ENGINEERING COMPANY
600 S. AIRPORT RD., BLDG A, SUITE 205
LONGMONT, CO 80503
TEL: 303.651.1468 FAX: 303.651.1469

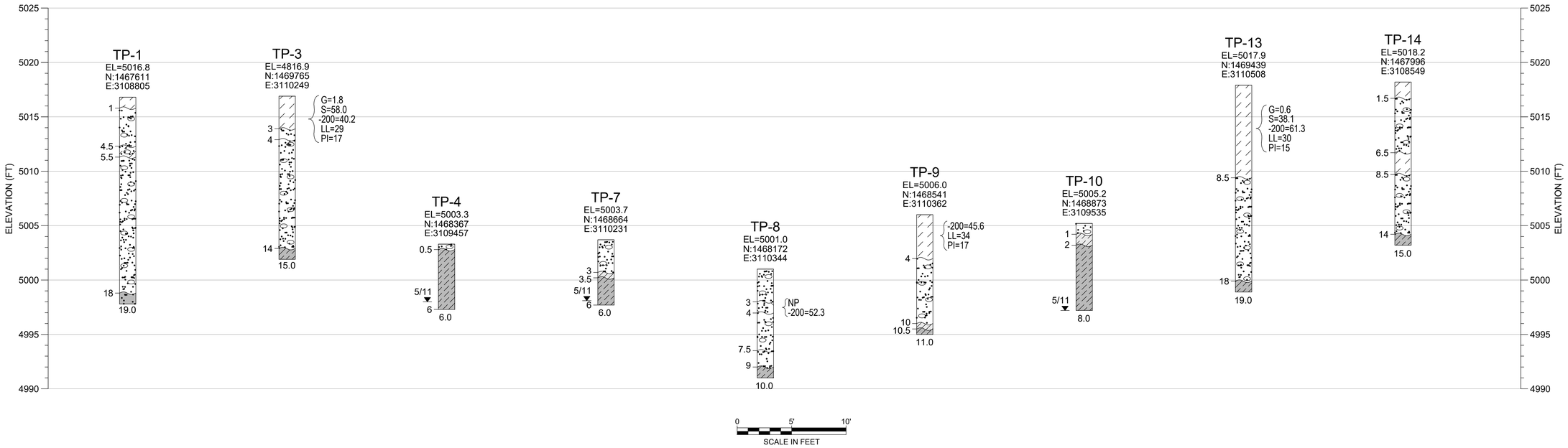
HOME OFFICE MINE
UNDERDRAIN AS-CONSTRUCTED

SUMMARY LOGS OF EXPLORATORY TESTHOLES

PROJECT: DA494019.00

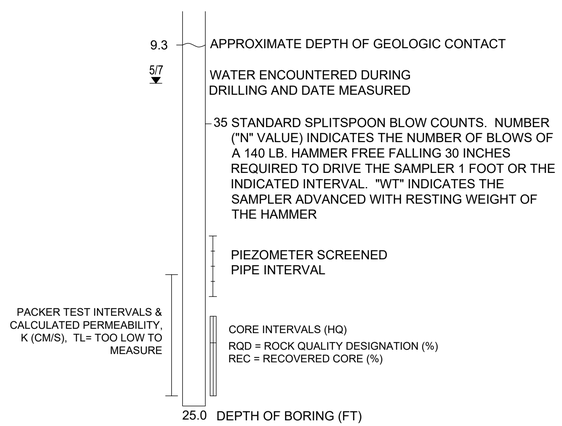
DATE: ----

SHEET
2 OF 9



LEGEND:

- CLAY: MEDIUM STIFF, SILTY, LOCALLY SANDY, FINE TO MEDIUM GRAINED, MOIST TO WET, BROWN TO DARK BROWN , MODERATELY PLASTIC TO PLASTIC, LOCALLY ROOTS
- CLAYEY SAND & GRAVEL: LOOSE TO MEDIUM DENSE, SILTY, FINE TO MEDIUM GRAINED, LOCAL SMALL GRAVEL, DRY TO WET, LIGHT BROWN TO DARK BROWN, LOW TO MODERATELY PLASTIC, LOCAL PLANT ROOTS (SC)
- SILTY SAND & GRAVEL: LOOSE TO MEDIUM DENSE, CLAYEY, BROWN, DRY, NON TO VERY LOW PLASTIC, LOCAL ROOTS PRESENT
- SAND: LOOSE, POORLY GRADED, FINE TO MEDIUM GRAINED, MOIST, GRAYISH BROWN
- SAND & GRAVEL: MEDIUM DENSE TO VERY DENSE, SANDY GRAVEL TO GRAVELLY SAND, LOCAL COBBLES, WELL GRADED, FINE TO COARSE GRAINED SAND, SMALL GRAVEL UP TO 8 INCH GRAVEL, ROUNDED GRAVEL AND COBBLES, DRY TO WET, PINKISH BROWN TO LIGHT BROWN TO TANISH BROWN (SW, GW, GM)
- WEATHERED CLAYSTONE: STIFF (SOILS), VERY LOW TO LOW STRENGTH BEDROCK, DRY TO SLIGHTLY MOIST, VERY FINE SAND, MOTTLED BROWN TO BROWNISH TAN, LOCAL IRON STAINING, MIXTURE OF BEDROCK COLORS, MODERATELY PLASTIC, BLOCKY
- CLAYSTONE: HARD TO VERY HARD (SOILS), VERY LOW TO LOW STRENGTH BEDROCK, DRY TO SLIGHTLY MOIST, SANDY, VERY FINE SAND, GRAY TO DARK GRAY, LOW TO MODERATELY PLASTIC, LOCAL SHALE THINLY BEDDED, BLOCKY
- SANDSTONE: HARD (SOILS), LOW TO MODERATE STRENGTH BEDROCK, DRY TO SLIGHTLY MOIST, CLAYEY, VERY FINE SAND, LIGHT GRAY TO GRAY, LOCAL SHALE THINLY BEDDED, BLOCKY, FRESH



NOTES:

1. EXPLORATORY BORINGS HO-1 THROUGH HO-10 WERE DRILLED 5/7/18 THROUGH 5/9/18, BORINGS HO-12 THROUGH HO-14 WERE DRILLED ON 4/22/21 AND HO-11 WAS DRILLED ON 5/25/21 USING A TRUCK-MOUNTED CME 75 DRILL RIG. BORINGS WERE DRILLED WITH 4.25-INCH I.D. HOLLOW STEM AUGERS.
2. EXPLORATORY TEST PITS WERE EXCAVATED ON 5/11/18 USING A 350LC HITACHI BACKHOE.
3. ALL EXPLORATORY BORING LOCATIONS AND ELEVATIONS WERE SURVEYED BY KING SURVEYORS FOR THE GEOTECHNICAL INVESTIGATION LOCATIONS AND ELEVATIONS FOR ALL OTHER TEST PITS WERE INTERPOLATED FROM THE SURVEYED GROUND SURFACE.
4. TP 1, 4, 7, 10 & 14 LOCATIONS AND ELEVATIONS WERE SURVEYED BY KING SURVEYORS ON 5/17/18
5. LINES BETWEEN MATERIALS REPRESENT APPROXIMATE BOUNDARIES BETWEEN TYPES AND TRANSITIONS MAY BE GRADUAL.
6. GROUNDWATER LEVELS WERE MEASURED AT THE TIME OF DRILLING. GROUNDWATER LEVELS MAY FLUCTUATE SEASONALLY AND DUE TO SITE MINING AND DEWATERING OPERATIONS.
7. LAB TESTING:
   - G = % GRAVEL BY WEIGHT
   - S = % SAND BY WEIGHT
   - -200 = % BY WEIGHT PASSING THE #200 SIEVE (FINES)
   - LL = LIQUID LIMIT
   - PI = PLASTICITY INDEX
   - NP = NON PLASTIC MATERIAL
8. STOCKPILE SAMPLE TESTING SUMMARY:

TP-2	TP-5	TP-11
G=0.6	G=5.5	G=15.3
S=50.3	S=41.2	S=46.7
-200=49.1	-200=53.3	-200=38.0
LL=27	LL=32	LL=25
PI=7	PI=15	PI=10
	%ORGANIC=2.3	

NO.	DESCRIPTION	BY	DATE
1	60% DESIGN - NOT FOR CONSTRUCTION	WK	11-2-21
2	FOR CONSTRUCTION	SR	11-29-21
3	AS-CONSTRUCTED	SR	5-23-22

DESIGNED BY: ---- DRAWN BY: ---- CHECKED BY: ----

DATE: ____
STATE PROFESSIONAL ENGINEER NO. XXXXX

DEERE & AULT
A SCHNABEL ENGINEERING COMPANY
600 S. AIRPORT RD., BLDG A, SUITE 205
LONGMONT, CO 80503
TEL 303.651.1468 FAX 303.651.1469

HOME OFFICE MINE
UNDERDRAIN AS-CONSTRUCTED

SUMMARY LOGS OF EXPLORATORY TESTPITS

PROJECT: DA494019.00
DATE: ----
SHEET 3 OF 9

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LEGEND:

- 4800 — EXISTING CONTOUR
- FLOOD PLAIN
- PROPERTY LINE
- HO-1 — MONITORING WELL LOCATION
- TP-7 — TEST PIT LOCATION
- HO-5 — TEST HOLE LOCATION
- CONVEYOR
- UTILITY EASEMENT

0 150 300

SCALE IN FEET
CONTOUR INTERVAL=1'

NO.	DESCRIPTION	REV	BY	DATE
1	60% DESIGN - NOT FOR CONSTRUCTION	WK	SR	11-2-21
2	FOR CONSTRUCTION	SR	SR	11-29-21
	AS-CONSTRUCTED	SR	SR	5-23-22

DESIGNED BY: ----
DRAWN BY: ----
CHECKED BY: ----

DATE: ----
STATE PROFESSIONAL ENGINEER NO. XXXXX

DEERE & AULT
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600 S. AIRPORT RD., BLDG. A, SUITE 205
LONGMONT, CO 80503
TEL 303.651.1468 FAX 303.651.1469

HOME OFFICE MINE
UNDERDRAIN AS-CONSTRUCTED

EXISTING CONDITIONS

PROJECT: DA494019.00
DATE: ----
SHEET
4 OF 9

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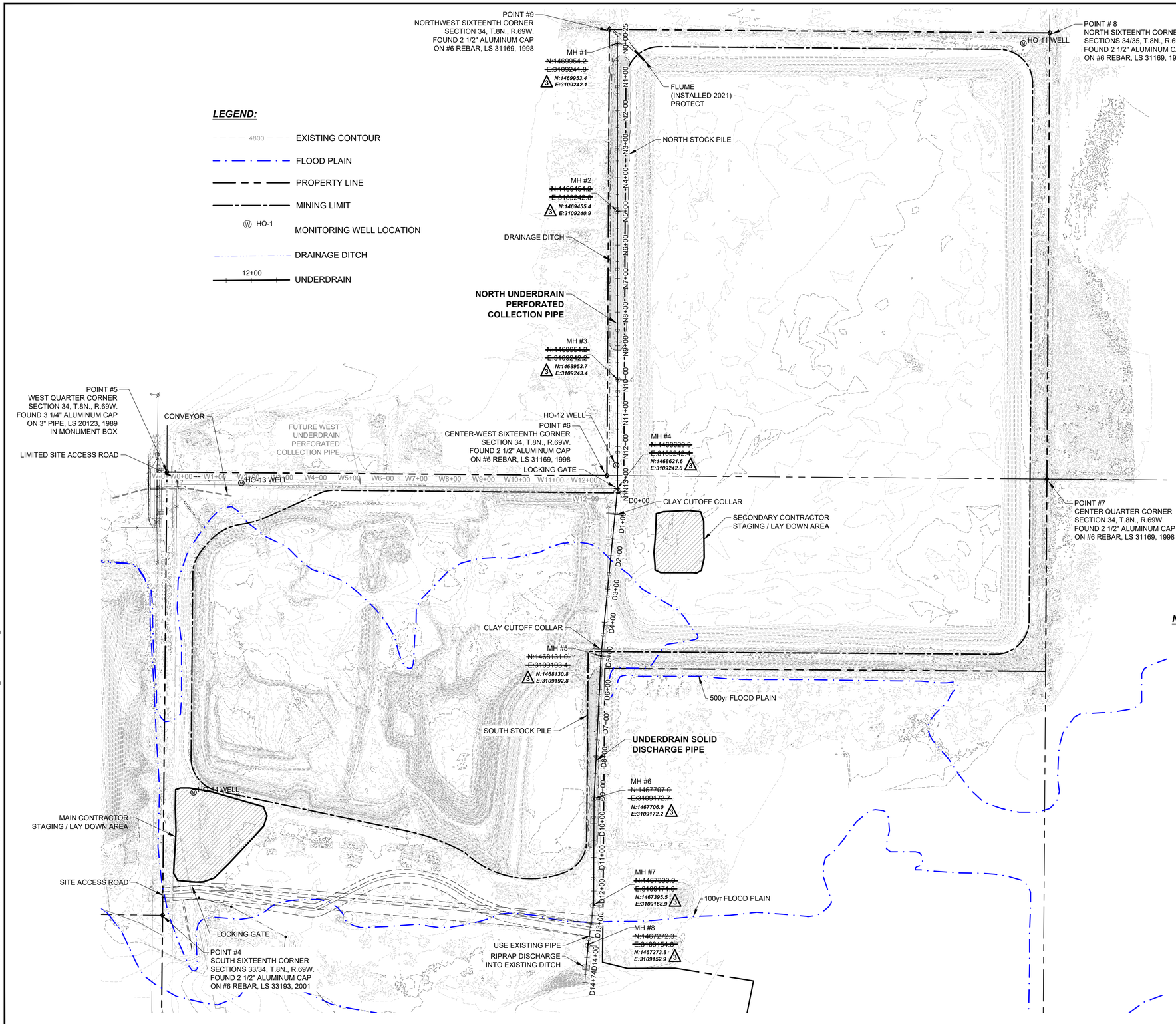

LEGEND:

- 4800 — EXISTING CONTOUR
- FLOOD PLAIN
- PROPERTY LINE
- MINING LIMIT
- Ⓦ HO-1 — MONITORING WELL LOCATION
- DRAINAGE DITCH
- 12+00 — UNDERDRAIN

NOTES:

1. ALL UTILITIES TO BE LOCATED, VERIFIED & POT HOLED AS NECESSARY BY CONTRACTOR.
2. TRAFFIC ON LIMITED SITE ACCESS ROAD TO BE COORDINATED WITH ADJACENT PROPERTY OWNERS & ONLY ALLOWED DURING STANDARD WORK HOURS.
3. SURVEY CONTROL PROVIDE BY KING SURVEYOR'S. COORDINATE VALUES ARE THAT OF THE COLORADO STATE PLANE COORDINATE SYSTEM, NORTH ZONE, NORTH AMERICAN DATUM 1983/92. TO CONVERT TO GROUND (MODIFIED) SCALE ABOUT POINT 0,0 AT A FACTOR OF 1.00026675 (0.99973332 CF) VERTICAL DATUM: NAVD 88.

EXISTING SURVEY POINT TABLE

POINT #	NORTHING	EASTING	DESCRIPTION
1	1466019.4650	3110502.4980	S. 1/4 COR, SEC
2	1466033.7797	3109189.4328	W. 1/16 COR, SEC34
3	1466042.3880	3107875.3510	SW. COR, SEC34
4	1467360.3495	3107888.3763	S. 1/16 COR, SEC 34
5	1468678.1520	3107901.4000	W. 1/4 COR, SEC 34
6	1468667.8170	3109210.9885	CENTER W. 1/16 COR, SEC 34
7	1468657.4820	3110520.5770	CENTER 1/4 COR, SEC 34
8	1469985.2060	3110529.0260	N. 1/16 COR, SEC 34
9	1469995.6070	3109218.1910	NW. 1/16 COR, SEC 34

NOTE: POINT # 1,2,3 ARE NOT SHOWN ON THIS PAGE. THEY ARE SOUTH OF THE SITE.

REV.	DESCRIPTION	BY	DATE
1	80% DESIGN - NOT FOR CONSTRUCTION	WK	11-2-21
2	FOR CONSTRUCTION	SR	11-29-21
3	AS-CONSTRUCTED	SR	5-23-22

DESIGNED BY: ---- DRAWN BY: ---- CHECKED BY: ----

DATE: ----

STATE PROFESSIONAL ENGINEER NO. XXXXX

DEERE & AULT
A SCHNABEL ENGINEERING COMPANY
600 S. AIRPORT RD., BLDG. A, SUITE 205
LONGMONT, CO 80503
TEL 303.651.1468 FAX 303.651.1469

HOME OFFICE MINE
UNDERDRAIN AS-CONSTRUCTED

GENERAL PLAN

PROJECT: DA494019.00

DATE: ----

SHEET
5 OF 9

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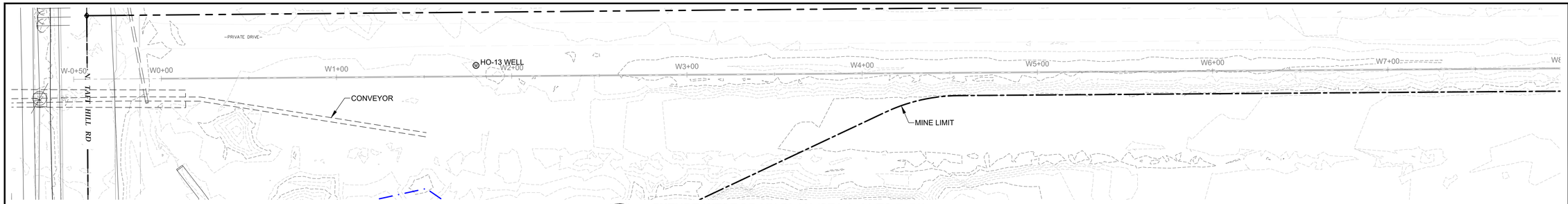
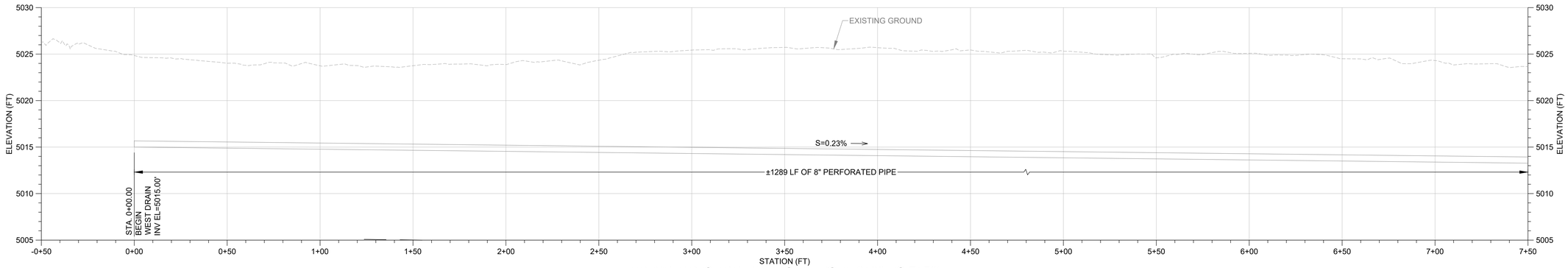


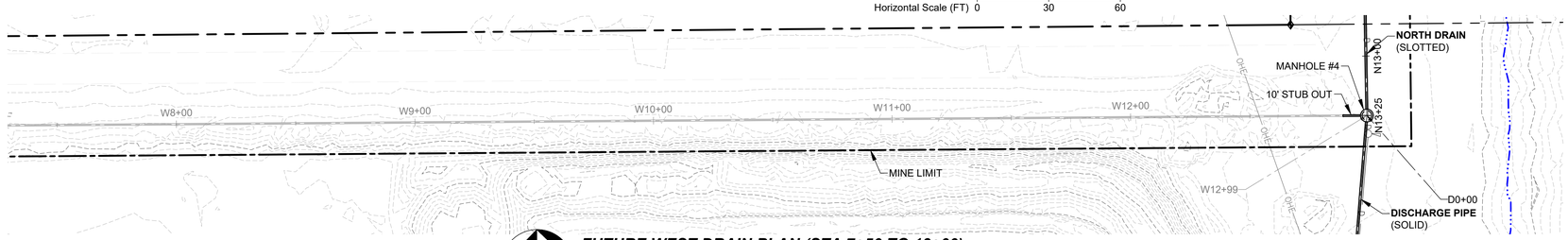
FUTURE WEST DRAIN PLAN (STA 0+00 TO 7+50)

0 30 60
SCALE IN FEET



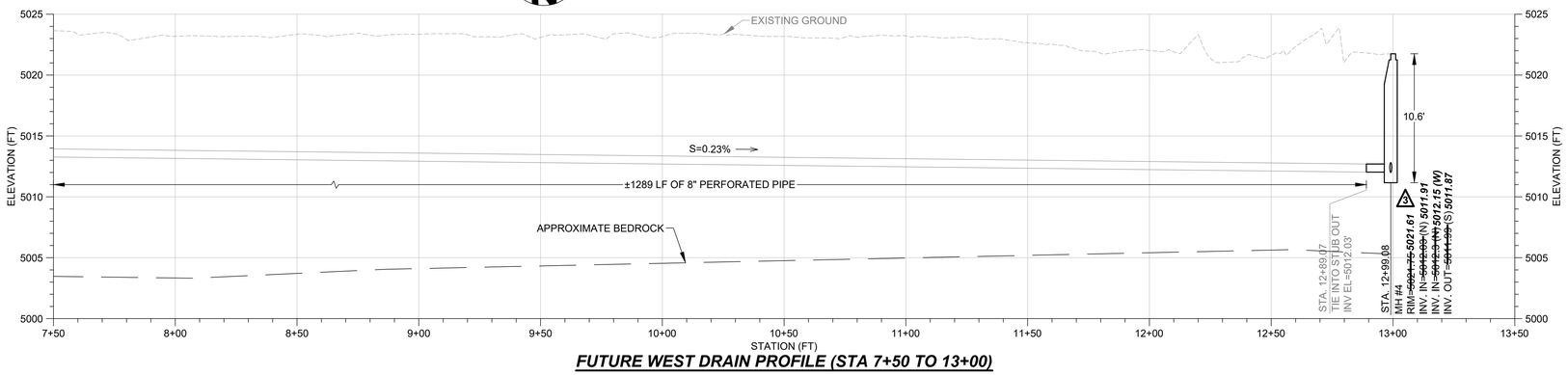
FUTURE WEST DRAIN PROFILE (STA 0+00 TO 7+50)

Vertical Scale (FT) 0 6 12
Horizontal Scale (FT) 0 30 60



FUTURE WEST DRAIN PLAN (STA 7+50 TO 13+00)

0 30 60
SCALE IN FEET



FUTURE WEST DRAIN PROFILE (STA 7+50 TO 13+00)

Vertical Scale (FT) 0 6 12
Horizontal Scale (FT) 0 30 60

REV	DESCRIPTION	BY	DATE
1	60% DESIGN - NOT FOR CONSTRUCTION	WK	11-2-21
2	FOR CONSTRUCTION	SR	11-29-21
3	AS-CONSTRUCTED	SR	5-23-22

DESIGNED BY: ----
DRAWN BY: ----
CHECKED BY: ----

DATE: ----
STATE PROFESSIONAL ENGINEER NO. XXXXX

DEERE & AULT
A SCHNABEL ENGINEERING COMPANY
600 S. AIRPORT RD., BLDG. A, SUITE 205
LONGMONT, CO 80503
TEL 303.651.1468 FAX 303.651.1469

HOME OFFICE MINE
UNDERDRAIN AS-CONSTRUCTED

WEST DRAIN LINE PLAN AND PROFILE

PROJECT: DA494019.00
DATE: ----
SHEET
6 OF 9

U:\0494 MARTIN MARIETTA\0494.019 HOME OFFICE MINE\CAD\WORKING\UNDERDRAIN\HO UNDERDRAIN AS-CONSTRUCTEDASB_HO UNDERDRAIN_PIPE PROFILES.DWG

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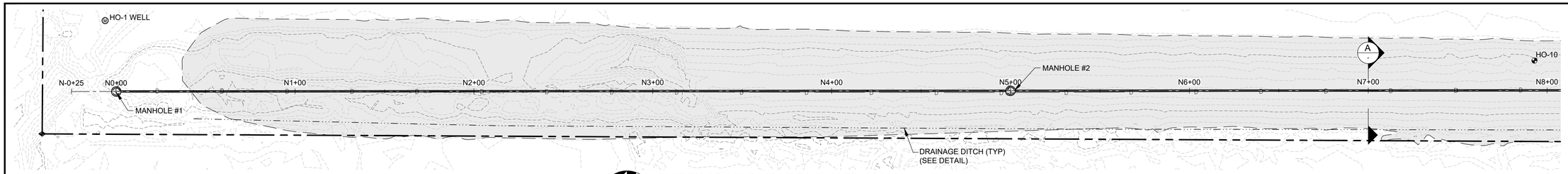
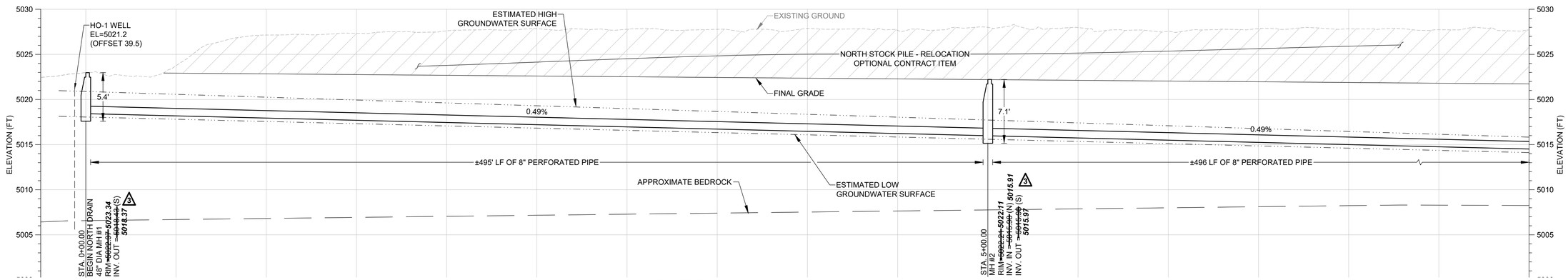
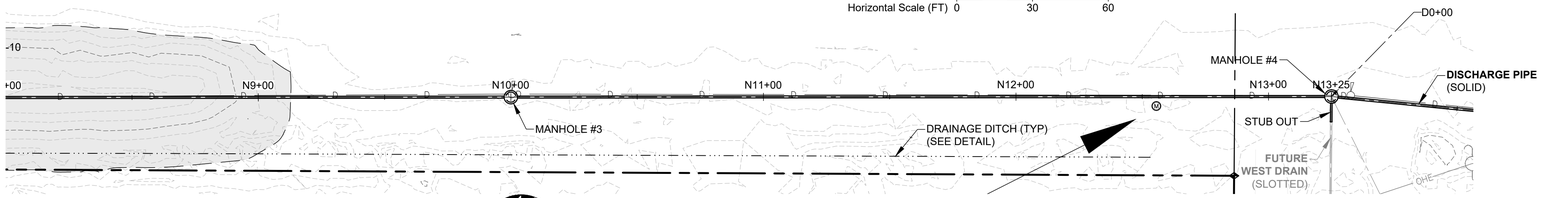


NORTH DRAIN PLAN (STA 0+00 TO 8+00)

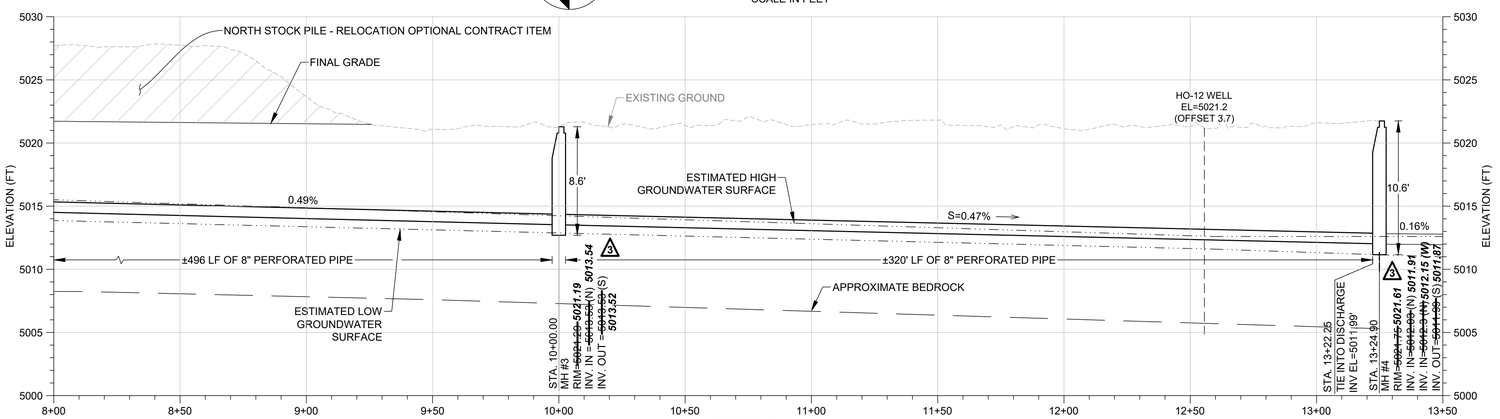
0 30 60
SCALE IN FEET



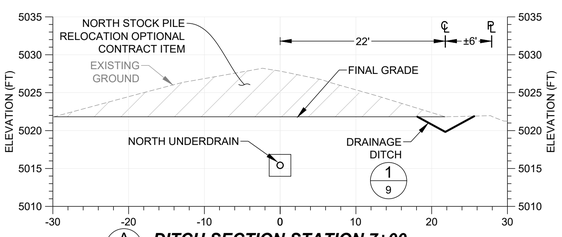


NORTH DRAIN PLAN (STA 8+00 TO 14+00)

0 30 60
SCALE IN FEET



NORTH DRAIN PROFILE (STA 8+00 TO 13+50)



DITCH SECTION STATION 7+00

0 10 20
SCALE IN FEET

REV	DESCRIPTION	BY	DATE
1	60% DESIGN - NOT FOR CONSTRUCTION	WK	11-2-21
2	FOR CONSTRUCTION	SR	11-29-21
3	AS-CONSTRUCTED	SR	5-23-22

DESIGNED BY: ---
DRAWN BY: ---
CHECKED BY: ---
DATE: ---
STATE PROFESSIONAL ENGINEER NO. XXXXX

DEERE & AULT
A SCHNABEL ENGINEERING COMPANY
600 S. AIRPORT RD., BLDG. A, SUITE 205
LONGMONT, CO 80503
TEL 303.651.1468 FAX 303.651.1469

HOME OFFICE MINE
UNDERDRAIN AS-CONSTRUCTED

NORTH DRAIN LINE PLAN AND PROFILE

PROJECT: DA494019.00
DATE: ----
SHEET 7 OF 9

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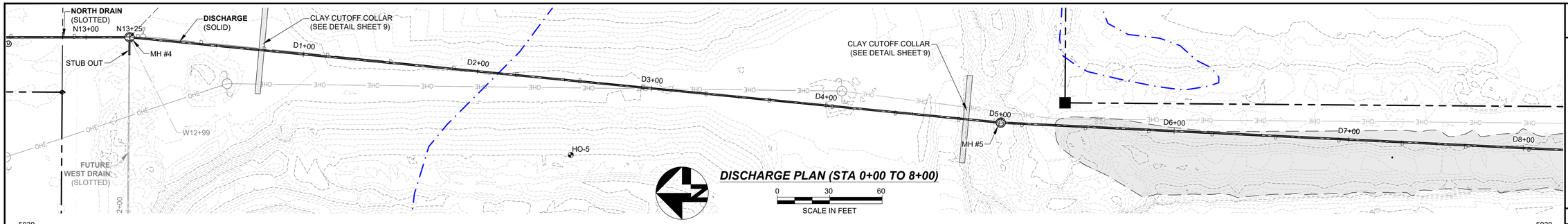
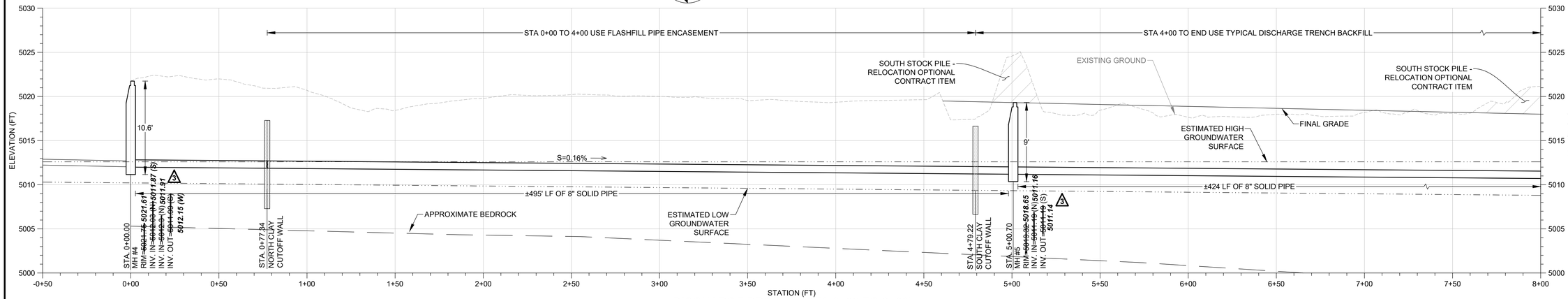


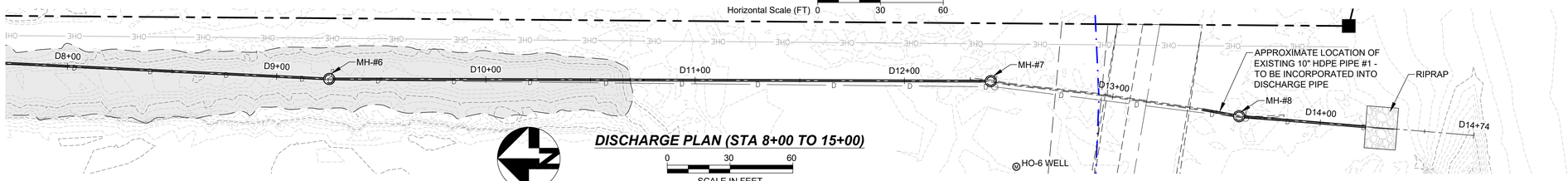
DISCHARGE PLAN (STA 0+00 TO 8+00)

0 30 60
SCALE IN FEET



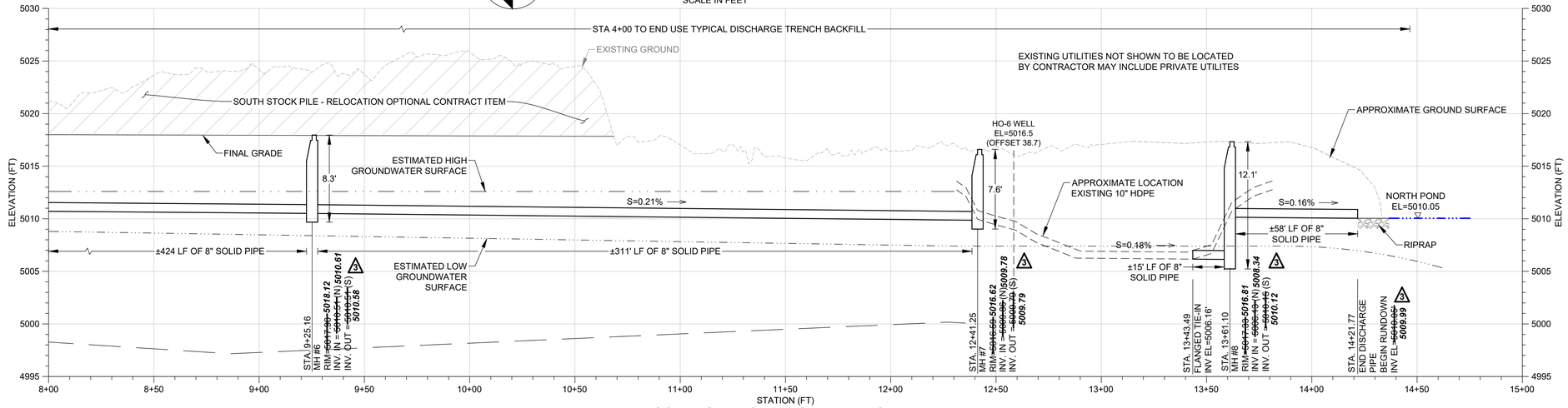
DISCHARGE PROFILE (STA 0+00 TO 8+00)

Vertical Scale (FT) 0 6 12
Horizontal Scale (FT) 0 30 60



DISCHARGE PLAN (STA 8+00 TO 15+00)

0 30 60
SCALE IN FEET



DISCHARGE PROFILE (STA 8+00 TO 15+00)

Vertical Scale (FT) 0 6 12
Horizontal Scale (FT) 0 30 60

REV	DESCRIPTION	BY	DATE
1	60% DESIGN - NOT FOR CONSTRUCTION	WK	11-2-21
2	FOR CONSTRUCTION	SR	11-29-21
3	AS-CONSTRUCTED	SR	5-23-22

DESIGNED BY: ---
DRAWN BY: ---
CHECKED BY: ---

DATE: ---
STATE PROFESSIONAL ENGINEER NO. XXXXX

BHOONESS 5/24/2022 2:08:43 PM

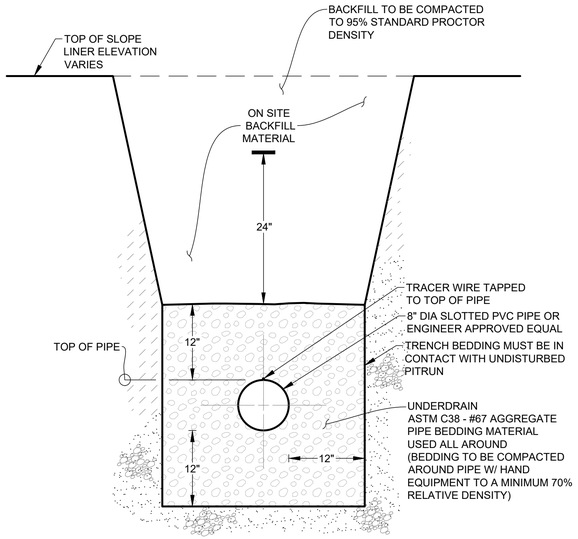
DEERE & AULT
A SCHNABEL ENGINEERING COMPANY
600 S. AIRPORT RD., BLDG. A, SUITE 205
LONGMONT, CO 80503
TEL 303.651.1468 FAX 303.651.1469

HOME OFFICE MINE
UNDERDRAIN AS-CONSTRUCTED

DISCHARGE PIPE PLAN AND PROFILE

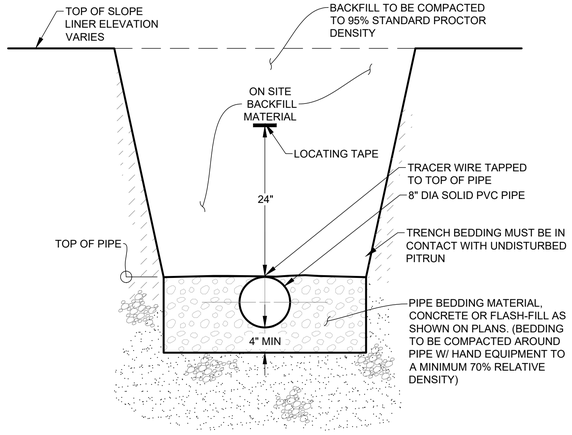
PROJECT: DA494019.00
DATE: ----
SHEET 8 OF 9

U:\0494 MARTIN MARIETTA\0494.019 HOME OFFICE MINE\CAD\WORKING\UNDERDRAIN\HO UNDERDRAIN AS-CONSTRUCTEDASB_HO UNDERDRAIN_PIPE PROFILES.DWG



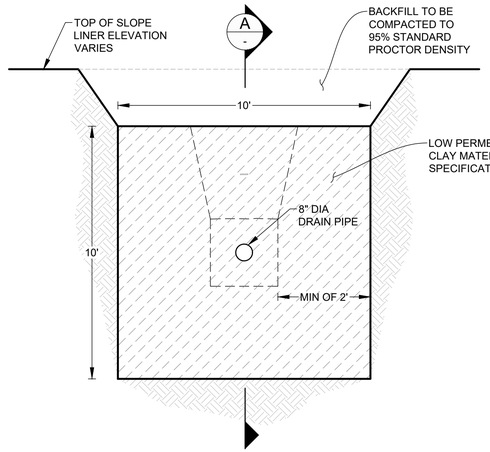
TYPICAL SLOTTED UNDERDRAIN PIPE TRENCH DETAIL

0 1 2
SCALE IN FEET



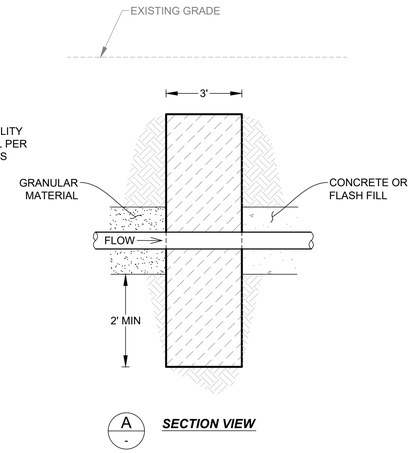
TYPICAL DISCHARGE PIPE TRENCH DETAIL

0 1 2
SCALE IN FEET



CLAY CUTOFF WALL

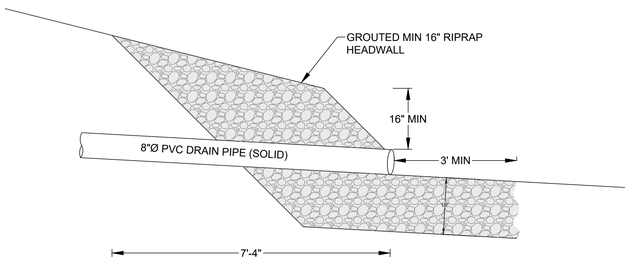
0 3 6
SCALE IN FEET



SECTION VIEW

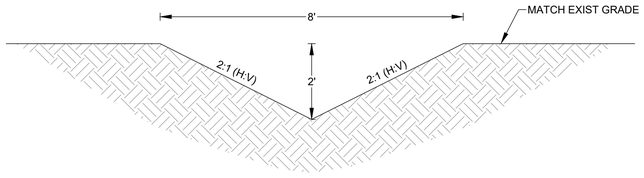
NOTE:

ALL TRENCHES SHALL BE SLOPED, BRACED, OR SHEETED IN CONFORMANCE WITH O.S.H.A. STANDARDS AND TO ACCOMMODATE EASEMENT WIDTH AND EXISTING STRUCTURES.



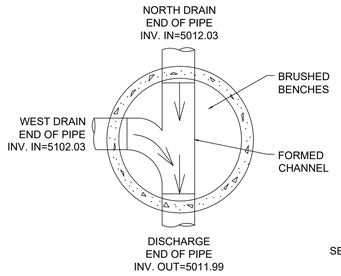
DRAIN PIPE DISCHARGE

0 2 4
SCALE IN FEET



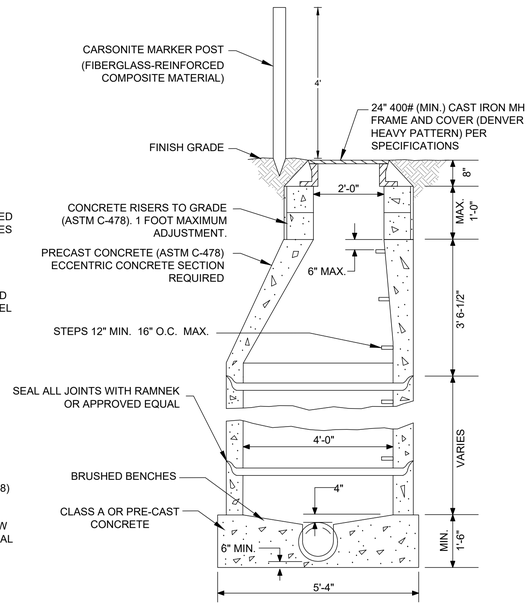
1/9 *TYPICAL DRAINAGE DITCH DETAIL*

0 2 4
SCALE IN FEET



NOTES:

1. IF MANHOLES ARE LOCATED IN OPEN FIELDS, THEY SHALL BE LEFT SIX (6) INCHES TO EIGHT (8) INCHES ABOVE GRADE WHERE PRACTICAL.
2. FLAT TOP SECTIONS MAY BE USED ON SHALLOW LINES WHERE CONE SECTIONS ARE IMPRACTICAL TO USE AS SPECIFIED BY THE DEVELOPMENT ENGINEERING MANAGER.
3. MANHOLE INVERTS SHALL BE FORMED AS INDICATED BELOW TO ENSURE SMOOTH FLOW THROUGH MANHOLE.
4. WATERPROOFING IS REQUIRED WHERE GROUNDWATER IS PRESENT.



STANDARD MANHOLE

REV	DESCRIPTION	BY	DATE
1	60% DESIGN - NOT FOR CONSTRUCTION	WK	11-2-21
2	FOR CONSTRUCTION	SR	11-29-21
	AS-CONSTRUCTED	SR	5-23-22

CHECKED BY: ---
DRAWN BY: ---
DESIGNED BY: ---
DATE: ----
STATE PROFESSIONAL ENGINEER NO. XXXXX

DEERE & AULT
A SCHNABEL ENGINEERING COMPANY
600 S. AIRPORT RD., BLDG. A, SUITE 205
LONGMONT, CO 80503
TEL 303.651.1468 FAX 303.651.1469

HOME OFFICE MINE
UNDERDRAIN AS-CONSTRUCTED

DETAILS

PROJECT: DA494019.00
DATE: ----
SHEET
9 OF 9

Martin Marietta

Julie Mikulas
Regional Land Manager

January 10, 2023

Ms. Angela Myers
Larimer County Clerk and Recorder
200 West Oak Street
Fort Collins, CO 80521

RE: Home Office Pits, revised pages for 112 Regular Construction Materials Reclamation Permit Amendment Application, County Copy of Public Notice Documents

Dear Ms. Myers:

Attached are revised pages to the 112(c) application to the Colorado Division of Reclamation, Mining, and Safety for the operation known as the Home Office Pits. This information has been provided to the Colorado Division of Reclamation, Mining, and Safety as part of the permit application process and are to be available for public review until the amendment is approved.

If you have any questions or concerns, please contact me at (970) 407-3661.

Sincerely,

Julie Mikulas

Julie Mikulas
Regional Land Manager

The public notice documents were received on the following date: January 11, 2023

By: Dianne Cheney

LARIMER COUNTY CLERK SEAL COLORADO

Rocky Mountain Division – Northern Office
1800 N Taft Hill Road, Fort Collins, CO 80534
julie.mikulas@martinmarietta.com
www.martinmarietta.com