

February 7, 2023

Mr. Brock Bowles Division of Reclamation, Mining and Safety 1313 Sherman Street, Room 215 Denver, CO 80203

RE: Adequacy Questions 3, DRMS AM-4 application - Home Office Mine, Permit M1977-439

Dear Mr. Bowles:

This letter is in response to your Adequacy Questions 3 letter, received via electronic copy on January 24, 2023 regarding Martin Marietta's 112c Permit Amendment Application (AM4) for the Home Office Mine (File No. M-1977-439) that was submitted to you on November 18, 2022. Please find below our responses to the Adequacy Questions 3:

6.4.7 EXHIBIT G - Water Information

36. Please update Exhibit G to include all the revisions proposed in the AM4 adequacy responses to ensure consistency.

Response

We have updated Exhibit G to include the revisions proposed in in the Adequacy Review 2 cover letter.

39. Martin Marietta has agreed to evaluate how each pond will safely convey the expected 100-year flood event throughout the life of the mine including final reclamation and will submit a technical revision to address this issue.

Ponds E1 and E2 are the only ponds that need this evaluation done as they are still under construction. All other ponds within the current permit boundary are either outside the 100-year floodplain (Figure G-4 of Exhibit G) or are considered pre-1981 by DWR (SWSP, January 17, 2023).

Please commit to submitting a technical revision by August 28, 2023 to evaluate how Pond E1 and E2 will safely convey the expected 100-year flood event throughout the life of the mine including final reclamation.

Response

Martin Marietta will submit a technical revision by August 28, 2023 to evaluate how Pond E1 and E2 will safely convey the expected 100-year flood event throughout the life of the mine including final reclamation.

- 40. The following questions are for the water information submitted with the AM4 application and refer to Stage G: Discussion of why the two wells shown in Exhibit G downgradient of phases G1 and G2 (identified as 246541, and 49917-F) are not likely to be impacted by shadowing from lined reservoirs.
 - a. Please clarify that MMM is committed to collecting groundwater data from each well on a monthly basis and submitting the data to DRMS on a quarterly basis.

Response

Martin Marietta is not committed to collecting groundwater data from 246541 and 49917-F on a monthly basis. Martin Marietta does not have legal access to the KOA wells. We will be collecting data for HO-06 and submitting the data to DRMS on a quarterly basis and this monitoring well is near 246541 and 49917-F. As previously stated, we received no complaints while we were dewatering. The wells are near the Cache la Poudre River and the river largely controls the water levels and will not be impacted by the

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installation of the liners. We also should note that there are three unlined ponds between the wells and the river and our underdrain along the west side of G-I and the north side of G-II also feeds into these ponds.

b. Exhibit G was submitted in the last adequacy response and did not contain any updates from the last adequacy response. Please update Exhibit G to include all the responses contained in the last several adequacy responses.

Response

This language is incorporated into the attached version of Exhibit G under Section 2.2 Monitoring.

d. The response contained the following 2 sentences: 1) If the data indicates that there is no reason to believe the plaintiff's well was impacted by the lined reservoirs that will conclude the action taken by Martin Marietta, and 2) Consequently, if a complaint is received and the water level data are in the range of the existing data set, the complaint will be judged as unreasonable, and this conclusion will be reported to the DRMS.

It is the responsibility of MMM to report any and all complaints to DRMS and it is the responsibility of DRMS to make the determination if the well was impacted by the lined reservoir. Please remove these sentences from the response and incorporate the remainder of the response into Exhibit G.

Response

The text in Section 2.3 of Exhibit G has been modified, as requested.

e. The response contained the following sentence: In the instance of a complaints that a well has gone dry, if the well monitoring data available indicates that it is reasonable to think the cause of the well going dry may be related to the lining of the reservoirs, Martin Marietta commits to implementing the mitigation measures outlined in the Groundwater Mitigation Plan within 7 days of receipt of the complaint or as soon as is practically possible.

It is the responsibility of MMM to report any and all complaints to DRMS and it is the responsibility of DRMS to make the determination if the well was impacted by the lined reservoir and determine mitigation measures. Please remove this sentence from the response and incorporate the remainder of the response into Exhibit G

<u>Response</u>

The text in Section 2.3 of Exhibit G has been modified, as requested.

f. MMM has committed to installing a drain if there is a groundwater mounding issue on the north side of GI, but only a rough description of the drain and no trigger level for the drain installation were submitted for review. Please submit a drain design for review and a commitment to install the drains if groundwater level is within 4 feet of the ground surface for more than 2 consecutive months.

<u>Response</u>

Martin Marietta will design a drain for the north side of G-I and submit it to the DRMS by May 31, 2023 as a Technical Revision. The drain will be installed if groundwater level is within 4 feet of the ground surface for more than 2 consecutive months.

g. The text states that as-constructed drawings for drains on the west side of G1 and the north side of G2 are included. The Attachment A drawing in Exhibit G shows future drawings for the North side of G2. Has G2 been installed? Also, please commit to installing the drains if groundwater level is within 4 feet of the ground surface for more than 2 consecutive months.

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Response

The drains on the north side of G-II and west side of G-I were both installed in 2022. A copy of installed drain west of G-I has been added to Exhibit G, Attachment 2 with the other underdrain design drawings. Because both of these drains are already installed, there is no need to modify any of the text in Exhibit G to commit to installing these drains within 4 feet of the ground surface for more than 2 consecutive months.

6.4.7 EXHIBIT L - Reclamation Costs

41. A The Division accepts MMM cost estimate for the Ponds G1 and G2 expansion.

Response

Noted

Please note that the decision date for this application is February 27, 2023. This date has been set by the MLRB on January 18, 2023 as allowed by Rule 1.4.1(9).

Response

Noted

All corrected pages must also be provided to Larimer County Clerk & Recorder.

Response

A copy of this letter and Attachments A has been submitted to the Larimer County Clerk and Recorder. Documentation that it was submitted to the County is attached to this letter as Attachment B.

Thank you for your consideration. If you have any questions or need additional information, please contact me at 720-864-4507, pam.hora@tetratech.com.

Sincerely,

TETRA TECH

Pamela Franch Hora, AICP

Senior Planner

Attachment A: Exhibit G, revised February 2023

Attachment B: Documentation showing Cover Letter and Attachment A is on file at Larimer County

cc: Julie Mikulas, Martin Marietta

Pamela Franch Hora

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EXHIBIT G: WATER INFORMATION

Martin Marietta is amending the existing 112 Reclamation Permit No. M-1977-439 to change the final reclamation for Area G of the Home Office site from one open water lake to two sealed water storage reservoirs using compacted clay embankment liners.

1.0 INTRODUCTION AND BACKGROUND

Martin Marietta owns properties known as the "Home Office" site in Larimer County, Colorado. The properties are located on the west and east sides of North Taft Hill Road, approximately ½ mile south of Larimer County Road 54G, in Sections 33 and 34 of Township 8 North, Range 69 West of the 6th Principal Meridian, and Sections 3 and 4 of Township 7 North, Range 69 West of the 6th Principal Meridian.

This Groundwater Monitoring and Mitigation Plan presents the methods for monitoring groundwater during mining and reclamation, and for mitigating any potential groundwater impacts associated with permitted mining at the site. Martin Marietta is applying to the Colorado Division of Reclamation, Mining and Safety (DRMS) for an Amendment to the existing 112 Reclamation Permit No. N-1977-439 to change the final reclamation for Area G of this site from one open water lake to two sealed water storage reservoirs using a compacted clay embankment liner.

Exhibit B shows the location of the Affected Area and Area G. Exhibits F1 and F2 show all the Affected Area in the permit. Exhibit F3 details Area G, which is located in the northern portion of the Affected Area. The changes within the Affected Area are limited to Area G. Consequently, this discussion is limited to potential changes in the hydrologic balance as a result of the installation of compacted clay embankment liners in Area G-I and Area G-II. Figure G-1, enclosed, shows the Affected Area; Area G; adjacent parcels to Area G and property owners; and conceptual groundwater flows before and after the installation of the compacted clay liners.

1.1 HISTORIC USE

Area G mining and associated dewatering began in Spring 2016 and is currently underway. A compacted clay liner was installed in Area G-I in September 2020 upon completion of mining. A compacted clay liner will be installed in Area G-II upon completion of mining.

Figure G-1 shows the adjacent properties to Area G, lined cells, unlined cells, cells that will be lined in the future, and irrigation ditches in the vicinity of the Affected Area. Agricultural land uses are located north and east of Area G. Loveland Ready Mix owns properties to the north and east of Area G. Irrigation ditches in the area include the New Mercer Canal, the Larimer County Canal No. 2 Ditch, the Arthur Canal, and Taylor & Gill Lateral (owned by Martin Marietta).

Loveland Ready Mix irrigates fields north and east of Area G outside of the permit area. Irrigation field tiles are reported to have been installed in the properties owned by Constance A Fredman located southeast of Area G of the Affected Area (Figure G-2). After reports of poor drainage, the field tiles were repaired by Loveland Ready Mix. Flooding was reported to Martin Marietta on the PKR Farm LLC property located east of the Affected Area (Figure G-2) as part of the field tile issue. In response, Martin Marietta worked with Loveland Ready Mix and PKR Farm LLC and installed drainage ditches between Loveland Ready Mix and PKR Farms LLC and along the east side of Area G-I. The drainage ditches convey irrigation runoff from the area east of the Affected Area to the south to the Cache La Poudre River, similar to the historic drainage patterns. Since the installation of the ditches, there have not been further reports of flooding.

Based on discussions with Seaworth Properties LLC, water is rising on the east side of a parcel owned by Seaworth Properties LLC, on the west side of Area G-I during the irrigation season. Martin Marietta is working with Seaworth Properties LLC and installed a groundwater perimeter drain between January 10, 2022 and

February 4, 2022 to address the groundwater rise and irrigation return flow to the river. Figure F-3 presents the location and design drawings for the perimeter drain (See Section 2.1 Mining Plan). An additional section of perimeter drain was installed along the north side of Area G-II. This section will connect to the installed perimeter drain at the northeast corner of Area G-II. A swale was installed on Martin Marietta's property on the west side of Area G-I to facilitate surface drainage for landowners located west of Area G-I. Attachment 2, Sheet 5 of 9 shows the approximate locations of the perimeter drain. An additional drain will be installed on the north side of G-I if groundwater levels are within 4' of the ground surface for more than two consecutive months. The design of the drain will be prepared and submitted as a Technical Revision by May 31, 2023.

Chart 1 illustrates the mining cells in the vicinity of the Affected Area and the liner status of each cell.

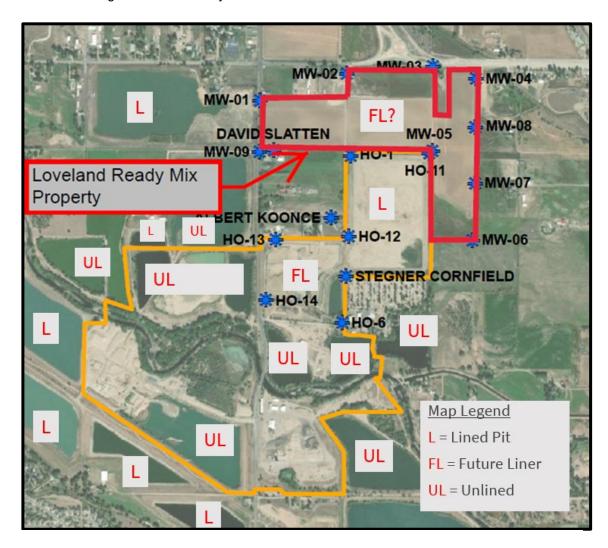


Chart 1: Mining Cells in the Vicinity & Liner Status

1.2 EXISTING WELLS

1.2.1 Monitoring Wells

Six monitoring wells (HO-1, 6, 11, 12, 13 and 14) were installed outside the limits of proposed mining, allowing for groundwater monitoring to occur during and after mining.

Monitoring well HO-1 was installed in the northwest corner of Area G-I. Monitoring well HO-6 was installed in the southeast corner of Area G-II. Both HO-1 and HO-6 were installed in 2018. Monthly groundwater monitoring for wells HO-1 and HO-6 began in May 2018. The well monitoring program documented pre-mining groundwater levels near the project area and seasonable fluctuations.

The following monitoring wells were installed in Area G in 2021:

- HO-11, in the northeast corner of Area G-I
- HO-12, near the southern portion of Area G-I, and near the northeast corner of Area G-II
- HO-13, near the northwest corner of Area G-II
- HO-14, near the southwest corner of Area G-II

Monthly groundwater monitoring for wells HO-11, HO-12, HO-13, and HO-14 began in May 2021. Exhibit F3, dated August 2021, and Figure G-3 show the locations of the wells.

Chart 1 shows water level measurements for HO-1 and HO-6. Chart 2 shows water level measurements for HO-11, HO-12, HO-13, and HO-14. Exhibit G, Attachment 1 shares the raw data for wells HO-1, HO-6, HO-11, HO-12, HO-13, and HO-14.

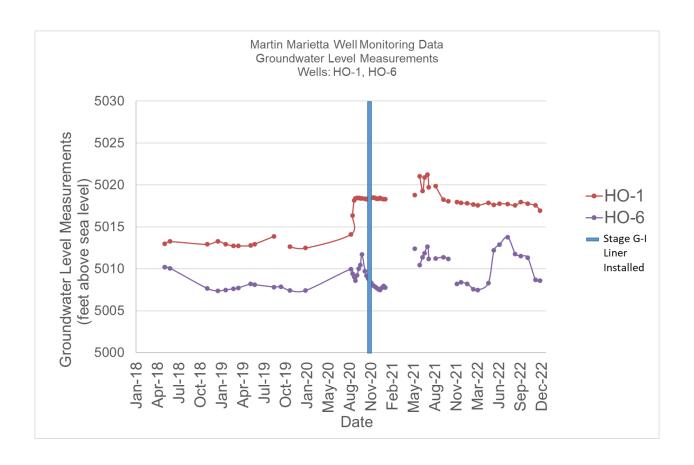


Chart 2: Martin Marietta Monitoring Data (HO-1 and HO-6)

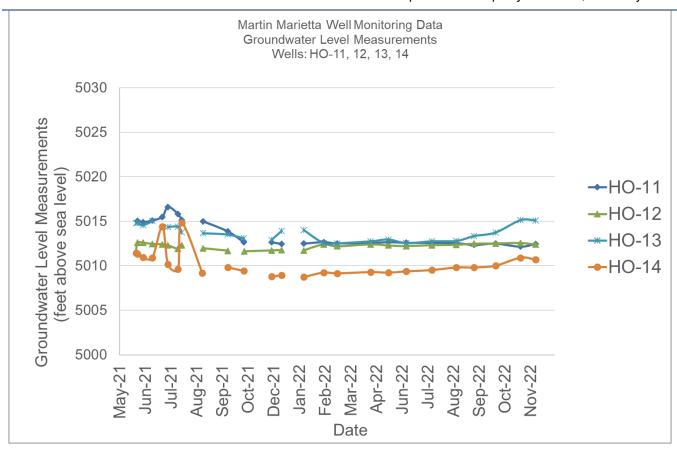


Chart 3: Well Monitoring Data (HO-11, 12, 13, 14)

Loveland Ready Mix installed monitoring wells on their lands in July 2016. Chart 4 and 5 show water level measurements that have been collected monthly since the wells were installed. Monitoring wells MW-02, MW-03 and MW-04 are located north of Area G-I. Monitoring wells MW-05, MW-06, MW-07 are located east of Area G-I. Groundwater monitoring for monitoring wells MW-02 through MW-07 began in July 2016. Groundwater levels fluctuate throughout the seasons. Exhibit G, Attachment 1 presents data provided by Loveland Ready Mix.

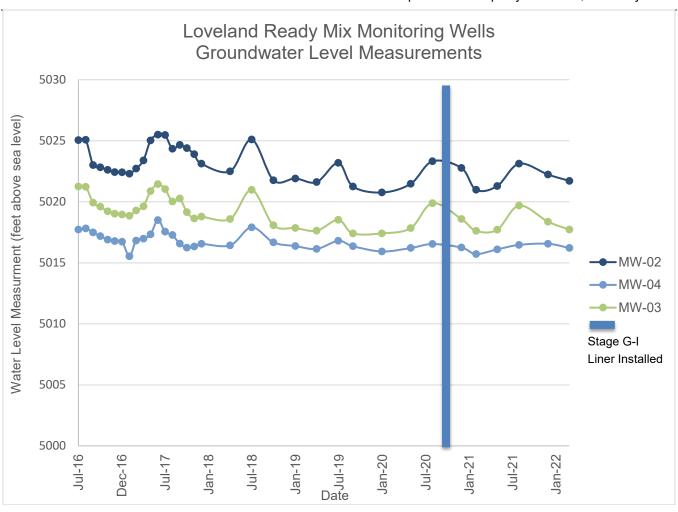


Chart 4: Loveland Ready Mix Monitoring Wells, MW-02, 03, 04

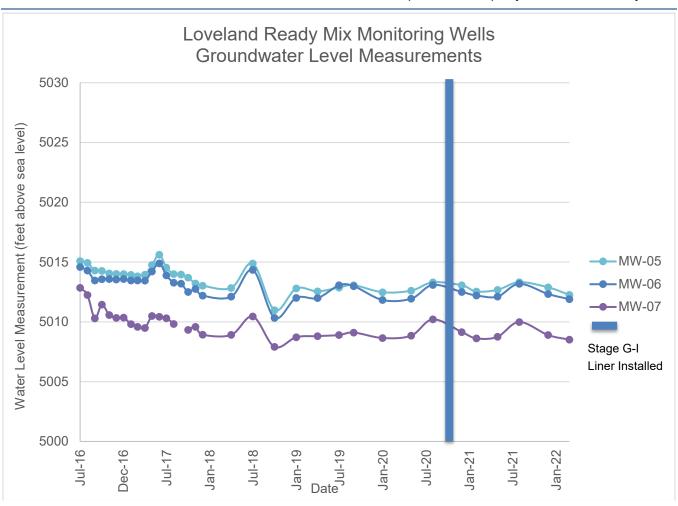


Chart 5: Loveland Ready Mix Monitoring Wells, MW-05, 06, 07

1.2.2 Nearby Wells

Numerous wells were installed in 2001 to establish groundwater baselines to monitor the groundwater conditions before, during, and after mining per the 2005 Well Monitoring Program Interim Report for Home Office. Water level measurements from 2004 and 2005 are included in the 2005 Well Monitoring Program Interim Report Well locations are shown on Figure G-3. Well owners, David L and Virginia S Slatten's property is located west of Area G-I at the northernmost portion of Area G-I (Figure G-1). Well Owners, Albert R and Sharon E Koonce's property is located west of Area G-I, just north of Area G-II. A well owned by Martin Marietta, referred to as "MM near KOA" is located east of Area G-II and south of Area G-I. Groundwater monitoring for the Slatten, Koonce and MM near KOA wells began in April 2007. Groundwater levels fluctuate throughout the seasons. The range of water level measurements for the Slatten well is 6.6 feet. The range of water level measurements for the Koonce well is 8.5 feet. The range of water level measurements for the Koonce well is 7.7 feet. Table 1 presents the maximum, minimum and average saturated thickness for the monitoring well data set. Mining and associated dewatering began in Spring 2016. A shift in water level measurements is shown in Chart 3, which presents historical well monitoring data from the Slatten, Koonce and MM near KOA wells. Exhibit G, Attachment 1 presents raw data for the nearby wells.

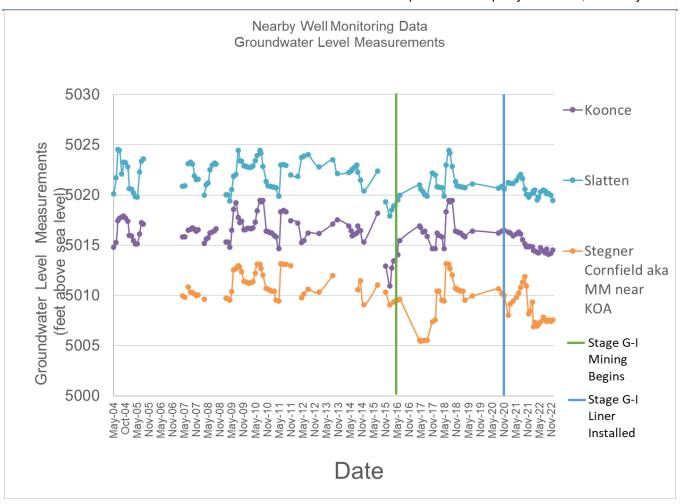


Chart 6: Nearby Well Monitoring Data

1.3 WELL INVENTORY

In December 2021, a well inventory of the Affected Area and adjacent areas was conducted to identify domestic wells near the project site. The inventory involved a review of constructed well records on file with the Colorado Division of Water Resources, located within ½ mile plus 200 feet of the Affected Area. The well inventory identified 98 constructed wells within ½ mile plus 200 feet, of the Affected Area. Figure G-2 enclosed shows the Affected Area and the constructed well locations on file with the Colorado Division of Water Resources.

1.4 HISTORIC AND FUTURE GROUND WATER LEVELS, AND IMPACTS TO WELLS

The presence of lined and unlined cells near Area G and previous dewatering of Area G demonstrates that the revised reclamation plan, i.e. lining of Area G cells will result in changes, but manageable changes, to the hydrologic balance.

The principal change to the hydrologic balance will be mounding on the upgradient (west) side of Area G. Martin Marietta has been preemptive in addressing the mounding on the east side by installing a perimeter drain. Exhibit G, Attachment 2 presents the location and design of the perimeter drain.

Regarding the down gradient impacts, monitoring well data demonstrates minimal impacts to wells will occur on the down gradient shadow effect (east and southeast) sides of Area G. Table 1 presents the historic range of

saturated thickness in the monitoring wells. The period or record for the data includes the time when the Area G cells were being actively dewatered. The impacts from dewatering are greater than the shadow effect of the lined cells. Monitoring wells HO-6 and Stenger Cornfield/MM Near KOA are adjacent to the dewater cells yet they had minimum statured thickness of 6.9 feet and 4.6 feet respectively. Chart 7 presents the hydrograph of wells HO-6 and the Stegner Cornfield/MM Near KOA wells. The impacts of dewatering and subsequent recovery of the water levels after installation of the liner in the northern Area G cell.

The saturated thickness in the shadow zone after installation of the liner will be on the order of five to six feet. Water wells completed in sand and gravel aquifers typically provide approximately 25 to 30 gallons per minute per foot of drawdown or saturated thickness in the well. The wells on the down gradient side of Area G are domestic with permitted maximum pumping rates of 15 gallons per minute (gpm). Consequently, five to six feet of saturated thickness will provide the allowed pumping rates of 15 gpm. Wells located further from Area G will have even more saturated thickness and hence will be able to pump the permitted rates.

Table 1: Saturated Thickness

Saturated Thickness (feet)											
Well	Minimum	Maximum	Delta	Average							
HO-1	5.8	14.6	8.8	10.4							
HO-6	6.9	13.3	6.4	8.9							
HO-11	9.7	14.2	4.5	11.1							
HO-12	6.4	7.5	1.0	7.0							
HO-13	5.8	8.4	2.6	7.0							
HO-14	3.2	9.3	6.1	4.7							
Koonce	5.5	14.0	8.51	10.7							
Slaten	7.9	14.5	6.57	11.5							
Stegner Cornfield/MM Near KOA	4.6	12.4	7.7	9.3							
MW-01	9.1	16.8	7.7	11.4							
MW-02	11.8	16.5	4.7	14.0							
MW-03	10.4	14.4	4.0	12.1							
MW-04	11.5	14.5	3.0	12.7							
MW-05	7.5	12.1	4.6	10.0							
MW-06	6.3	10.9	4.6	9.0							
MW-07	8.9	13.9	4.9	10.7							
MW-08	6.8	16.0	9.2	10.5							
MW-09	6.0	12.8	6.8	8.2							

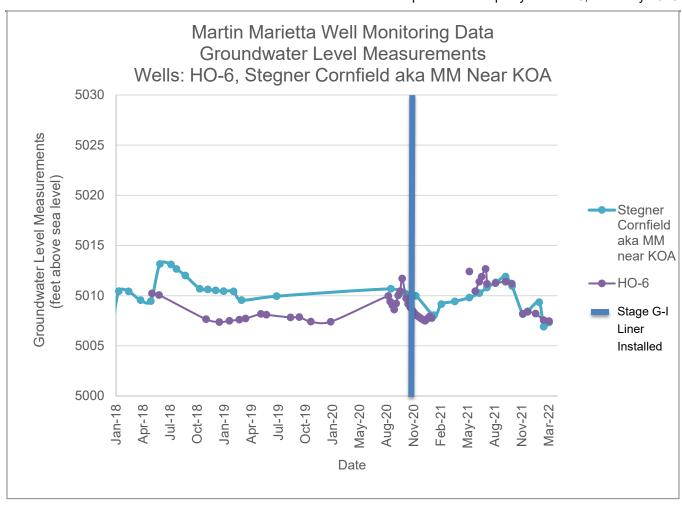


Chart 7: Monitoring Well Data (HO-6, Stegner Cornfield aka MM Near KOA)

2.0 MONITORING AND MANAGEMENT

2.1 MINING PLAN

The Reclamation Plan has been designed to reduce potential groundwater impacts to adjacent properties. A perimeter drain was designed by Deere and Ault to mitigate groundwater rise and to allow irrigation return flow off of the properties being irrigated west of Area G-I. The perimeter drain is installed along the west Area G-I. An additional portion of perimeter drain is proposed along the north side of Area G-II. Mining is expected to be completed in Area G-II by the end of 2022. The portion of the perimeter drain north of Area G-II will be installed upon completion of mining. The perimeter drain discharges through the berm between Area G-II and II to an unlined pond that is tributary to the Cache la Poudre River.

2.2 MONITORING

Martin Marietta will collect groundwater data from each well on a monthly basis and submit ground water level monitoring reports to the DRMS on a quarterly basis. The reports will include a map of showing the wells that Martin Marietta has legal access to along with information about the water levels measured in those wells. Where available, the data reports will include the historic water level measurements. The data will be provided in tabular format and as hydrographs of water level elevation versus time.

2.2.1 Martin Marietta Monitoring Wells

Monthly water level monitoring at HO-1, HO-6, HO-11, HO-12, HO-13, and HO-14 will continue during mining. When mining is complete, quarterly water level monitoring at HO-1, HO-6, HO-11, HO-12, HO-13, and HO-14 will continue until reclamation is complete and the DRMS releases the financial warranty bond.

2.2.2 Domestic Water Wells

Monthly water level monitoring at the Slatten and Koonce wells will continue during mining. When mining is complete, quarterly water level monitoring at the Slatten and Koonce wells will continue until reclamation is complete and the DRMS releases the financial warranty bond.

2.3 MITIGATION

The available monitoring well data will be used to identify changes in alluvial groundwater flow associated with mining and reclamation activities. Baseline data collected from the monitoring program will provide a range of relative water levels associated with pre-mining groundwater conditions. These data will be utilized to evaluate the nature and extent of the change to the prevailing hydrologic balance and if necessary, provide for the development of corrective actions.

In the event of a well owner complaint, Martin Marietta will report to the DRMS any complaints received from well owners within 48 hours, will investigate the complaint as soon as practical, and will submit the results to the DRMS for evaluation within 30 days.

For the investigation, the first level of response will be to review water level data from the monitoring well network and, if available, a measurement of the water level in the plaintiff's well. The information will be evaluated to determine if there is a reason to believe the plaintiff's complaint may be tied to the lined reservoirs. If the data does not clearly show there is no impact, as a second level of response, Martin Marietta will present a contract to the well owner that requests access to the well to perform a mechanical and electrical inspection and testing of the well and associated system, e.g. pressure tank. The agreement will explain that if the problem with the well is not due to a lower water level and is instead due to a mechanical or electrical issue, the well owner will be responsible for the repairs. If the well is determined to be in good working order and the problem is due to a lower water level, then the mitigation steps outlined in the previously submitted Groundwater Mitigation Plan will

be implemented. The report will include discussions with any well owner who has contacted Martin Marietta regarding a concern and a review of baseline data from the well and vicinity to evaluate whether changes may be due to seasonal variations, climate, mining, or other factors. The report will identify the extent of potential or actual impacts associated with the changes. If the extent of groundwater changes due to mining or reclamation activities is determined to be a significant contributing factor that has or may create adverse impacts, the mining-associated impacts will be addressed to the satisfaction of the DRMS.

Martin Marietta is regularly collecting and tracking the water level data from the wells that they have legal access to. The existing data collected to date demonstrates that the water levels in surrounding wells have not been materially impacted by dewatering or the installation of the Stage G1 liner. If the water levels are two feet lower than the existing data set, the mitigation plan will be activated and that plan of action will be reported to the DRMS.

If the DRMS determines that the impact on a well for which temporary mitigation has been initiated is not a result of Martin Marietta's activities, or is not solely a result of Martin Marietta's activities, Martin Marietta will reduce or cease mitigation accordingly.

If a well goes dry due to mining or reclamation activities, Martin Marietta will implement mitigation measures as soon as practical. Mitigation measures would include providing a temporary alternative water supply that meets the documented historic well production or need, until further investigation can be conducted to determine if the well condition is due to the mining operation. Martin Marietta will investigate the status of the complaint and report any corrective actions and the results of the investigation, as well as any proposed remediation or rationale for discontinuing mitigation, to the DRMS for approval within 30 days.

Martin Marietta will begin to implement one or more mitigation measures if mining or reclamation activity is determined to be a significant contributing factor to groundwater changes requiring mitigation.

Temporary mitigation measures may include, but are not limited to:

- Compensation for well owners to use their existing treated water system to replace the well production loss
- Provide a water tank and deliver water as necessary to meet documented historic well production or need.
- Other means acceptable to both the well owner and Martin Marietta.

Long-term mitigation measures may include, but are not limited to:

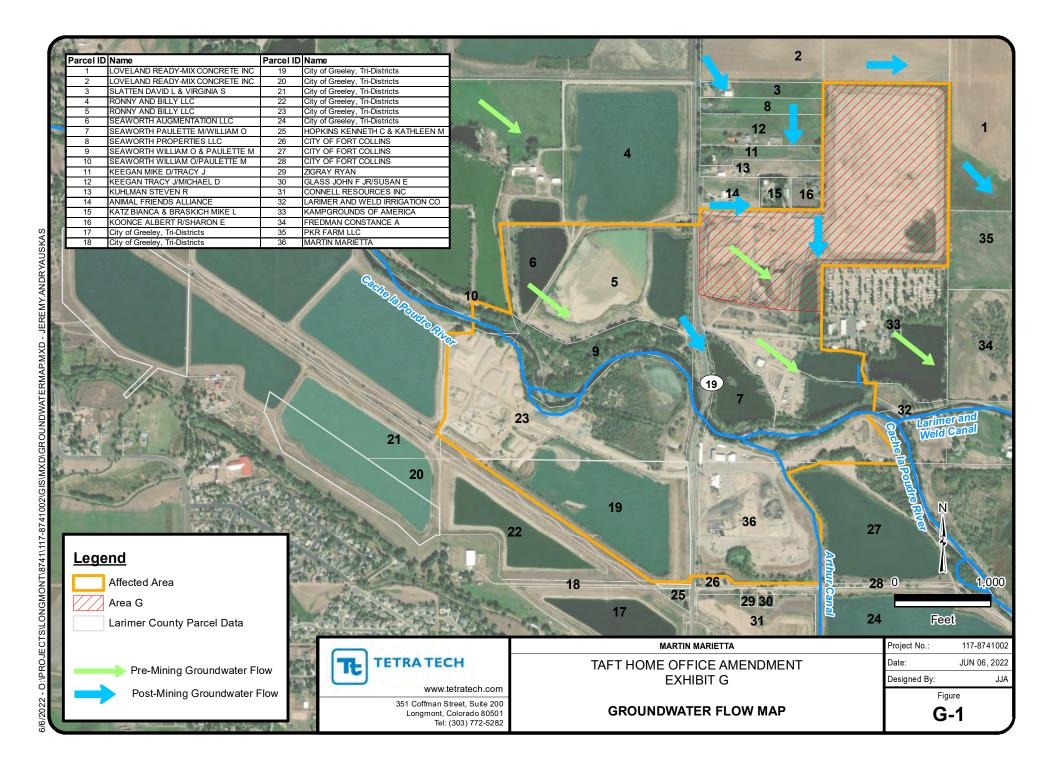
- Cleaning a well to improve efficiency.
- Providing an alternative source of water or purchasing additional water to support historic well use with respect to water quantity and quality. If needed, water quality parameters will be checked in affected wells to ensure alternative sources support the historic use.
- Modifying a well to operate under lower groundwater conditions. This could include deepening existing
 wells or lowering the pumps. All work would be done at Martin Marietta's expense with the exception of
 replacing equipment that was non-functional prior to mining.
- If existing wells cannot be retrofitted or repaired, replacing the impacted well with a new replacement well.
- Design and installation of a cistern.

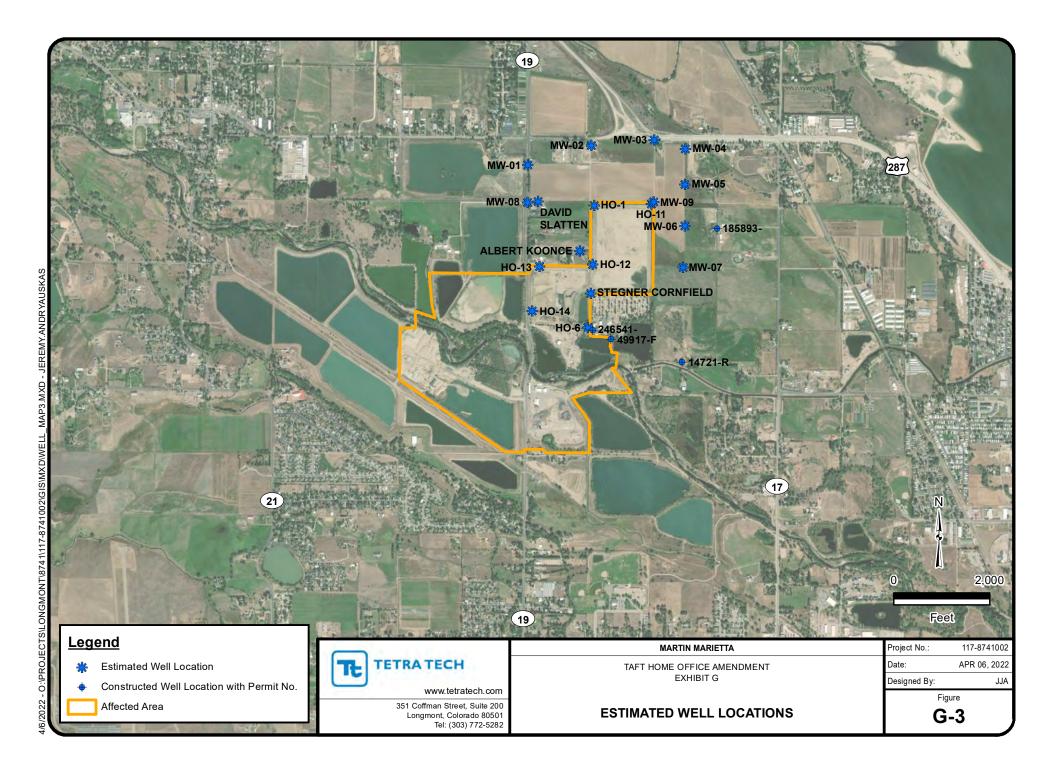
If a groundwater mitigation action is required, Martin Marietta will notify the DRMS of the condition, action taken and report the results and present a plan for monitoring the mitigation.

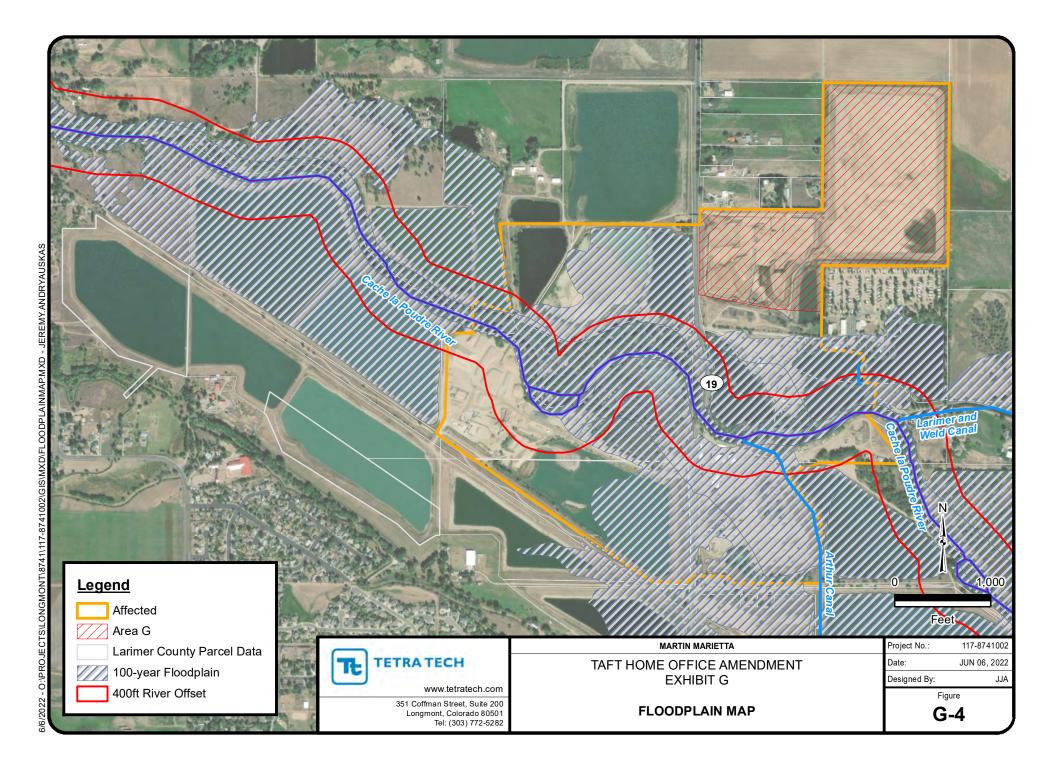
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FIGURES







ATTACHMENT 1 RAW MONITORING WELL DATA

Home Office DRMS Amendment

			HOMI	E OFFICE MONITO	RING WEL	LS (COMBIN	IED MEASUI	REMENTS P	ROVIDED B	BY DEERE & A	ULT AND M	ARTIN MAR	IETTA)					
Monitoring Well Name		HO-1		1	HO-6	•		HO-11			HO-12		,	HO-13			HO-14*	
Well Location	1	1469960, 310928	32	14673	379, 3109131	1469954, 3110451		14	68699, 3109239			1468646, 310812	23		1467725, 31079	80		
Top of PVC Casing Elevation (ft.)		5024.08			5019.39			5021.36			5023.53			5026.66 5023.		5023.84		
Ground Elevation of Well (ft.)		5021.21			5016.45			5018.43			5021.17			5023.74 5021.58		5021.58		
PCV Stickup (approx.)		2.87			2.94			2.93			2.36			2.92 2.20		2.20		
Bottom Elevation of Well (ft.)		4996.21			4987.45			5004.43			5004.17			5008.74			5004.53	
Estimated Bedrock Elevation (ft)		5006.71			5000.45			5002.43			5005.17			5006.74			5005.58	
Total Well Depth Ground-Bottom (ft.)		25.00			29.00			14.00			17.00			15.00			17.00	
	Water Depth	Saturated	Ground Water		Saturated	Ground Water	Water Depth	Saturated	Ground Water	Water Depth from	Saturated	Ground Water	Water Depth	Saturated	Ground Water	Water Depth	Saturated	Ground Water
Date	from Top of	Thickness	Elevation (ft.)	PVC	Thickness	Elevation (ft.)	from Top of	Thickness	Elevation (ft.)	Top of PVC	Thickness	Elevation (ft.)	from Top of	Thickness	Elevation (ft.)	from Top of	Thickness	Elevation (ft.)
Date	PVC (ft.)			(ft.)			PVC (ft.)			(ft.)			PVC (ft.)			PVC (ft.)		
							(10.)						(10.)			(10)		
Monday, May 7, 2018	11.08	6.29	5013.00	9.17	9.77	5010.22												
Friday, June 1, 2018	10.82	6.55	5013.26	9.33	9.61	5010.06												
Tuesday, November 13, 2018	11.15	6.22	5012.93	11.74	7.20	5007.65												
Sunday, December 30, 2018	10.78	6.59	5013.30	12.03	6.91	5007.36												1
Sunday, February 3, 2019	11.12	6.25	5012.96	11.90	7.04	5007.49												\bot
Sunday, March 10, 2019	11.35	6.02	5012.73	11.79	7.15	5007.60				ļ								ļ
Monday, April 1, 2019	11.35	6.02	5012.73	11.67	7.27	5007.72				ļ								
Saturday, May 25, 2019	11.31	6.06	5012.77	11.21	7.73	5008.18												
Thursday, June 13, 2019	11.12	6.25	5012.96	11.29	7.65	5008.10												+
Friday, September 6, 2019	10.20	7.17	5013.88	11.56	7.38	5007.83												
Monday, October 7, 2019	44.45	5.92	=0.40.00	11.53	7.41 6.96	5007.86												
Saturday, November 16, 2019 Saturday, January 25, 2020	11.45 11.60	5.92	5012.63 5012.48	11.98 11.99	6.95	5007.41												
		7.41			9.52	5007.40												
Friday, August 14, 2020	9.96 7.71	9.66	5014.12 5016.37	9.42 9.98	8.96	5009.97 5009.41												+
Friday, August 21, 2020 Friday, August 28, 2020	5.91	11.46	5016.37	9.98	8.60	5009.41												
Friday, August 26, 2020 Friday, September 4, 2020	5.70	11.67	5018.38	10.34	8.15	5009.00												+
Friday, September 11, 2020	5.63	11.74	5018.45	10.19	8.76	5009.21												+
Friday, September 18, 2020	5.64	11.73	5018.44	9.36	9.58	5010.03												+
Friday, September 15, 2020	5.67	11.70	5018.41	8.95	9.99	5010.44												+
Friday, October 2, 2020	5.70	11.67	5018.38	7.69	11.25	5011.70												+
Friday, October 16, 2020	5.73	11.64	5018.35	9.66	9.28	5009.73												+
Friday, October 23, 2020	5.79	11.58	5018.29	10.22	8.72	5009.17												+
Friday, October 30, 2020	5.68	11.69	5018.40	10.55	8.39	5008.84												+
Friday, November 6, 2020	5.61	11.76	5018.47	10.83	8.11	5008.56												+
Friday, November 13, 2020	5.61	11.76	5018.47	11.10	7.84	5008.29												+
Monday, November 23, 2020	5.60	11.77	5018.48	11.37	7.57	5008.02												+ -
Monday, November 30, 2020	5.64	11.73	5018.44	11.51	7.43	5007.88												+ -
Monday, December 7, 2020	5.73	11.64	5018.35	11.66	7.28	5007.73												+
Tuesday, December 15, 2020	5.73	11.64	5018.35	11.80	7.14	5007.59												\top
Tuesday, December 22, 2020	5.62	11.75	5018.46	11.90	7.04	5007.49												
Tuesday, December 29, 2020	5.73	11.64	5018.35	11.70	7.24	5007.69												
Thursday, January 7, 2021	5.79	11.58	5018.29	11.45	7.49	5007.94												
Thursday, January 14, 2021	5.78	11.59	5018.30	11.63	7.31	5007.76												
Monday, May 24, 2021										11.93	6.43	5011.60	11.85	8.07	5014.81	12.30	5.86	5011.43
Wednesday, May 26, 2021	5.3	12.07	5018.78	6.98	11.96	5012.41	6.3	12.63	5015.06	10.90	7.46	5012.63	11.83	8.09	5014.83	12.40	5.76	5011.33
Thursday, June 3, 2021							6.45	12.48	5014.91	10.91	7.45	5012.62	12.10	7.82	5014.56	12.79	5.36	5010.94
Wednesday, June 16, 2021	3.04	14.33	5021.04	8.95	9.99	5010.44	6.28	12.65	5015.08	11.05	7.31	5012.48	11.62	8.30	5015.04	12.84	5.32	5010.89
Wednesday, June 30, 2021	4.80	12.57	5019.28	8.00	10.94	5011.39	5.87	13.06	5015.49	11.10	7.26	5012.43				9.30	8.85	5014.43
Thursday, July 8, 2021	3.17	14.20	5020.91	7.51	11.43	5011.88	4.76	14.17	5016.60	11.22	7.14	5012.31	12.30	7.62	5014.36	13.58	4.57	5010.15
Thursday, July 22, 2021	2.82	14.55	5021.26	6.73	12.21	5012.66	5.54	13.39	5015.82	11.60	6.76	5011.93	12.26	7.66	5014.40	14.08	4.07	5009.65
Tuesday, July 27, 2021	4.35	13.02	5019.73	8.21	10.73	5011.18	6.21	12.72	5015.15	11.2	7.16	5012.33	12.9	7.02	5013.76	8.97	9.29	5014.87
Wednesday, August 25, 2021		40.45			40.00			40.50								14.63	3.63	5009.21
Thursday, August 26, 2021	4.22	13.15	5019.86	8.15	10.79	5011.24	6.37	12.56	5014.99	11.54	6.82	5011.99	13	6.92	5013.66		4.00	
Thursday, September 30, 2021	5.83	11.54	5018.25	8	10.94	5011.39	7.5	11.43	5013.86	11.83	6.53	5011.70	13.15	6.77	5013.51	14	4.26	5009.84
Friday, October 22, 2021	6.01	11.36	5018.07	8.2	10.74	5011.19	8.7	10.23	5012.66		6.40	F04 1 00	13.57	6.35	5013.09	14.4	3.86	5009.44
Saturday, October 23, 2021		11.27	5047.00		7.74	E000 40		10.00	E040.00	11.9	6.46 6.56	5011.63	40	6.18	5040.00	45	3.24	5000.00
Tuesday, November 30, 2021	6.1	11.27	5017.98	11.2	1.14	5008.19	8.7	10.23	5012.66	11.8	0.56	5011.73	13.74	0.18	5012.92	15.02	3.24	5008.82

Home Office DRMS Amendment Exhibit G, Attachment 1

			HOM	E OFFICE MONITO	RING WEL	LS (COMBIN	ED MEASUR	REMENTS F	ROVIDED B	Y DEERE & AL	JLT AND M	ARTIN MAR	IETTA)					-		
Monitoring Well Name		HO-1			HO-6			HO-11			HO-12			HO-13			HO-14*			
Well Location	1	1469960, 310928	82	1467	379, 3109131		1	469954, 311045	1	14	68699, 3109239	ı	1468646, 3108123			1	1467725, 3107980			
Top of PVC Casing Elevation (ft.)		5024.08			5019.39			5021.36			5023.53		5026.66			5023.84				
Ground Elevation of Well (ft.)		5021.21			5016.45			5018.43		5021.17			5023.74			5021.58				
PCV Stickup (approx.)		2.87			2.94			2.93			2.36			2.92		2.20				
Bottom Elevation of Well (ft.)		4996.21			4987.45			5004.43			5004.17			5008.74			5004.53			
Estimated Bedrock Elevation (ft)		5006.71			5000.45			5002.43			5005.17		5006.74		5006.74		5006.74		5005.58	
Total Well Depth Ground-Bottom (ft.)		25.00			29.00			14.00			17.00			15.00		17.00				
Date	Water Depth from Top of PVC (ft.)	Saturated Thickness	Ground Water Elevation (ft.)	Water Depth from Top of PVC (ft.)	Saturated Thickness	Ground Water Elevation (ft.)	Water Depth from Top of PVC (ft.)	Saturated Thickness	Ground Water Elevation (ft.)	Water Depth from Top of PVC (ft.)	Saturated Thickness	Ground Water Elevation (ft.)	Water Depth from Top of PVC (ft.)	Saturated Thickness	Ground Water Elevation (ft.)	Water Depth from Top of PVC (ft.)	Saturated Thickness	Ground Water Elevation (ft.)		
Tuesday, December 14, 2021							8.9	10.03	5012.46	11.75	6.61	5011.78	12.75	7.17	5013.91	14.9	3.36	5008.94		
Friday, December 17, 2021	6.21	11.16	5017.87	11	7.94	5008.39												1		
Friday, January 14, 2022	6.26	11.11	5017.82	11.18	7.76	5008.21	8.87	10.06	5012.49	11.8	6.56	5011.73	12.62	7.30	5014.04	15.1	3.16	5008.74		
Friday, February 11, 2022	6.4	10.97	5017.68	11.84	7.10	5007.55	8.7	10.23	5012.66	11.1	7.26	5012.43	14.1	5.82	5012.56	14.58	3.68	5009.26		
Wednesday, March 2, 2022	6.52	10.85	5017.56	11.93	7.01	5007.46	8.83	10.10	5012.53	11.33	7.03	5012.20	14.13	5.79	5012.53	14.67	3.59	5009.17		
Monday, April 18, 2022	6.22	11.15	5017.86	11.10	7.84	5008.29	8.80	10.13	5012.56	11.10	7.26	5012.43	13.90	6.02	5012.76	14.53	3.73	5009.31		
Friday, May 13, 2022	6.44	10.93	5017.64	7.20	11.74	5012.19	8.70	10.23	5012.66	11.23	7.13	5012.30	13.70	6.22	5012.96	14.59	3.67	5009.25		
Tuesday, June 7, 2022	6.30	11.07	5017.78	6.50	12.44	5012.89	8.77	10.16	5012.59	11.30	7.06	5012.23	14.10	5.82	5012.56	14.45	3.81	5009.39		
Wednesday, July 13, 2022	6.35	11.02	5017.73	5.60	13.34	5013.79	8.79	10.14	5012.57	11.20	7.16	5012.33	13.90	6.02	5012.76	14.30	3.96	5009.54		
Tuesday, August 16, 2022	6.52	10.85	5017.56	7.61	11.33	5011.78	8.81	10.12	5012.55	11.15	7.21	5012.38	13.86	6.06	5012.80	14.01	4.25	5009.83		
Saturday, September 10, 2022	6.12	11.25	5017.96	7.85	11.09	5011.54	9.02	9.91	5012.34	11.03	7.33	5012.50	13.31	6.61	5013.35	14.01	4.25	5009.83		
Monday, October 10, 2022	6.31	11.06	5017.77	8.05	10.89	5011.34	8.87	10.06	5012.49	11.00	7.36	5012.53	12.92	7.00	5013.74	13.81	4.45	5010.03		
Monday, November 14, 2022	6.51	10.86	5017.57	10.71	8.23	5008.68	9.21	9.72	5012.15	10.95	7.41	5012.58	11.53	8.39	5015.13	12.91	5.35	5010.93		
Monday, December 5, 2022	7.15	10.22	5016.93	10.81	8.13	5008.58	8.90	10.03	5012.46	11.12	7.24	5012.41	11.55	8.37	5015.11	13.11	5.15	5010.73		

Home Office Nearby Wells Water Level Monitoring												
LOCATION	ALBI	ERT KOONC			VID SLATTEN		STEGNER CORNFIELD/MM Near KOA					
LATTITUDE		0 37'08.6 N			40 37'18.8 N		40 36'59.8 N					
LONGTITUDE		5 06'37.4 W			05 06'48.6 W		105 06'34.5 W					
DESCRIPTION	2" CASE-BY	NORTH FE	NCELINE	2"CASE-CE	NTER NORT	H FENCE	PVC	BY RV PAR	K			
ELEVATION OF BENCHMARK		5023.4			5028.0		5018.5					
ELEVATION OF GROUND		5022.4			5027.0		5017.8					
SURFACE ESTIMATED BEDROCK		5005.4			5010.0		5000.8					
ELEVATION												
DATE	READING (Measurement from Benchmark to Water Level, ft)	WATER ELEV	SATURATED THICKNESS (ft)	READING (Measurement from Benchmark to Water Level, ft)	WATER ELEV	SATURATED THICKNESS (ft)	READING (Measurement from Benchmark to Water Level, ft)	WATER ELEV	SATURATED THICKNESS (ft)			
05/04/04	8.62	5014.82	9.4	7.88	5020.12	10.1						
06/10/04	8.15	5015.29	9.9	6.29	5021.71	11.7						
07/08/04	5.98	5017.46	12.0	3.45	5024.55	14.6						
08/03/04	5.75	5017.69	12.3	3.55	5024.45	14.4						
09/07/04	5.62	5017.82	12.4	5.91	5022.09	12.1			1			
10/04/04	5.55	5017.89	12.4	4.74	5023.26	13.3			+			
11/01/04	5.73	5017.89	12.4	4.77	5023.23	13.2			 			
				5.18					 			
12/06/04	6.04	5017.40	12.0		5022.82	12.8			 			
01/03/05	7.46	5015.98	10.5	7.36	5020.64	10.6			1			
02/07/05	7.53	5015.91	10.5	7.42	5020.58	10.6			ļ			
03/07/05	7.96	5015.48	10.0	7.77	5020.23	10.2						
04/04/05	8.31	5015.13	9.7	8.16	5019.84	9.8						
05/02/05	8.3	5015.14	9.7	8.22	5019.78	9.8						
06/07/05	7.33	5016.11	10.7	5.7	5022.3	12.3						
07/06/05	6.21	5017.23	11.8	4.62	5023.38	13.4						
08/10/05	6.33	5017.11	11.7	4.42	5023.58	13.6						
01/01/07												
02/01/07												
03/01/07												
04/02/07	7.62	5015.82	10.4	7.12	5020.88	10.9	8.6	5009.95	9.1			
05/09/07	7.58	5015.86	10.4	7.1	5020.9	10.9	8.71	5009.84	9.0			
06/01/07 07/02/07	6.94	5016.50	11.1	4.89	5023.11	10.1	7.7	5010.85	10.1			
08/06/07	6.87	5016.57	11.1	4.74	5023.11	13.1 13.3	8.21	5010.83	9.5			
09/05/07	6.72	5016.72	11.3	4.92	5023.08	13.1	8.28	5010.27	9.5			
10/08/07	6.84	5016.60	11.2	6.1	5021.9	11.9	8.41	5010.14	9.3			
11/05/07	7.04	5016.40	11.0	6.48	5021.52	11.5	8.52	5010.03	9.2			
12/03/07	6.92	5016.52	11.1	6.42	5021.58	11.6	8.5	5010.05	9.3			
01/01/08												
02/01/08												
03/03/08	8.26	5015.18	9.7	8	5020	10.0	8.92	5009.63	8.8			
04/07/08 05/05/08	7.85	5015.59 5015.73	10.1 10.3	6.99 6.82	5021.01 5021.18	11.0 11.2						
06/01/08	7.71 7.23	5015.73	10.3	5.5	5021.18	12.5	1		 			
07/07/08	7.11	5016.33	10.9	4.99	5023.01	13.0						
08/11/08	6.98	5016.46	11.0	4.84	5023.16	13.2						
09/08/09	6.82	5016.62	11.2	4.92	5023.08	13.1						
10/01/08												
11/01/08												
12/01/08	-						1		 			
01/01/09 02/03/09	8.1	5015.34	9.9	7.98	5020.02	10.0	8.82	5009.73	8.9			
03/03/09	8.13	5015.34	9.9	7.98	5020.02	10.0	8.85	5009.73	8.9			
04/07/09	8.62	5014.82	9.4	8.57	5019.43	9.4	9	5009.70	8.8			
05/05/09	6.93	5016.51	11.1	7.48	5020.52	10.5	8.18	5010.37	9.6			
06/02/09	4.88	5018.56	13.1	6.15	5021.85	11.9	6	5012.55	11.8			
07/07/09	4.21	5019.23	13.8	5.94	5022.06	12.1	5.86	5012.69	11.9			
08/12/09	5.67	5017.77	12.3	3.53	5024.47	14.5	5.6	5012.95	12.1			
09/08/09	6.21	5017.23	11.8	4.58	5023.42	13.4	5.71	5012.84	12.0			
10/06/09	6.05	5017.39	11.9	4.62	5023.38	13.4	6.21	5012.34	11.5			
11/11/09 12/01/09	6.91 6.89	5016.53 5016.55	11.1 11.1	5.1 5.11	5022.9 5022.89	12.9 12.9	7.12 7.19	5011.43 5011.36	10.6 10.6			
01/01/10	6.77	5016.55	11.2	5.18	5022.89	12.8	7.19	5011.30	10.6			
2/1/2010	6.74	5016.70	11.3	5.24	5022.76	12.8	7.32	5011.23	10.4			
3/1/2010	6.78	5016.66	11.2	5.26	5022.74	12.7	7.28	5011.27	10.5			
4/1/2010	6.72	5016.72	11.3	5.11	5022.89	12.9	7.2	5011.35	10.6			
5/1/2010	6.15	5017.29	11.9	4.59	5023.41	13.4	6.31	5012.24	11.4			
6/1/2010	5	5018.44	13.0	4.05	5023.95	13.9	5.42	5013.13	12.3			

Home Office Nearby Wells Water Level Monitoring												
LOCATION		ERT KOONC		DA	VID SLATTEN		STEGNER CORNFIELD/MM Near KOA					
LATTITUDE		0 37'08.6 N 5 06'37.4 W			40 37'18.8 N 05 06'48.6 W		40 36'59.8 N 105 06'34.5 W					
LONGTITUDE DESCRIPTION	2" CASE-BY		NCELINE		NTER NORTI	H FENCE	PVC BY RV PARK					
ELEVATION OF	2 6/102 81		TOLLINE.	2 0/102 02		TTENOL	5018.5					
BENCHMARK		5023.4			5028.0			5018.5				
ELEVATION OF GROUND SURFACE		5022.4			5027.0		5017.8					
ESTIMATED BEDROCK		5005.4			5010.0		5000.8					
ELEVATION						I			I			
DATE	READING (Measurement from Benchmark to Water Level, ft)	WATER ELEV	SATURATED THICKNESS (ft)	READING (Measurement from Benchmark to Water Level, ft)	WATER ELEV	SATURATED THICKNESS (ft)	READING (Measurement from Benchmark to Water Level, ft)	WATER ELEV	SATURATED THICKNESS (ft)			
7/13/2010	3.99	5019.45	14.0	3.56	5024.44	14.4	5.44	5013.11	12.3			
8/1/2010	4.12	5019.32	13.9	3.84	5024.16	14.2	5.88	5012.67	11.9			
9/1/2010	4	5019.44	14.0	5.14	5022.86	12.9	6.54	5012.01	11.2			
10/22/2010	7.03	5016.41	11.0	6.66	5021.34	11.3	7.86	5010.69	9.9			
11/19/2010 12/17/2010	7.1 7.18	5016.34 5016.26	10.9 10.8	7.09 7.14	5020.91 5020.86	10.9 10.9	7.93 8.03	5010.62 5010.52	9.8 9.7			
1/14/2011	7.18	5016.26	10.8	7.14	5020.86	10.9	8.03	5010.52	9.7			
2/18/2011	7.48	5015.96	10.5	7.10	5020.79	10.8	8.12	5010.43	9.6			
3/18/2011	7.61	5015.83	10.4	7.28	5020.72	10.7	9	5009.55	8.8			
5/3/2011	8.76	5014.68	9.2	8.1	5019.9	9.9	9.1	5009.45	8.6			
6/6/2011 7/16/2011	5.1 4.99	5018.34 5018.45	12.9 13.0	5 4.96	5023 5023.04	13.0 13.0	5.4 5.42	5013.15 5013.13	12.4 12.3			
8/19/2011	5.12	5018.43	12.9	5.04	5023.04	13.0	5.45	5013.10	12.3			
9/30/2011	-								-			
11/4/2011	6	5017.44	12.0	5.98	5022.02	12.0	5.58	5012.97	12.2			
2/17/2012	6.25 8.17	5017.19 5015.27	11.8 9.8	6.14 4.25	5021.86 5023.75	11.9 13.8	8.8	5009.75	8.9			
4/20/2012 5/25/2012	8.17	5015.27	10.0	4.25 4.14	5023.75	13.8	8.42	5009.75	9.3			
7/30/2012	7.21	5016.23	10.8	4	5024	14.0	7.93	5010.62	9.8			
1/11/2013	7.25	5016.19	10.8	5.2	5022.8	12.8	8.22	5010.33	9.5			
8/13/2013	6.34	5017.10	11.7	4.5	5023.5	13.5	6.55	5012.00	11.2			
10/23/2013 4/17/2014	5.9 6.5	5017.54 5016.94	12.1 11.5	5.85 5.75	5022.15 5022.25	12.1 12.3						
5/23/2014	7	5016.44	11.0	5.62	5022.38	12.4						
6/11/2014	7.5	5015.94	10.5	5.44	5022.56	12.6						
7/16/2014	7.38	5016.06	10.6	5.24	5022.76	12.8						
8/20/2014 9/8/2014	7.21 6.5	5016.23 5016.94	10.8 11.5	5 5.7	5023 5022.3	13.0 12.3	8	5010.55	9.8			
10/16/2014	7	5016.94	11.0	6.5	5022.5	11.5	7.1	5010.55	10.6			
12/9/2014	8.1	5015.34	9.9	7.6	5020.4	10.4	9.5	5009.05	8.3			
7/7/2015	5.26	5018.18	12.7	5.63	5022.37	12.4	7.5	5011.05	10.3			
8/18/2015	40.5	E040.04	7.5	0.7	5040.0	0.0	0.0	E040.0E	0.0			
11/12/2015 1/12/2016	10.5 12.5	5012.94 5010.94	7.5 5.5	8.7 10.1	5019.3 5017.9	9.3 7.9	8.2 9.5	5010.35 5009.05	9.6 8.3			
2/12/2016	10.7	5010.94	7.3	9.5	5017.9	8.5	5.5	5505.05	0.0			
3/12/2016	10	5013.44	8.0	9.1	5018.9	8.9	9.2	5009.35	8.6			
4/5/2016	9.97	5013.47	8.0	9	5019	9.0	9.1	5009.45	8.6			
5/15/2016 6/16/2016	9.4 8	5014.04 5015.44	8.6 10.0	8.5 8	5019.5 5020	9.5 10.0	9 8.9	5009.55 5009.65	8.8 8.9			
4/15/2017	6.5	5015.44	11.5	7	5020	11.0	13	5005.55	4.8			
5/2/2017	6.68	5016.76	11.3	7.43	5020.57	10.6	13.1	5005.45	4.6			
6/12/2017	7.12	5016.32	10.9	7.67	5020.33	10.3	13.08	5005.47	4.7			
7/7/2017	7.05	5016.39	10.9	7.99	5020.01	10.0	12.99	5005.56	4.8			
8/10/2017 11/1/2017	7.55 8.8	5015.89 5014.64	10.4 9.2	8.12 5.8	5019.88 5022.2	9.9 12.2	13 11.2	5005.55 5007.35	4.8 6.6			
12/17/2017	8.78	5014.66	9.2	6	5022.2	12.0	11.2	5007.55	6.8			
1/11/2018	7.21	5016.23	10.8	7.18	5020.82	10.8	8.1	5010.45	9.6			
2/14/2018	7.48	5015.96	10.5	7.21	5020.79	10.8	8.12	5010.43	9.6			
3/28/2018	7.61	5015.83	10.4	7.28	5020.72	10.7	9	5009.55	8.8			
5/3/2018 6/4/2018	8.76 5.1	5014.68 5018.34	9.2 12.9	8.1 5	5019.9 5023	9.9 13.0	9.1 5.4	5009.45 5013.15	8.6 12.4			
7/13/2018	3.99	5019.45	14.0	3.56	5024.44	14.4	5.44	5013.11	12.3			
8/1/2018	4.12	5019.32	13.9	3.84	5024.16	14.2	5.88	5012.67	11.9			
9/1/2018	4	5019.44	14.0	5.14	5022.86	12.9	6.54	5012.01	11.2			
10/22/2018 11/19/2018	7.03 7.1	5016.41 5016.34	11.0 10.9	6.66 7.09	5021.34 5020.91	11.3 10.9	7.86 7.93	5010.69 5010.62	9.9 9.8			
12/17/2018	7.1	5016.34	10.9	7.09	5020.91	10.9	8.03	5010.62	9.8			
1/14/2019	7.10	5016.23	10.8	7.18	5020.82	10.8	8.1	5010.45	9.6			
2/18/2019	7.48	5015.96	10.5	7.21	5020.79	10.8	8.12	5010.43	9.6			
3/18/2019	7.61	5015.83	10.4	7.28	5020.72	10.7	9	5009.55	8.8			
7/19/2019	7.05	5016.39	10.9	6.89	5021.11	11.1	8.6	5009.95	9.1			
8/23/2020	7.22	5016.22	10.8	7.32	5020.68	10.7	7.87	5010.68	9.9			

Home Office Nearby Wells Water Level Monitoring													
LOCATION	ALBI	ERT KOONO			VID SLATTEN		STEGNER COR	RNFIELD/MN	M Near KOA				
LATTITUDE	4	0 37'08.6 N			40 37'18.8 N		40	36'59.8 N					
LONGTITUDE	10	5 06'37.4 W		1	05 06'48.6 W		105 06'34.5 W						
DESCRIPTION	2" CASE-BY	NORTH FE	NCELINE	2"CASE-CE	NTER NORT	H FENCE	PVC BY RV PARK						
ELEVATION OF BENCHMARK		5023.4			5028.0		5018.5						
ELEVATION OF GROUND		5022.4			5027.0			5017.8					
SURFACE		0022.1			0020								
ESTIMATED BEDROCK		5005.4			5010.0			5000.8					
ELEVATION					00.0.0	•			,				
DATE	READING (Measurement from Benchmark to Water Level, ft)	WATER ELEV	SATURATED THICKNESS (ft)	READING (Measurement from Benchmark to Water Level, ft)	WATER ELEV	SATURATED THICKNESS (ft)	READING (Measurement from Benchmark to Water Level, ft)	WATER ELEV	SATURATED THICKNESS (ft)				
10/13/2020	7.02	5016.42	11.0	7.12	5020.88	10.9	8.35	5010.20	9.4				
11/19/2020	6.93	5016.51	11.1	7.42	5020.58	10.6	8.56	5009.99	9.2				
1/22/2021	7.13	5016.31	10.9	6.74	5021.26	11.3	10.5	5008.05	7.3				
2/17/2021	7.21	5016.23	10.8	6.85	5021.15	11.1	9.4	5009.15	8.4				
4/5/2021	7.5	5015.94	10.5	6.86	5021.14	11.1	9.12	5009.43	8.6				
5/26/2021	7.34	5016.10	10.7	6.55	5021.45	11.4	8.74	5009.81	9.0				
6/30/2021	7.13	5016.31	10.9	6.19	5021.81	11.8	8.3	5010.25	9.4				
7/27/2021	7.34	5016.10	10.7	5.93	5022.07	12.1	7.73	5010.82	10.0				
8/27/2021	7.87	5015.57	10.1	6.37	5021.63	11.6	7.23	5011.32	10.5				
9/30/2021	8.32	5015.12	9.7	7.4	5020.6	10.6	6.65	5011.90	11.1				
10/23/2021	8.6	5014.84	9.4	7.93	5020.07	10.1	7.6	5010.95	10.1				
11/30/2021	8.6	5014.84	9.4	8.2	5019.8	9.8	10.4	5008.15	7.4				
12/17/2021	8.53	5014.91	9.5	7.97	5020.03	10.0	10.11	5008.44	7.6				
1/26/2022	8.61	5014.83	9.4	7.85	5020.15	10.1	9.21	5009.34	8.5				
2/11/2022	8.96	5014.48	9.0	7.6	5020.4	10.4	11.65	5006.90	6.1				
3/2/2022	9	5014.44	9.0	7.52	5020.48	10.5	11.21	5007.34	6.5				
4/11/2022	9.15	5014.29	8.9	8.5	5019.5	9.5	11.6	5006.95	6.1				
5/1/2022	9.2	5014.24	8.8	8.2	5019.8	9.8	11.41	5007.14	6.3				
6/1/2022	8.7	5014.74	9.3	7.7	5020.3	10.3	11.2	5007.35	6.6				
7/18/2022	9	5014.44	9.0	7.5	5020.5	10.5	10.71	5007.84	7.0				
8/10/2022	9.21	5014.23	8.8	7.61	5020.39	10.4	11.01 5007.54 6.7						
9/2/2022	8.81	5014.63	9.2	7.88	5020.12	10.1	11.15	5007.40	6.6				
10/6/2022	9.32	5014.12	8.7	7.82	5020.18	10.2	10.99	5007.56	6.8				
11/11/2022	9.25	5014.19	8.8	8.01	5019.99	10.0	11.12	5007.43	6.6				
	8.91	5014.53	9.1	8.52	5019.48	9.5	11.01	5007.54	6.7				

12/5/2022 8.91 5014.53 9.1

Data Source Notes:
Coordinates provided by Martin Marietta, 2021-08-19

Elevations provided by Martin Marietta, 2021-08-19

Water level measurements provided by Martin Marietta
Ground Surface for Koonce and Slatten approximated as 1 foot below BM.

Bedrock depth estimated as 17 feet below ground surface

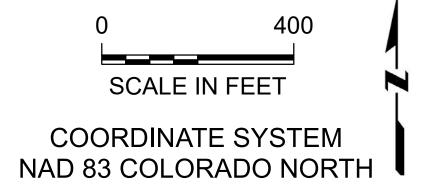
	Stegner - Monitoring Well Measurements												
			Depth T	o Water from	Surface (Fo	eet)							
Date Measured	MW-01	MW-02	MW-03	MW-04	MW-05	MW-06	MW-07	MW-08	MW-09				
July-16	2.62	0.95	1.75	2.28	2.91	2.41	3.15	6.41	2.92				
August-16	4.15	0.93	1.78	2.18	3.06	2.71	3.75	7.38	3.56				
September-16	6.89	3.00	3.08	2.52	3.70	3.53	5.70	9.28	5.09				
October-16	-0.29	3.18	3.40	2.81	3.73	3.43	4.56	9.99	5.27				
November-16	7.01	3.39	3.78	3.10	3.95	3.41	5.43	10.23	5.37				
December-16	7.21	3.57	3.98	3.22	3.99	3.45	5.66	10.56	5.45				
January-17	7.18	3.58	4.05	3.27	4.00	3.42	5.64	10.63	5.42				
February-17	7.19	3.70	4.15	4.47	4.07	3.53	6.19	10.73	5.57				
March-17	7.16	3.28	3.72	3.18	4.18	3.53	6.41	10.62	5.62				
April-17	5.56	2.61	3.38	3.03	4.05	3.55	6.51	8.95	5.53				
May-17	1.76	0.97	2.13	2.67	3.24	2.79	5.50	3.11	3.41				
June-17	0.44	0.50	1.55	1.50	2.38	2.10	5.57	1.51	2.20				
July-17	1.75	0.53	1.96	2.45	3.47	3.11	5.69	2.18	3.64				
August-17	2.60	1.66	2.98	2.73	4.00	3.72	6.17	3.07	6.25				
September-17	2.31	1.35	2.73	3.43	4.04	3.80		3.64	6.44				
October-17	2.62	1.61	3.85	3.76	4.30	4.50	6.67	4.26	7.81				
November-17	2.72	2.11	4.37	3.67	4.79	4.24	6.41	5.23	8.42				
December-17	5.63	2.88	4.22	3.45	4.97	4.80	7.07	7.18	8.57				
April-18	6.69	3.50	4.41	3.57	5.16	4.89	7.09	8.93	8.74				
July-18	2.67	0.90	2.03	2.09	3.11	2.65	5.55	4.71	6.49				
October-18	6.94	4.24	4.93	3.33	7.02	6.66	8.09	3.87	8.98				
January-19	6.14	4.09	5.15	3.63	5.21	4.99	7.28	7.85	8.84				
April-19	6.98	4.39	5.37	3.86	5.45	5.01	7.19	9.31	8.94				
July-19	5.84	2.81	4.48	3.20	5.13	3.94	7.10	6.63	8.76				
September-19	6.32	4.75	5.59	3.64	4.93	4.01	6.89	6.58	8.39				
January-20	7.43	5.24	5.59	4.07	5.52	5.17	7.35	9.59	8.97				
May-20	7.03	4.53	5.16	3.79	5.39	5.06	7.15	9.24	8.78				
August-20	4.59	2.68	3.12	3.46	4.69	3.91	5.79	4.16	7.18				
December-20	6.45	3.23	4.41	3.75	4.94	4.50	6.85	5.78	7.73				
February-21	6.69	5.02	5.38	4.28	5.46	4.79	7.38	8.07	8.56				
May-21	6.94	4.71	5.29	3.90	5.32	4.89	7.25	9.04	8.74				
August-21	4.58	2.88	3.31	3.53	4.68	3.82	6.01	4.06	7.28				
December-21	6.51	3.77	4.63	3.44	5.11	4.67	7.10	6.86	8.46				
March-22	6.96	4.29	5.28	3.78	5.74	5.10	7.49	7.61	8.98				



Legend

Stegner Monitoring Wells

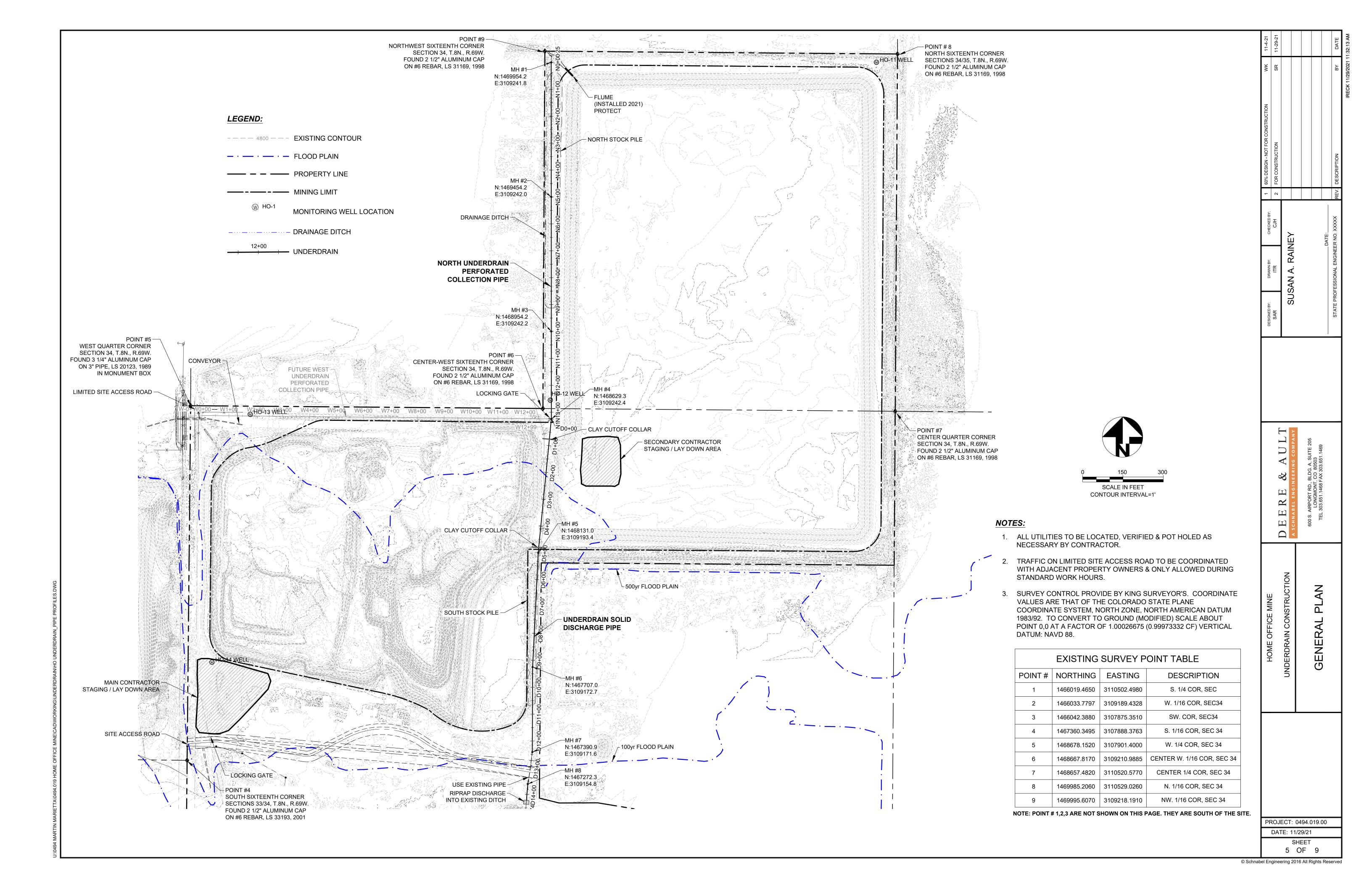
Stegner Parcels

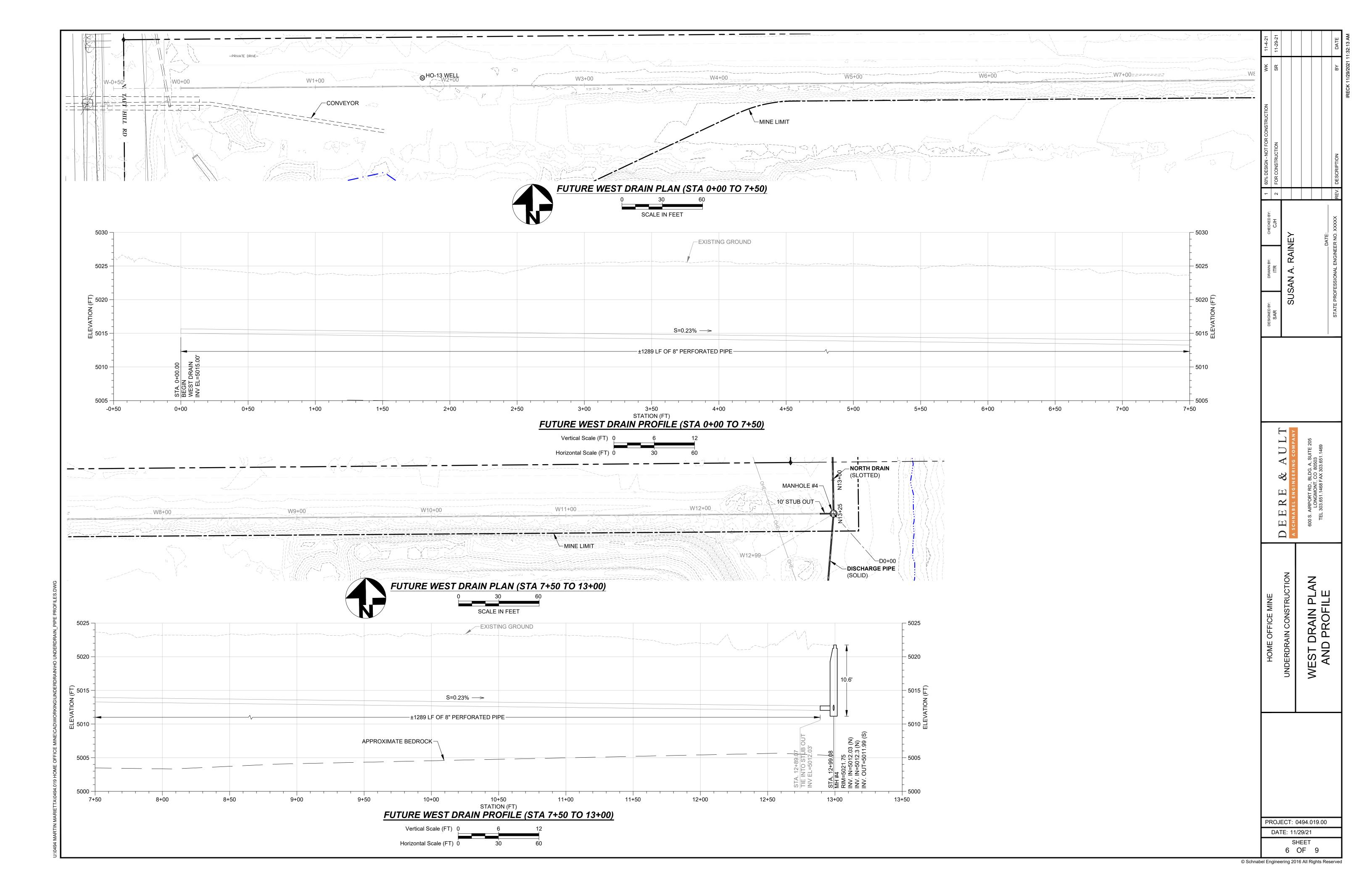


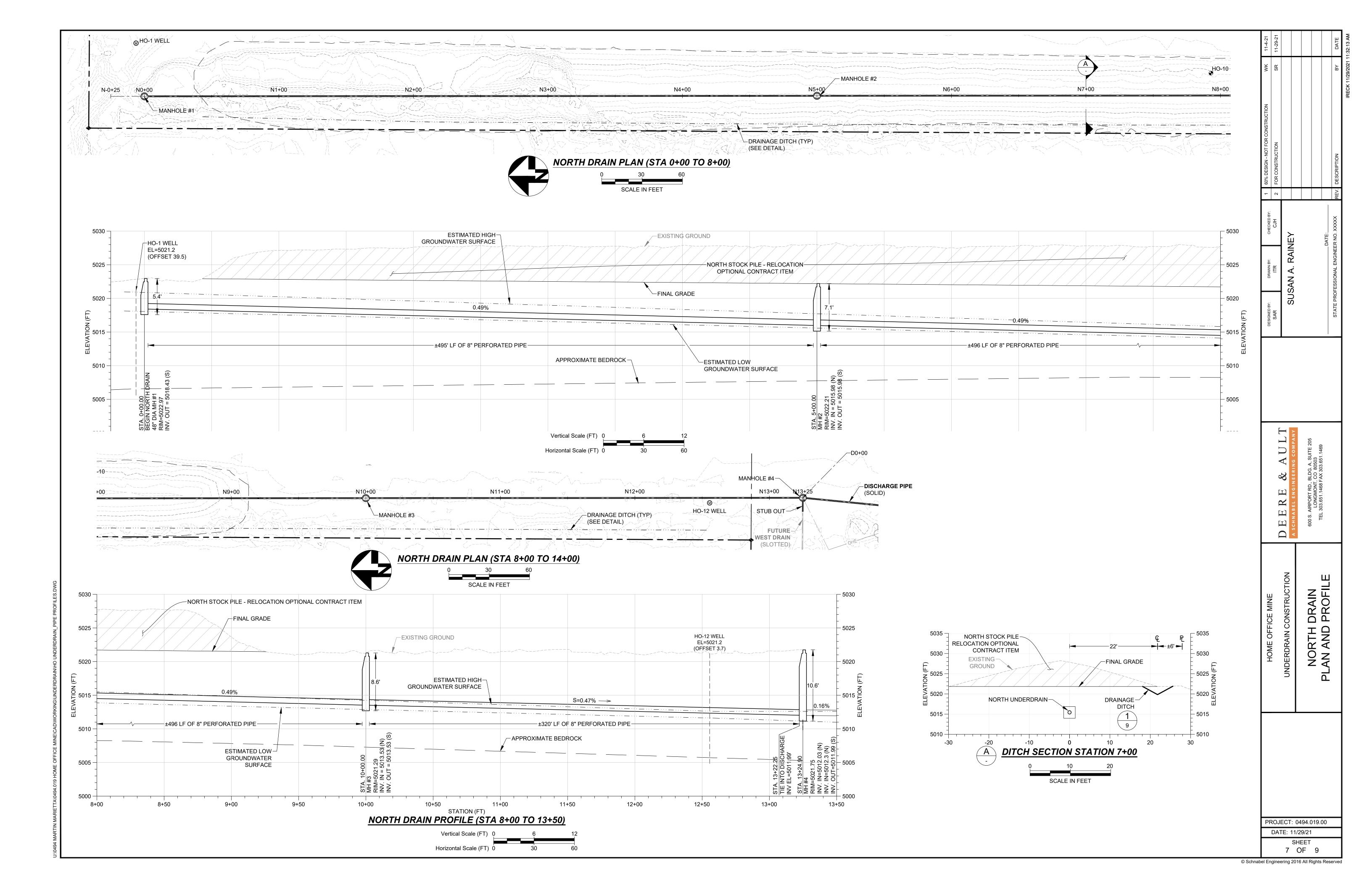
STEGNER BASEMAP

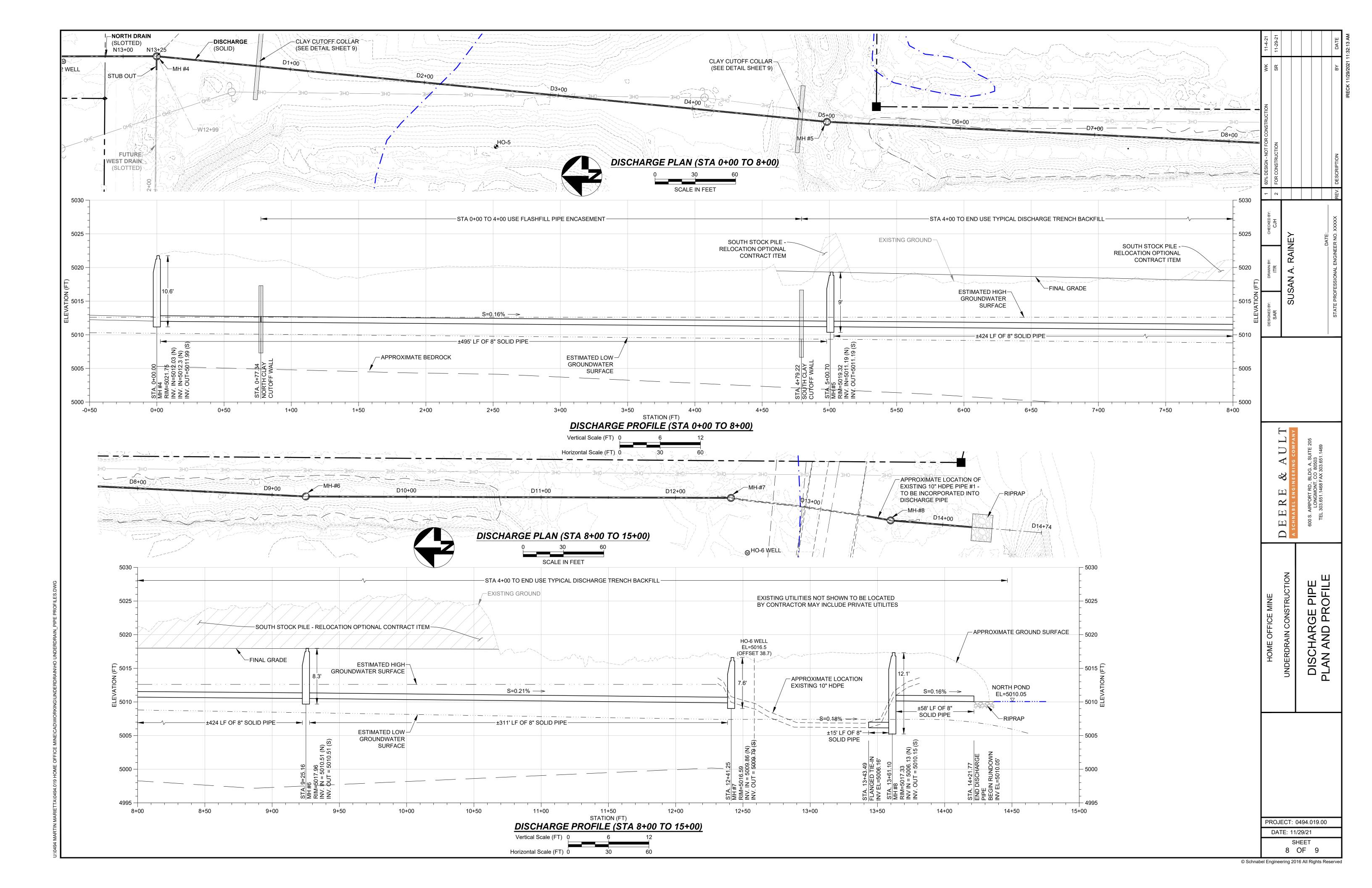


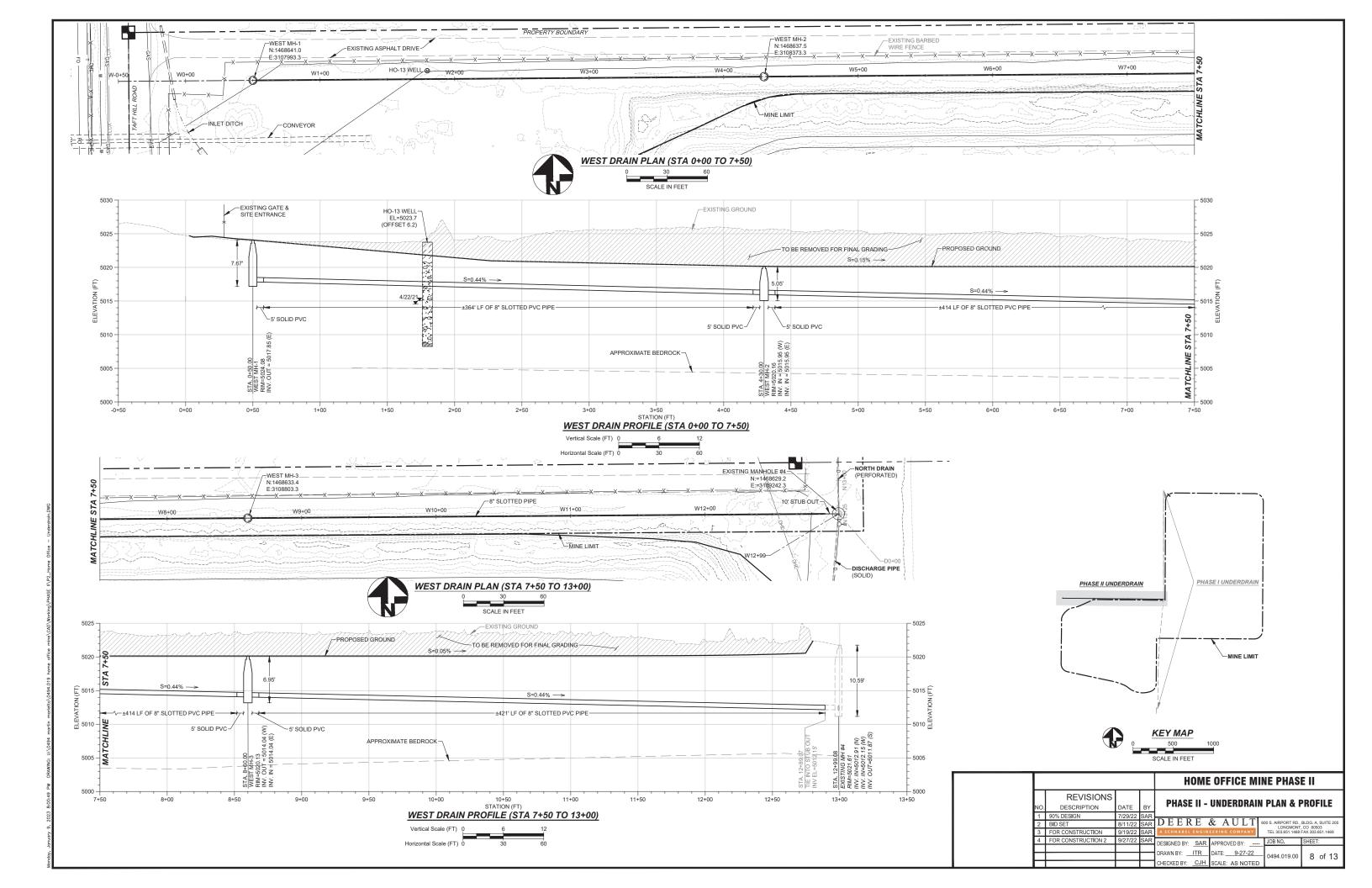
ATTACHMENT 2 UNDERDRAIN DESIGN













600 South Airport Road, Suite A-205 Longmont, Colorado 80503 T/ 303-651-1468 F/ 303-651-1469

TECHNICAL MEMORANDUM

TO: Julie Mikulas **DATE**: 6/21/2022

Britney Guggisberg

COMPANY: Martin Marietta **SUBJECT:** Underdrain Calculations

ADDRESS: 1800 North Taft Hill Road PROJECT Home Office

Fort Collins, CO 80521 **NAME/NO.:** DA494019.00

FROM: Susan A. Rainey, PE CC: Pam Hora – Tetra Tech

This document presents the calculations performed as part of the groundwater underdrain design at the Home Office Mine.

PROJECT BACKGROUND

The Home Office Mine is located in Larimer County, Colorado in the northwest portion of the city of Fort Collins. As mining progressed at the site, Martin Marietta plans to construct a slope liner in stages around the perimeter of the mined area. The slope liner and berm in the center of the site will create two cells. Phase I, or the first cell and dividing berm, was constructed from August to October in 2020. Phase II is scheduled to be constructed in approximately the same time frame this year. Martin Marietta elected to construct a groundwater collection system or underdrain to deal with possible groundwater mounding that could occur due to the liner configuration. The first section of underdrain (the north drain and discharge) was constructed at the beginning of 2022. The remainder of the underdrain (the west drain) will be constructed as part of Phase II.

CALCULATIONS

Seepage analyses were performed using Seep/W, a finite element computer model software program, to estimate the possible groundwater flow into the underdrain. The seepage analyses were performed using two different K values for the native sand and gravel. The first value ($2.54 \times 10^{-3} \, \text{cm/s}$) was selected using the NAVFAC DM 7.2 Table 1 typical coefficient of permeability for SW (well-graded sand) soil type as a guideline. This table lists a permeability of greater than 1 x $10^{-3} \, \text{ft/min}$. We selected 5 x $10^{-3} \, \text{ft/min}$ or $2.54 \times 10^{-3} \, \text{cm/s}$ for a possible lower end value, which is greater than the minimum ($1 \times 10^{-3} \, \text{ft/min}$) typical permeability for well graded sand. This value was selected due to the presence of gravel and cobble on the site. A second seepage analysis was performed with a considerably higher permeability ($1 \times 10^{-2} \, \text{cm/s}$) for a possible higher end value. This value was selected based off the typical permeability on the NAVFAC DM 7.2 Table 1 for GW (well-graded gravel) of $5 \times 10^{-2} \, \text{ft/min}$. Our value of $1.00 \times 10^{-2} \, \text{cm/s}$ or $1.97 \times 10^{-2} \, \text{ft/min}$ is less than this maximum permeability.

We performed analyses for each K value with two different groundwater heights for a range of possible flows into the underdrain system. The resulting flows were entered into FlowMaster, a general purpose 1D computational fluid dynamics simulation software, along with other design parameters (pipe size, slope, length, etc.) to calculate how full the pipe would be. An additional flow of 100 gallons per minute or more than double the highest Seep/W flow rate was entered into FlowMaster, as a high-end extreme flow. This was done for the two sections of perforated pipe (north and west drain). A cumulative flow of 200 gallons per minute for the discharge section was

Home Office Mine Underdrain Calculations
also entered into FlowMaster. The resulting percent of pipe full of flow is shown on the attached calculations summary table.
A final check of water velocity through the slots in the perforated pipe was performed for the anticipated flows to confirm the open area of the perforated pipe was sufficient for the estimated possible, and high-end extreme flows.
U:\0494 MARTIN MARIETTA\0494.019 HOME OFFICE MINE\UNDERDRAIN\UNDERDRAIN CALCULATIONS SUMMARY MEMO 6-21-2022.DOCX

Home Office Underdrain Seep W Analysis Drain pipe sizing.

North Drain

Inputs

Length: 1370 ft. - from drawings

Slope 0.0047 ft./ft.
Pipe Diame 8 in
Manning's 0.0009

Slots 1.5 in2/ft specified minimum slot area

Outputs

		Seep V			Seep W Resu	Pipe					
Sand and Gravel (k)	Model Ground Surface	Model Groundwater Elevation	Groundwater depth below ground	Drain Elevation	Drain Depth	q	Total flow	Total flow	Flow height - solved in Flowmaster	% of diameter	Water velocity through slots
cm/sec	ft.	ft.	ft.	ft.	ft.	cfs/ft.	cfs	gpm	in		ft/sec
2.54E-03	5,024	5,019	5	5017	7	8.77E-06	1.20E-02	5.392287	0.6	7.1%	8.42E-04
2.54E-03	5,024	5,021	3	5017	7	1.85E-05	2.53E-02	11.37484	0.8	10.1%	1.78E-03
1.00E-02	5,024	5,019	5	5017	7	3.50E-05	4.80E-02	21.51996	1.1	13.7%	3.36E-03
1.00E-02	5,024	5,021	3	5017	7	7.39E-05	1.01E-01	45.43786	1.6	19.6%	7.09E-03
High flow check -	100 gpm, greater t	than 200% of mod	lel			1.63E-04	2.23E-01	100	2.3	29.3	1.56E-02

West Drain

Inputs

Length: 1326 ft. - from drawings

Slope 0.0023 ft./ft.
Pipe Diame 8 in
Manning's 0.0009

Slots 1.5 in2/ft specified minimum slot area

Outputs

		Seep V	V Input			Seep W Resu	ılts		Pipe			
Sand and Gravel (k)	Model Ground Surface	Model Groundwater Elevation	Groundwater depth below ground	Drain Elevation	Drain Depth	q	Total flow	Total flow	Flow height - solved in Flowmaster	% of diameter	Water velocity through slots	
cm/sec	ft.	ft.	ft.	ft.	ft.	cfs/ft.	cfs	gpm	in		ft/sec	
2.54E-03	5,024	5,019	5	5017	7	8.77E-06	1.16E-02	5.219104	0.7	8.2%	8.42E-04	
2.54E-03	5,024	5,021	3	5017	7	1.85E-05	2.45E-02	11.00951	0.9	11.8%	1.78E-03	
1.00E-02	5,024	5,019	5	5017	7	3.50E-05	4.64E-02	20.82881	1.3	16.0%	3.36E-03	
1.00E-02	5,024	5,021	3	5017	7	7.39E-05	9.80E-02	43.97854	1.8	23.1%	7.09E-03	
High flow check - :	100 gpm, greater t	than 200% of mod	el			1.68E-04	2.23E-01	100	2.8	35.3%	1.61E-02	

Discharge

Length: 1400 ft. - from drawings

Slope 0.0012 ft./ft.
Pipe Diame 8 in
Manning's 0.0009

		Seep V	V Input			Seep W Res	ults			Pipe
Sand and Gravel (k)	Model Ground Surface	Model Groundwater Elevation	Groundwater depth below ground	Drain Elevation	Drain Depth	q	Total flow	Total flow	Flow height - solved in Flowmaster	% of diameter
cm/sec	ft.	ft.	ft.	ft.	ft.	cfs/ft.	cfs	gpm	in	
2.54E-03	5,024	5,019	5	5017	7		2.36E-02	10.61139	1.1	13.5%
2.54E-03	5,024	5,021	3	5017	7		4.99E-02	22.38435	1.6	19.4%
1.00E-02	5,024	5,019	5	5017	7		9.44E-02	42.34877	2.1	26.7%
1.00E-02	5,024	5,021	3	5017	7		1.99E-01	89.4164	3.2	39.5%
High flow check -	200 gpm, greater t	than 200% of mod	el				4.46E-01	200	5.1	63.9%

Notes:

See USBR DS-15(5) - Filter Drsign. Paragraph 5.5.2 - "Drains sould be sized so that the depth of water in the drain pipe is less than 50% of the inside diameter.

Slot velocity not specified by design standard. Calculated to check, maintin below 0.06 ft/sec, which the maximum orfice velocity at 100 gpm as calculated in Flow Master



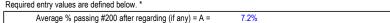
CALCULATION COVER SHEET

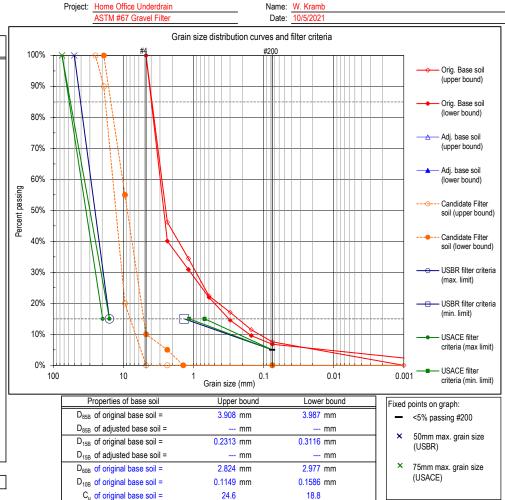
Project		Project Number
Title		
Commission Drawning Hood		Version/Release
Computer Programs Used		No.
Dumage and Objective		
Purpose and Objective		
Summary of Conclusions		
Originator		
_		
Print	Sign	Date
Checked	J	
Print	Sign	Date
1 11111	Sign	Date

Base Material

Determine the gradation curves of the base soil. Use enough samples to define the range of grain size for the base soil. Design the filter gradation based on the base soil that requires the smallest D_{15r} size. If soil has particles larger than the #4 sieve, an adjusted gradation is calculated. Input values below for the base soil (original) gradation (in red):

Particle size	gradation is calculate Sieve		nal), % passing	Adjusted grada	tion, % passing
(mm)	#	(upper bound)	(lower bound)	upper bound	lower bound
75	-				
37.5	-			(No adjustm	ont nooded)
19.0	-			(NO aujustin	ent needed)
9.5	-				
4.75	4	100.0%	100.0%		
4.00	5				
3.35	6				
2.80	7				
2.36	8	46.2%	40.1%		
2.00	10				
1.70	12				
1.40	14				
1.18	16	34.5%	30.9%		
1.00	18				
0.850	20				
0.710	25				
0.600	30	22.4%	21.9%		
0.500	35				
0.425	40				
0.300	50	17.1%	14.6%		
0.250	60				
0.212	70				
0.180	80				
0.150	100	11.5%	9.6%		
0.125	120				
0.106	140				
0.090	170				
0.075	200	7.6%	6.8%		
0.053	270				
0.037	-				
0.019	-				
0.009	-				
0.005	-				
0.002	-				
0.001	-	0.0%			
0.0001		0.0%	0.0%		
Required entry values	s are defined below. *			-	





Last Revised: 10/11/02 Filter Design 2 of 2

Filter Material

Filter criteria required by the USBR as published in Design Standards - Embankment Dams No. 13 (1994):

Dailis No. 15 (1334).			
D _{85B} used in filter design	3.948		
Average Passing #200 sieve of base soil	7.2%		
Base soil category	4		
Base soil description	Sands and gravels		
Filter criteria (mm)	$\label{eq:D15F} \begin{aligned} &\text{Maximum:} & & D_{15F} \leq & 15.79 \\ &\text{To ensure sufficient permeability:} \\ &\text{Minimum:} & & D_{15F} \geq & 1.36 \end{aligned}$		
Maximum particle size of filter (mm)	50		
Maximum % passing # 200 sieve	5%		
PI of material passing #40	0 when tested in accordace with USBR 5360, Earth Manual, on material passing #40		

Filter criteria required by the US Army Corps of Engineers as published in EM 1110-2-2300 (31 Jul 94)

2300 (31 341 34).					
D _{85B} used in filter design	3.948				
Average Passing #200 sieve of base soil	7.2%				
Base soil category	4**				
	Maximum:	$D_{15F} \leq$	15.79		
		to	19.74		
Filter criteria (mm)	To ensure sufficient permeability:				
	Minimum:	$D_{15F}\!\geq\!$	0.69		
		to	1.16		
Maximum particle size of filter (mm)	75				
Maximum % passing # 200 sieve	5%				
	0				
PI of material passing #40		when tested in accordance with			
	EM 1110-2-	1906			

^{**}If the base soil is in category 4, use the lower of the two 'max. D_{15F}' values when the filter is beneath riprap subject to wave action or beneath drains which may be subject to violent surging and/or vibration.

*Required entry values for base soil & candidate filter gradations:

1. Particle size for 100% passing. 2. % Passing the #4 sieve.

3. % Passing the #200 sieve.

4. One point in the 85% - 90% range and another point

in the 80% - 85% range, or the 85% point.

5. One point in the 15% - 20% range and another point

in the 10% - 15% range, or the 15% point.

6. No duplicate entries; if D100<#4, enter 101% for #4 and 100% for appropriate size.

USBR filter gradation limits:

Maximu	ım limit
Grain size (mm)	% Passing
50.00	100.0%
15.79	15.0%

Minimu	ım limit
Grain size (mm)	% Passing
1.36	15.0%
0.075	5.0%

USACE filter gradation limits:

Maximum limit					
Grain size (mm) % Passing					
100.0%					
15.0%					
15.0%					

ım limit
% Passing
15.0%
15.0%
5.0%

Candidate filter soil gradation. Values shown in red in the left column, and all values in the two right columns, can be changed.

Particle size	Sieve	% Passing	% Passing
mm	#	(upper bound)	(lower bound)
150.0	-	(upper bound)	(lower bound)
100.0			
	-		
90.0	-		
75.0 63.0	-		
	-		
50.0	-		
37.5	-	400.00/	
25.0 19.0	-	100.0% 90.0%	100.0%
12.5	_	00.070	100.070
9.5	_	20.0%	55.0%
4.75	4	0.0%	10.0%
3.35	6		
2.36	8	0.0%	5.0%
2.00	10	0.070	0.070
1.70	12		
1.40	14		(0.0%)
1.18	16		(
0.850	20		
0.600	30		
0.425	40		
0.300	50		
0.250	60		
0.212	70		
0.180	80		
0.150	100		
0.125 0.106	120 140		
0.100	170		
0.090	200	(0.0%)	(0.0%)
0.075	270	(0.070)	(0.070)
0.037	-		
0.019	-		
0.009 0.0001	-	0.0%	0.0%
0.0001	-	0.070	0.070
Doguired ento		-£ *	

Acceptibility of candidate filter (CF) soil:

7 toooptibility of ourididate linter (of) coll.					
USBR criteria	Upper				
OODIT CITICITA	bound	Lower bound			
Max % passing #200:	OK	OK			
Max particle size (mm):	OK	OK			
Maximum D _{15CF} :	OK	OK			
Minimum D _{15CF} :	OK	OK			
To minimize segregation (from Table 2)***					
Max allowable D _{90CF} =	40	OK			
Max D _{90CF} =	19.00	OK			

USACE criteria	Upper	
	bound	Lower bound
Max % passing #200:	OK	OK
Max particle size (mm):	OK	OK
Maximum D _{15CF} :	OK	OK
Minimum D _{15CF} (3×D _{15B}):	OK	OK
Minimum D _{15CF} (5×D _{15B}):	OK	OK
To minimize segregation (from	Table B-3)	***
Max allowable D _{90CF} =	40	OK
Max D _{90CF} =	19.00	OIX

Filters should be relatively uniform (see the C_{IJ} value of the candidate filter soil.). Also, filters should not be gapgraded.

*** Generally, this requirement is only necessary for coarse filters and gravel zones that serve as both filters and drains. For sand filters with $D_{90} < \sim 20$ mm, these limitations are usually not necessary.

Required entry values are defined above. *

Properties of candidate filter soil (CF). D sizes are in mm:								
	D _{85CF}	D _{15CF}	D _{60CF}	D _{10CF}	Cu			
upper bound	18.08	7.99	14.12	6.72	2.10			
lower bound	15.08	5.13	10.26	4.75	2.16			

Home office underdrain 10-5-21

> of D15F to D85B over that used for protecting a natural or unprocessed soil. The ratio can be as high as 9, but 5 is generally found to meet the practical requirements of the situation. This increase is sometimes possible because the first-stage filter: (1) is a material processed to stringent gradation requirements and placed and compacted under controlled conditions, (2) is inspected and tested to verify that material properties conform to those that are specified, (3) usually has seepage gradients that are much less than those of a foundation material or impervious zone that needs filter protection, and (4) has D₈₅ particles in the first stage filter material that are larger than those in materials that are usually being protected and, therefore, less likely to move. However, this increase should be made with caution.

5.5.2 Drain Pipe Perforation Size

The maximum pipe perforation dimension 19 should be no larger than the finer side of the D₅₀E where D₅₀E is taken from the gradation of the envelope (drain) material D 50minfor Astm C33 #57gravel - ~ 15mm = 0.59in that surrounds the drainpipe. That is:

Max Perforation Dimension ≤ D₅₀E

#67 govel - ~ 12mm = 0.47 m

It is emphasized that inaccessible drainpipes beneath embankment dams should be Use 1 5 10 5 m 24 avoided. Drainpipes should be sized and located, and inspection wells should be 4" more reosono ble provided so that access for inspection, maintenance, and repair, if necessary, is easy. It is recommended that each pipe segment be accessible from both ends. In order to provide a margin of safety for the pipe capacity, drains should be sized so that the depth of water in the drainpipe is less than 50 percent of the inside diameter of the drainpipe at the maximum expected discharge. If it is anticipated that the drainpipe will collect a large amount of flow from a pervious foundation or embankment, the maximum depth of water should not exceed 25 percent of the inside pipe diameter due to uncertainties in predicting the amount of flow.

5.6 Laboratory Test Procedures

In the following section, test procedures for laboratory tests are presented. The procedures have been separated into two categories: particle retention and material quality. The particle retention tests evolved from the original test procedures used during research into particle movement. The material quality tests come mainly from industry standard tests, although one stems from research work.

DS-13(5)-9 November 2011

¹⁹ The maximum dimension as used in this standard is the width for a slot and the diameter for a hole.

Home Office Underdrain Seep W Analysis Drain pipe sizing.

North Drain

Inputs

Length: 1370 ft. - from drawings

Slope 0.0047 ft./ft.
Pipe Diame 8 in
Manning's 0.0009

Slots 1.5 in2/ft specified minimum slot area

Outputs

	Seep W Input						ılts	Pipe			
Sand and Gravel (k)	Model Ground Surface	Model Groundwater Elevation	Groundwater depth below ground	Drain Elevation	Drain Depth	q	Total flow	Total flow	Flow height - solved in Flowmaster	% of diameter	Water velocity through slots
cm/sec	ft.	ft.	ft.	ft.	ft.	cfs/ft.	cfs	gpm	in		ft/sec
2.54E-03	5,024	5,019	5	5017	7	8.77E-06	1.20E-02	5.392287	0.6	7.1%	8.42E-04
2.54E-03	5,024	5,021	3	5017	7	1.85E-05	2.53E-02	11.37484	0.8	10.1%	1.78E-03
1.00E-02	5,024	5,019	5	5017	7	3.50E-05	4.80E-02	21.51996	1.1	13.7%	3.36E-03
1.00E-02	5,024	5,021	3	5017	7	7.39E-05	1.01E-01	45.43786	1.6	19.6%	7.09E-03
High flow check - :	100 gpm, greater t	han 200% of mod	el			1.63E-04	2.23E-01	100	2.3	29.3	1.56E-02

West Drain

Inputs

Length: 1326 ft. - from drawings

Slope 0.0023 ft./ft.
Pipe Diame 8 in
Manning's 0.0009

Slots 1.5 in2/ft specified minimum slot area

Outputs

		Seep V		Seep W Resu	ılts	Pipe					
Sand and Gravel (k)	Model Ground Surface	Model Groundwater Elevation	Groundwater depth below ground	Drain Elevation	Drain Depth	q	Total flow	Total flow	Flow height - solved in Flowmaster	% of diameter	Water velocity through slots
cm/sec	ft.	ft.	ft.	ft.	ft.	cfs/ft.	cfs	gpm	in		ft/sec
2.54E-03	5,024	5,019	5	5017	7	8.77E-06	1.16E-02	5.219104	0.7	8.2%	8.42E-04
2.54E-03	5,024	5,021	3	5017	7	1.85E-05	2.45E-02	11.00951	0.9	11.8%	1.78E-03
1.00E-02	5,024	5,019	5	5017	7	3.50E-05	4.64E-02	20.82881	1.3	16.0%	3.36E-03
1.00E-02	5,024	5,021	3	5017	7	7.39E-05	9.80E-02	43.97854	1.8	23.1%	7.09E-03
High flow check - :	100 gpm, greater t	han 200% of mod	el			1.68E-04	2.23E-01	100	2.8	35.3%	1.61E-02

Discharge

Length: 1400 ft. - from drawings

Slope 0.0012 ft./ft.
Pipe Diame 8 in
Manning's 0.0009

		Seep V	V Input		Seep W Results							
Sand and Gravel (k)	Model Ground Surface	Model Groundwater Elevation	Groundwater depth below ground	Drain Elevation	Drain Depth	q	Total flow	Total flow	Flow height - solved in Flowmaster	% of diameter		
cm/sec	ft.	ft.	ft.	ft.	ft.	cfs/ft.	cfs	gpm	in			
2.54E-03	5,024	5,019	5	5017	7		2.36E-02	10.61139	1.1	13.5%		
2.54E-03	5,024	5,021	3	5017	7		4.99E-02	22.38435	1.6	19.4%		
1.00E-02	5,024	5,019	5	5017	7		9.44E-02	42.34877	2.1	26.7%		
1.00E-02	5,024	5,021	3	5017	7		1.99E-01	89.4164	3.2	39.5%		
High flow check - :	200 gpm, greater t	than 200% of mod	el				4.46E-01	200	5.1	63.9%		

Notes:

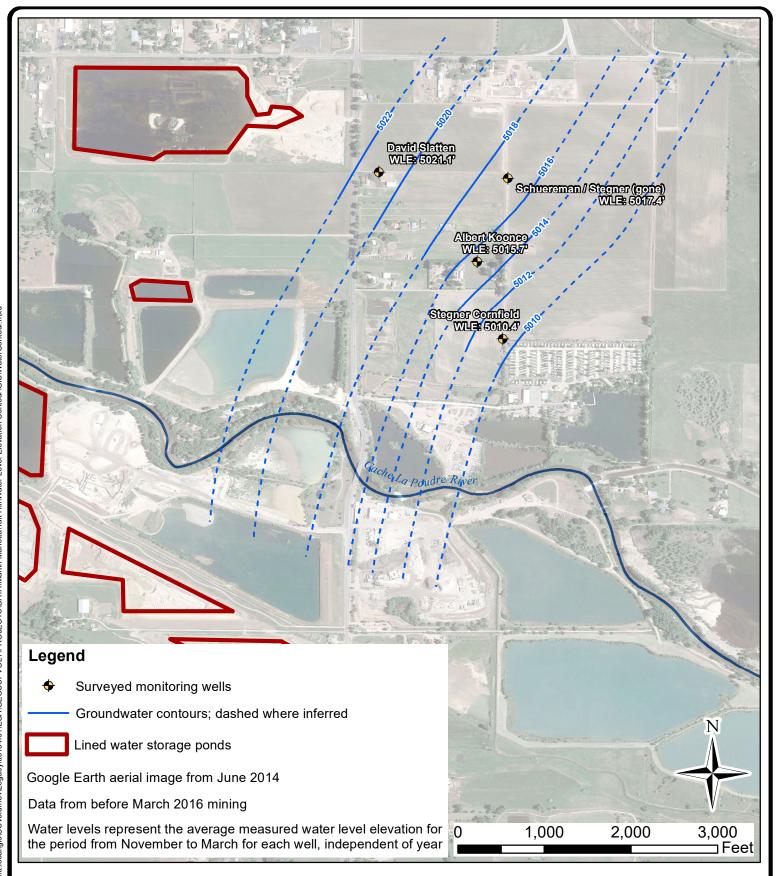
See USBR DS-15(5) - Filter Drsign. Paragraph 5.5.2 - "Drains sould be sized so that the depth of water in the drain pipe is less than 50% of the inside diameter.

Slot velocity not specified by design standard. Calculated to check, maintin below 0.06 ft/sec, which the maximum orfice velocity at 100 gpm as calculated in Flow Master

TABLE 5.5 Coarse Aggregate Grading Requirements for Concrete (Reprinted, with permission, from ASTM C33, Table 2, copyright ASTM International, 100 Barr Harbor Drive, West Conshohocken, PA 19428).

			Amounts	Finer T	han Eac	h Labora	tory Siev	e (Square	Opening	s), Weigh	t Percen	t		
Size No.	Nominal Size	4 in. (100 mm)	3 1/2 in. (90 mm)	3 in. (75 mm)	2 1/2 in. (63 mm)	2 in. (50 mm)	1 1/2 in. (37.5 mm)	1 in. (25.0 mm)	3/4 in. (19.0 mm)	1/2 in. (12.5 mm)	3/8 in. (9.5 mm)	No. 4 (4.75 mm)	No. 8 (2.36 mm)	No. 16 (1.18 mm)
1	3 1/2 to 1 1/2 in. (90 to 37.5 mm)	100	90 to 100		25 to 60		0 to 15		0 to 5					
2	2 1/2 to 1 1/2 in. (63 to 37.5 mm)			100	90 to 100	35 to 70	0 to 15		0 to 5					
3	2 to 1 in. (50 to 25.0 mm)	•••	•••	•••	100	90 to 100	35 to 70	0 to 15	•••	0 to 5	•••	•••		•••
357	2 in. to No. 4 (50 to 4.75 mm)	•••	•••	•••	100	95 to 100	•••	35 to 70	•••	10 to 30	•••	0 to 5	•••	•••
4	1 1/2 to 3/4 in. (37.5 to 19 mm)					100	90 to 100	20 to 55	0 to 15		0 to 5			
467	1 1/2 in. to No. 4 (37.5 to 4.75 mm)					100	95 to 100	•••	35 to 70	•••	10 to 30	0 to 5		
5	1 to 1/2 in. (25.0 to 12.5 mm)						100	90 to 100	20 to 55	0 to 10	0 to 5			
56	1 to 3/8 in. (25.0 to 9.5 mm)						100	90 to 100	40 to 85	10 to 40	0 to 15	0 to 5		
57	1 in. to No. 4 (25.0 to 4.75 mm)						100	95 to 100	•••	25 to 60		0 to 10	0 to 5	
6	3/4 in. to 3/8 in. (19.0 to 9.5 mm)	•••	•••	•••	•••	•••	•••	100	90 to 100	20 to 55	0 to 15	0 to 5	•••	•••
67	3/4 in. to No. 4 (19.0 to 4.75 mm)	•••						100	90 to 100		20 to 55	0 to 10	0 to 5	
7	1/2 in. to No. 4 (12.5 to 4.75 mm)								100	90 to 100	40 to 70	0 to 15	0 to 5	
8	3/8 in. to No. 8 (9.5 to 2.36 mm)	•••	•••		•••	•••	•••	•••	•••	100	85 to 100	10 to 30	0 to 10	0 to 5

ATTACHMENT 3
GROUNDWATER CONTOUR MAPS





351 Coffman St., Suite 200 Longmont, CO 80501 PH: (303) 772-5282

MARTIN MARIETTA

Taft Hill, Fort Collins, CO

PRE-MINING
GROUNDWATER CONTOURS

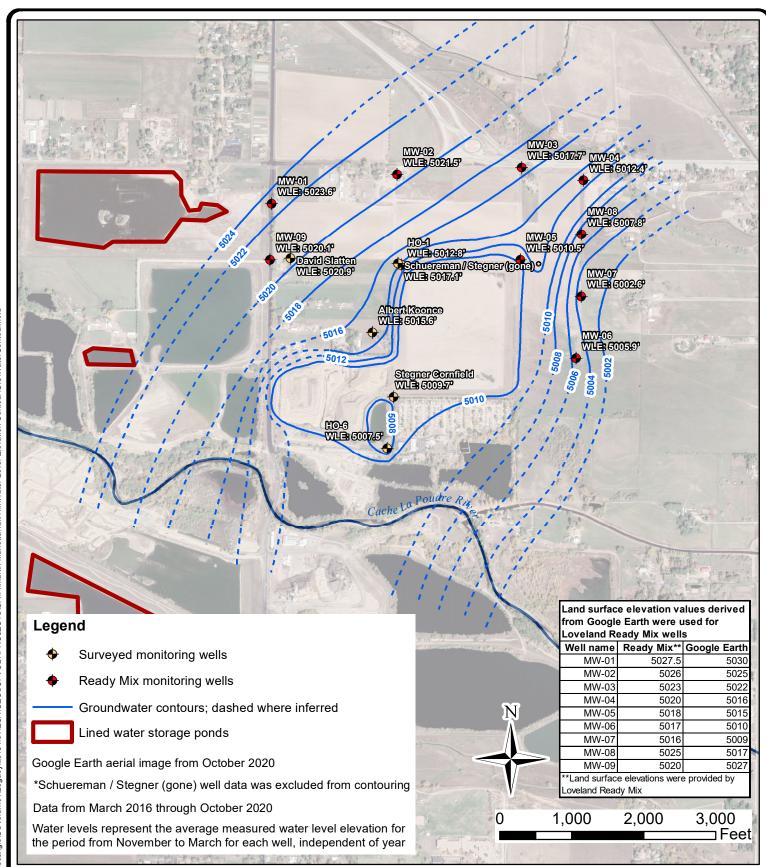
Project No.: 117-8741002

Date: January 11, 2023

January 11, 202

Figure







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MARTIN MARIETTA

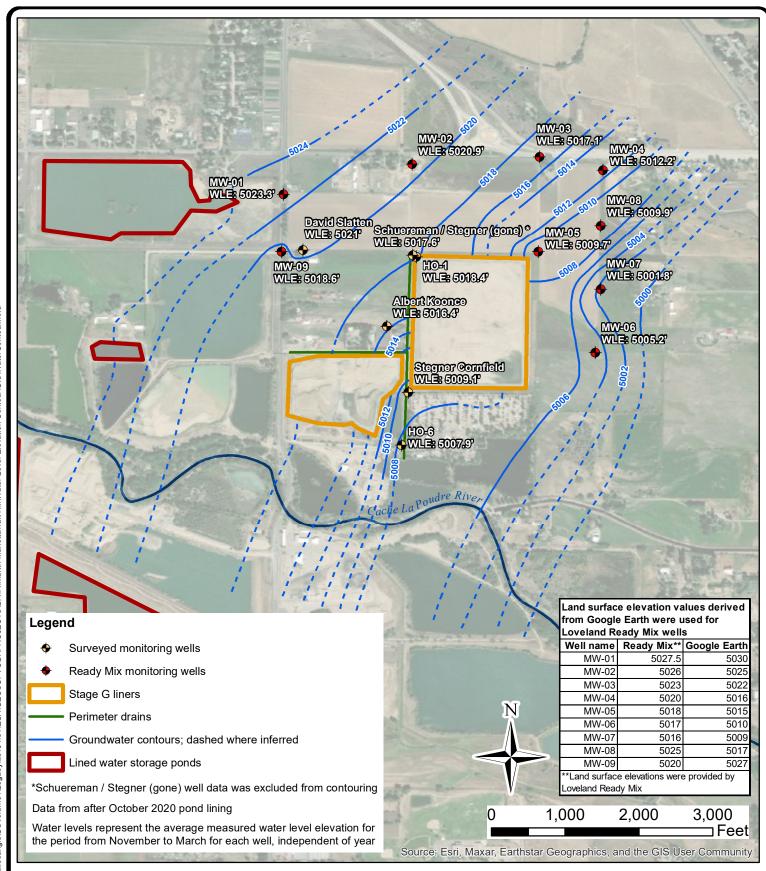
Taft Hill, Fort Collins, CO

DEWATERING GROUNDWATER CONTOURS

Date: January 11, 2023

Figure

В





351 Coffman St., Suite 200 Longmont, CO 80501 PH: (303) 772-5282

MARTIN MARIETTA

Taft Hill, Fort Collins, CO

LINED RESERVOIR GROUNDWATER CONTOURS

Project No.:	117-8741002
--------------	-------------

Date: January 11, 2023

Figure

C



February 6, 2023

Ms. Angela Myers Larimer County Clerk and Recorder 200 West Oak Street Fort Collins, CO 80521

RE: Home Office Pits, additional pages for 112 Regular Construction Materials Reclamation Permit Amendment Application, County Copy of Public Notice Documents

Dear Ms. Myers:

Attached are revised pages to the 112(c) application to the Colorado Division of Reclamation, Mining, and Safety for the operation known as the Home Office Pits. This information has been provided to the Colorado Division of Reclamation, Mining, and Safety as part of the permit application process and are to be available for public review until the amendment is approved.

If you have any questions or concerns, please contact me at (970) 407-3661.

Sincerely,

Julie Mikulas

Regional Land Manager

Julie Wikules

The public notice documents were received on the following date: February 6, 2023

By: Warne Chenery

