GENERAL NOTES

1.1 Fabrication shall be in accordance with G.S.C. standard practices in compliance with the applicable sections, relating to design requirements and allowable stresses of the latest edition of the "AWS Structural Welding Code D1.1 and D1.3". G.S.B. manufacturing procedures are certified by:

Reference	Certification numbers
Houston	G.S.C. #456

1.2 MATERIALS

	ASTM DESIGNATION	MIN. YIELD STRENGTH
Hot Rolled Steel Shapes (W, S, C & L)	A572	Fy = 50 KSI
Steel Pipes	A500	Fy = 42 KSI
Structural Tubing	A500	Fy = 46 KSI
Structural Steel Web Plate	A572/A1011	Fy = 55 KSI
Structural Steel Flange Plates/Bars	A529/A572	Fy = 55 KSI
Cold Formed Light Gage	A653/A1011	Fy = 50, 55 KSI
Roof and Wall Sheets	A792/A653	Fy = 50, 80 KSI
Cable Brace	A475 – TYPE 1	Extra High Strength
Rod Brace	A36	Fy = 36 KSI
		MIN. TENSILE STRENGTH
Machine Bolts & Nuts	A307	Fu = 60 KSI
High Strength Bolts (1"ø and less)	A325–TYPE 1	Fu = 120 KSI
High Strength Bolts (>1"ø to 1 1/2"ø)	A325–TYPE 1	Fu = 105 KSI
Anchor Bolts (if supplied)	A36/A307/F1554	Fu = 60 KSI

1.3 PRIMER
Shop primer paint is a rust inhibitive primer which meets the end performance of Federal Specification SSPC No. 15 and is G.S.C. Red Oxide color. This paint is not intended for long term exposure to the elements. G.S.C. is not responsible for any deterioration of the shop primer paint as a result of improper handling and/or jobsite storage. G.S.C. shall not be responsible for any field applied paint and/or coatings. (Section 6.5 AISC Code of Standard Practice, 14th Edition). Nominal thickness of primer will be 1 mil unless otherwise specified in contract documents.

1.4 GALVANIZED OR SPECIAL COATINGS:
See Contract Documents

1.5 ALL BOLTS ARE 1/2"ø x 0'–1" A307 EXCEPT :
a) Eave strut connection – 1/2"ø x 0'–1 1/4" A307
b) Endwall rafter splice – 5/8"ø x 0'–1 3/4" A325–N
c) Endwall column to rafter connection – 1/2"ø x 0'–1 1/4" A325–N
d) Main frame connections – SEE CROSS SECTION

NOTE: Washers are not supplied unless noted otherwise on drawing

1.6 A325 BOLT TIGHTENING REQUIREMENTS

All high strength bolts are A325–N unless specifically noted otherwise.

Structural bolts shall be tightened by the turn–of–the–nut method in accordance with the 14th Edition AISC "Specification For Structural Joints using ASTM A325 or A490 Bolts", when specifically required. A325–N bolts are supplied without washer unless otherwise noted on the drawings.

All bolted connections unless noted are designed as bearing type connections with bolt threads not excluded from the shear plane.

1.7 CLOSURE STRIPS ARE FURNISHED FOR APPLICATION:

INSIDE– Under roof panels at eave
OUTSIDE – Between endwall panels and rake trim
– Under continuous ridge vent skirts

1.8 ERECTION NOTE:
All bracing, strapping, & bridging shown and provided by G.S.C. for this building is required and shall be installed by the erector as a permanent part of the structure. If additional bracing is required for stability during erection, it shall be the erector's responsibility to determine the amount of such bracing and to procure and install as needed.

1.9 ERECTION AND UNLOADING NOT BY G.S.C.

1.10 SHORTAGES
Any claims or shortages by buyer must be made to G.S.C. within five (5) working days after delivery, or such claims will be considered to have been waived by the customer and disallowed.

1.11 CORRECTIONS OF ERRORS AND REPAIRS (MBMA 6.10)
Claims for correction of alleged misfits will be disallowed unless G.S.C. shall have received prior notice thereof and allowed reasonable inspection of such misfits. The correction of minor misfits by the use of drift pins to draw the components into line, moderate amounts of reaming, chipping and cutting, and the replacement of minor shortages of material are a normal part of erection and are not subject to claim. No part of the Building may be returned for alleged misfits without the prior approval of G.S.C.

BUYER/END USE CUSTOMER RESPONSIBILITIES

2.1 It is the responsibility of the BUYER/END USE CUSTOMER to obtain appropriate approvals and secure necessary permits from City, County, State, or Federal Agencies as required, and to advise/release G.S.C. to fabricate upon receiving such.

2.2 General Steel Corporation (hereafter referred to as G.S.C.) standard specifications apply unless stipulated otherwise in the Contract Documents. G.S.C. design, fabrication, quality criteria, standards, practice, methods and tolerances shall govern the work with any other interpretations to the contrary notwithstanding. It is understood by both Parties that the BUYER/END USE CUSTOMER is responsible for clarification of inclusions or exclusions from the architectural plans and/or specifications.

2.3 In case of discrepancies between G.S.C. structural steel plans and plans for other trades, G.S.C. plans shall govern. (Section. 3 AISC Code of Standard Practices, 14th Edition)

2.4 Approval of G.S.C. drawings and calculations indicates that G.S.C. has correctly interpreted and applied the Contract Documents. This approval constitutes the contractor/owners acceptance of the G.S.C. design concepts, assumptions, and loading. (Section 4 AISC Code 14th Edition and MBMA 3.3.3)

2.5 Once the BUYER/END USE CUSTOMER has signed G.S.C. Approval Package and the project is released for fabrication, changes shall be billed to the BUYER/END USE CUSTOMER including material, engineering and other costs. An additional fee may be charged if the project must be moved from the fabrication and shipping schedule.

GENERAL STEEL CORPORATION

FOR PERMIT

DRAWING PACKAGE

SALES NO.	68726	JOB NO.	148759	BUILDING	A
CUSTOMER	AMMC Industries				
END USER	AMMC Industries				
END USE	Shop				
STREET	19911 Hwy. 550				
CITY ST ZIP	Montrose, CO 81403				
COUNTY	Montrose				

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH THE FOLLOWING AS INDICATED:

DESIGN LOADS:

Design Code	: IBC 18
Dead Load (psf)	: Metal building structure only by RGB
Collateral Load (psf)	: 0.00
Wind Load	
Basic Design Wind Speed	: V (3 sec. gust) = 115 mph
Allowable Stress Design Wind Speed	: Vasd (3 sec. gust) = 89.080 mph
Risk Category	: II – Normal
Wind Exposure	: C
Internal Pressure Coefficient, GCPi	: 0.180 /–0.180
Design Wind Pressure For Wall	: Based on Allowable Stress Design Wind Speed
Components Wind Pressure (psf) asd	: 10.92
Components Wind Suction (psf) asd	: –12.00
Claddings Wind Pressure (psf) asd	: 12.78
Claddings Wind Suction (psf) asd	: –13.86
Enclosure	: Closed
Live Load	
Primary Framing (psf)	: 20.00
Trib. Area Reduction	: No
Secondary Framing (psf)	: 20.00
Snow Load	
Ground Snow Load, Pg (psf)	: 173.00
Roof Snow Load, Pf (psf)	: 121.10
Sloped Roof Snow Load, Ps (psf)	: 121.10
Snow Exposure Factor, Ce	: 1.000
Snow Importance Factor, Is	: 1.000
Thermal Factor, Ct	: 1.000
Sloped Factor, Cs	: 1.000
Seismic Load	
Seismic Importance Factor, Ie	: 1.000
Seismic Occupancy Category	: II – Normal
Site Class	: D
Mapped Spectral Response Acceleration	: Ss = 0.330 :S1 = 0.075
Spectral Response Coefficients	: Sds = 0.337 :Sd1 = 0.120
Seismic Design Category	: C
Basic Force Resisting Systems Used	: Steel Systems Not Specifically Detailed For Seismic Resistance : Rigid Frames : Braced Frames
Total Design Base Shear, V (kips)	: Longitudinal= 10.63 Transverse=10.70
Response Modification Factors, R	: Rigid Frames = 3.00 : SW X–Bracing = 3.00 : EW X–Bracing = 3.00
Seismic Response Coefficient, Cs	: Rigid Frames = 0.112 : SW X–Bracing = 0.112 : EW X–Bracing = 0.112
Analysis Procedure Used	: Equivalent Lateral Force Procedure
Rainfall Intensity (in/hr)	
Other Loads/Requirements	: NONE

BUILDING DESCRIPTION:

Width (ft)	: 50
Length (ft)	: 60
Eave Ht. at BSW (ft)	: 17
Eave Ht. at FSW (ft)	: 17
Roof Slope at BSW	: 2.0:12
Roof Slope at FSW	: 2.0:12
Bay Spacing (ft)	: 3 at 20

COVERING AND TRIMS:

Roof Panels & Trims	
Panel Type	: 26 Ga. PBR
Panel Color	: Glvm.Plus
Trim Colors	
Eave Trim	: S2000 Standard
Eave Gutter	:
Gable Trim	: S2000 Standard
Wall Panel & Trims	
Panel Type	: 26 Ga. PBR
Panel Color	: S2000 Standard
Trim Colors	
Corner Trims	: S2000 Standard
Opening Trims	: S2000 Standard
Downspouts	:
Base Trim	: S2000 Standard
Mas. Flash	: S2000 Standard
Special Requirements	: NONE

NOTE:
For Snow/Ice Removal Procedure, Refer to Metal Building System Manual 2012 Edition, Section A9.4, Page A–59

Before erecting your building, please see the Rigid Erection & Safety Manual at rigidbuilding.com/document–library

SEALING OF THIS DRAWING DOES NOT IMPLY OR CONSTITUTE THAT GENERAL STEEL ENGINEER IS THE ENGINEER OF RECORD OR THE DESIGN PROFESSIONAL FOR THIS PROJECT. ONLY THE DESIGN OF THE METAL BUILDING SYSTEM AS FURNISHED BY G.S.C. IS INCLUDED. FOUNDATION ANALYSIS, ELECTRICAL AND MECHANICAL SYSTEMS, AND/OR OTHER PARTS SUPPLIED BY ANYONE OTHER THAN G.S.C. ARE SPECIFICALLY EXCLUDED. NO INSPECTION OR SUPERVISION IS IMPLIED.

2.6 The BUYER/END USE CUSTOMER is responsible for overall project coordination. All interface, compatibility, and design considerations concerning any materials not furnished by G.S.C. and G.S.C. steel system are to be considered and coordinated by the BUYER/END USE CUSTOMER. Specific design criteria concerning this interface between materials must be furnished before release for fabrication or G.S.C. assumptions will govern (Section 4 and Commentary, AISC Code of Standard Practice, 14th Edition)

2.7 It is the responsibility of the BUYER/END USE CUSTOMER to insure that G.S.C. plans comply with the applicable requirements of any governing building authorities. The supplying of sealed engineering data and drawings for the metal building system does not imply or constitute an agreement that G.S.C. or its design engineers are acting as the engineer of record or design professional for a construction project. These drawings are sealed only to certify the design of the structural components furnished by G.S.C.

2.8 The BUYER/END USE CUSTOMER is responsible for setting of anchor bolts and erection of steel in accordance with G.S.C. "For Construction" drawings only. Temporary supports such as guys, braces, falsework, cribbing or other elements required for the erection operation shall be determined furnished and installed by the erector. No items should be purchased from a preliminary set of drawings, including anchor bolts. Use only final "FOR CONSTRUCTION DRAWINGS" for this use. (Section 7 AISC Code of Standard Practice, 14th Edition.)

2.9 General Steel Corp. is responsible for the design of the anchor bolt to permit the transfer of forces between the base plate and the anchor bolt in shear, bearing and tension, but is not responsible for the transfer of anchor bolt forces to the concrete or the adequacy of the anchor bolt in relation to the concrete.
Unless otherwise provided in the Order Documents, G.S.C. does not design and is not responsible for the design, material and construction of the foundation or foundation embedments. The END USE CUSTOMER should assure himself that adequate provisions are made in the foundation design for loads imposed by column reactions of the building, other imposed loads, and bearing capacity of the soil and other conditions of the building site.
It is recommended that the anchorage and foundation of the building be designed by a Registered Professional Engineer experienced in the design of such structures. (Chapter IV Section 3.2.2 Metal Building Systems Manual 2012 Edition)

2.10 Normal erection operations include the corrections of minor misfits by moderate amounts of reaming, chipping, welding or cutting, and the drawing of elements into line through the use of drift pins. Errors which cannot be corrected by the foregoing means or which require major changes in member configuration are to be reported immediately to G.S.C. by the BUYER/END USE CUSTOMER, to enable whoever is responsible either to correct the error or to approve the most efficient and economic method of correction to be used by others. (Section 7 AISC Code of Standard Practice, 14th Edition)

2.11 Neither the fabricator nor the BUYER/END USE CUSTOMER will cut, drill or otherwise alter his work, or the work of other trades, to accommodate other trades, unless such work is clearly specified in the contract documents. Whenever such work is specified, the BUYER/END USE CUSTOMER is responsible for furnishing complete information as to materials, size, location and number of alterations prior to preparation of shop drawings. (Section 7 AISC Code of Standard Practice, 14th Edition)

2.12 WARNING In no case should Galvalume steel panels be used in conjunction with lead or copper. Both lead and copper have harmful corrosive effects on the Galvalume alloy coating when they are in contact with Galvalume steel panels. Even run–off from copper flashing, wiring, or tubing onto Galvalume should be avoided.

2.13 SAFETY COMMITMENT General Steel Corp. has a commitment to manufacture quality building components that can be safely erected. However, the safety commitment and job site practices of the erector are beyond the control of G.S.C.
It is strongly recommended that safe working conditions and accident prevention practices be the top priority of any job site. Local, State, and Federal safety and health standards should always be followed to help insure workers safety. Make certain all employees know the safest and most productive way of erecting a building. Emergency procedures should be known to all employees.
Daily meetings highlighting safety procedures are also recommended. The use of hard hats, rubber sole shoes for roof work, proper equipment for handling material, and safety nets where applicable, are recommended.

2.14 Roof drainage systems (gutter, downspouts, etc.) must be free of any obstruction to ensure smooth operation at any given time.

2.15 It is recommended by Factory Mutual (Reference: 82.44) that roofs be cleared of snow when half of the maximum snow depth is reached. The maximum snow depth can be estimated based on the design snow load and the density of snow and/or ice buildup. See Chart below.

ROOF SNOW LOAD (IN PSF)	EQUIVALENT SNOW HEIGHT AT ROOF (IN INCHES)	RECOMMENDED SNOW HEIGHT WHEN SNOW REMOVAL SHOULD START (IN INCHES)
20	16.60	8.30
25	17.25	8.62
30	17.90	8.95
35	18.55	9.28
40	19.20	9.60
45	19.85	9.92
50	20.50	10.25
55	21.15	10.58
60	21.80	10.90
65	22.45	11.22
70	23.10	11.55
75	23.75	11.88
80	24.40	12.20

COLORADO LICENSED
CARLITO M. DAYAG
46701
PROFESSIONAL ENGINEER
11/5/20

CUSTOMER					
AMMC Industries					
SALES NO.	JOB NO.	BUILDING	ENG. NO.	REV.
68726	148759	A	C001	A

UNLOADING, HANDLING AND STORING OF MATERIALS

STRUCTURAL

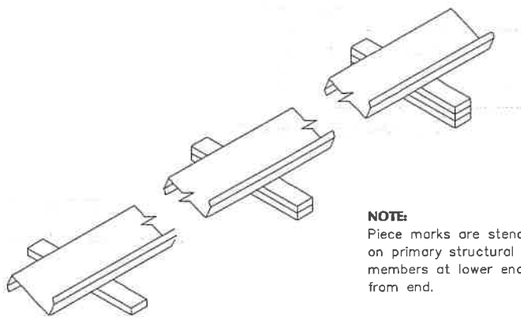
A great amount of time and trouble can be saved if the building site is according to a pre–arranged plan. Proper location and handling of components will eliminate unnecessary handling.

Inspect all shipments prior to releasing the tie–downs for loads that may have shifted during transit, **REMEMBER, SAFETY FIRST!**

Blocking under the columns and rafters protects the splice plates and the slab from damage during the unloading process. It also facilitates the placing of slings or cables around the members for later lifting and allows members to be bolted together into sub–assemblies while on the ground. Extra care should always be exercised in the unloading operations to prevent injuries from handling the steel and to prevent damage to materials and the concrete slabs.

If water is allowed to remain for extended periods in bundles of primed parts such as girts, purlins etc., the pigment will fade and the paint will gradually soften, reducing the bond to the steel. Therefore, upon receipt of a job, all bundles of primed parts should be stored at an angle to allow any trapped water to drain away and permit air circulation for drying. Puddles of water should not be allowed to collect and remain on columns, rafters or beams for the same reason.

All Primer should be touched up as required before erection!



NOTE:
Piece marks are stenciled on primary structural members at lower end, 1'–0" from end.

WALLS AND ROOF PANELS

G.S.C.'s wall and roof panels including color coated, galvalume and galvanized, provide excellent service under widely varied conditions. All unloading and erection personnel should fully understand that these panels are quality merchandise which merit cautious care in handling.

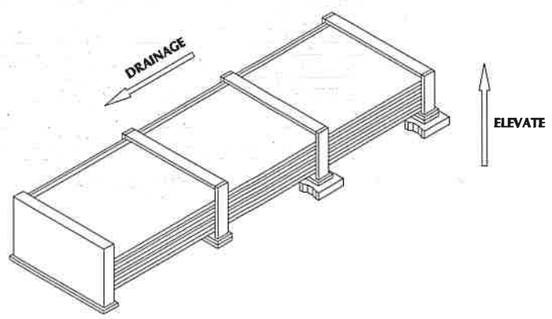
Under no circumstances should panels be handled roughly. Packages of sheets should be lifted off the truck with extreme care taken to insure that no damage occurs to ends of the sheets or to side ribs. The packages should be stored off the ground sufficiently high to allow air circulation underneath the packages. This avoids ground moisture and deters people from walking on the packages. One end of the package should always be elevated to encourage drainage in case of rain.

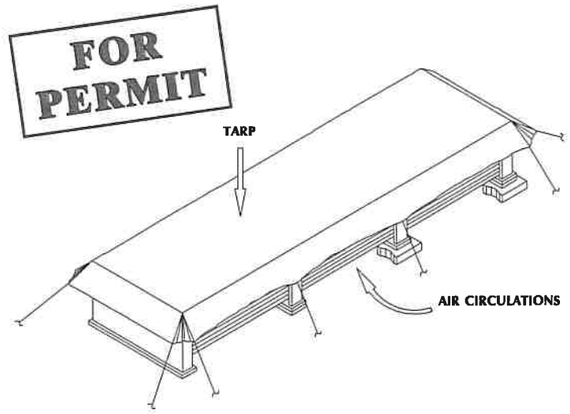
All stacked metal panels are subject, to some degree, to localized discoloration or stain when water is trapped between their closely nested surfaces. GSC exercises extreme caution during fabricating and shipping operations to insure that all panel stock is kept dry. However, due to climatic conditions, water formed by condensation of humid air can be trapped between stacked sheets. Water can also be trapped between stacked sheets when exposed to rain. This discoloration caused by trapped moisture is often called wet storage stain.

The stain is usually superficial and has little effect on the appearance or service life of the panels as long as it is not permitted to remain on the panels. However, moisture in contact with the surface of the panels over an extended period can severely attack the finish and reduce the effective service life. Therefore, it is imperative that all panels be inspected for moisture upon receipt of the order. If moisture is present, dry the panels at once and store in a dry, warm place.

CAUTION: Care should always be taken when walking on panels. Use softey lines and nets when necessary! Panels are slippery. Oil or wax applied to the roof and wall panels for protection against weather damage will make them a very slippery surface. Wipe dry any oil that has puddled from bundles stored on a slope. Dew, frost, or other forms of moisture greatly increase the slipperiness of the panels. Always assume panel surface is slippery and act accordingly. **Think safety!!**

Use wood blocking to elevate and slope the panels in a manner that will allow moisture to drain. Wood blocking placed between bundles will provide additional air circulation. Cover the stacked bundles with a tarp or plastic cover leaving enough opening at the bottom for air to circulate.





When handling or uncrating the panels, lift, rather than slide, them apart. Burred edges may scratch the coated surfaces when sheets are slid over one another. Never allow panels to be walked on while on the ground.

Rough and improper handling of a panel is inexcusable and a prime example of poor job supervision.

NOTE:
Use gloves when handling metal panels to prevent hand injuries. Be aware, of the dangers of handling panels on a windy day. A large panel can catch enough wind to knock a worker off his feet, even at ground level!! **Safety first!**

GENERAL NOTE:
1. OIL CANNING OF PANELS IS NOT A CAUSE OF REJECTION.
2. EXTREME CARE MUST BE EXERCISED DURING THE ERECTION OF ROOF PANELS AND TRIMS. FOOT TRAFFIC MAY RESULT IN PERMANENT PANEL DISTORTION AND FINISH ABRASION.

11/5/20

COLORADO LICENSED
CARLITO M. DAYAG
46701
PROFESSIONAL ENGINEER

SEALING OF THIS DRAWING DOES NOT IMPLY OR CONSTITUTE THAT GENERAL STEEL CORPORATION IS THE ENGINEER OF RECORD OR THE DESIGN PROFESSIONAL FOR THIS PROJECT. ONLY THE DESIGN OF THE METAL BUILDING SYSTEM AS FURNISHED BY G.S.C. IS INCLUDED. FOUNDATION ANALYSIS, ELECTRICAL, AND MECHANICAL SYSTEMS, AND/OR OTHER PARTS SUPPLIED BY ANYONE OTHER THAN G.S.C. ARE SPECIFICALLY EXCLUDED. NO INSPECTION OR SUPERVISION IS IMPLIED.

AMMC Industries				
SALES JOB# 68726	JOB# 148759	DRAWING A	DWG. NO. C002	REV. A

LEGENDS & ABBREVIATIONS

DESIGN:

Accel.	Acceleration
Coeff.	Coefficient
CL, Collat	Collateral Load
DL, Dead	Dead Load
H, Horz, Horiz	Horizontal
L	Left
LL, Live	Live Load
LnWnd, LnWind, LWIND	Longitudinal Wind Load
Min, min	Minimum
Max, max	Maximum
R	Right
SL	Snow Load
Slide	Sliding Snow Load
SEIS, Seis	Seismic Load
U_Snow	Unbalance Snow Load
V, Vert	Vertical
WL, Wind_L	Wind Load Left
WR, Wind_R	Wind Load Right
WP, Wind_P	Wind Pressure
WS, Wind_S	Wind Suction

ENGLISH UNITS

Acre	Acres
FT, ft	Feet
GA, Ga, ga	Gage
Gal	Gallons
IN, In	Inches
K, k	Kips
KSI, ksi	Kips Per Square-Inches
lb, #	Pounds
MPH, mph	Miles Per Hour
PLF, plf, lb/ft	Pounds Per Linear-Foot
PSF, psf, lb/ft^2	Pounds Per Square-Feet
TON, ton	Tons
Yd	Yard

METRIC UNITS

cm	Centimeters
Hec	Hectares
liter	Liters
m	Meters
mm	Millimeters
N	Newtons
km	Kilometers
kN	Kilonewtons
kN/m^2	Kilonewtons Per Square-meter
kPa	Kilopascals
kph	Kilometers Per Hour
Pa	Pascals

USEFUL CONVERSION

English	To	English	Metric	To	Metric
1 mile		1760 Yd	1 km		1000 m
1 Yd		3 Ft	1 m		100 cm
1 Ft		12 In	1 cm		10 mm
1 in		16/16 In	1 kN		1000 N
1 Ton (English)		2 Kips	1 kg		9.8066 N
1 Kip		1000 lb	1 Ton (Metric)		1000 kg
1 lb		16 ounces	1 Hec		10,000 m^2
1 Acre		43560 Ft^2	1 m^3		1000 liter
1 Ft^3		7.4805 Gal	1 kPa		1 kN/m^2

English	To	Metric	Metric	To	English
1 in		2.54 cm	1 cm		0.3937 in
1 ft		0.3048 m	1 m		3.2808 ft
1 lb		0.4536 kg	1 kg		2.2046 lb
1 Ton (English)		907.18 kg	1 Ton (Metric)		2204.6 lb
1 Kip		4.4482 kN	1 kN		0.2248 kip
1 mile		1.6093 km	1 km		0.6213 mile
1 Acre		0.4046 Hec	1 Hec		2.4715 Acres
1 lb/ft^2		0.0478 kPa	1 kPa		20.8854 lb/ft^2

Fraction	To	Decimal	Fraction	To	Decimal
1/16		0.0625	9/16		0.5625
1/8		0.1250	5/8		0.6250
3/16		0.1875	11/16		0.6875
1/4		0.2500	3/4		0.7500
5/16		0.3125	13/16		0.8125
3/8		0.3750	7/8		0.8750
7/16		0.4375	15/16		0.9375
1/2		0.5000	16/16		1.0000

DRAWINGS:

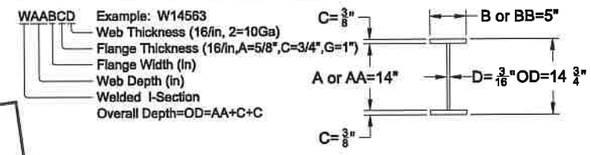
AB, A.B.	Anchor Bolt
AS, As Shown	As Shown
Aux.	Auxiliary
BLDG., Bldg.	Building
B.P., Base PL	Base Plate
BOTT., Bott.	Bottom
Bott. Base PL, B.O.B.P	Bottom Of Base Plate
B.O.S.	Bottom Of Steel
BSW	Back Sidewall
BY OTHERS	By Other Supplier or Not By Rigid
C/C	Center to Center
C.I.P.	Cast-In-Place
CL, ℄	Center Line
CLR.	Clear, Clearance
CMU	Concrete Masonry Unit
COL., Col.	Column
CONC., Conc.	Concrete
CONT.	Continuous, Continuation
DET.	Detail
DIA., Dia., Ø	Diameter
DIM., Dim.	Dimension
DWG., Dwg.	Drawing
EH, E.H.	Eave Height
EJ, Exp. Jt.	Expansion Joint
EL, Elev.	Elevation
EP	End Plate
ES, E.S.	Eave Strut
EW	Endwall
EW COL, EC	Endwall Column
EW RAF	Endwall Rafter
Exp. Bolt	Expansion Bolt
FFL, Fin. Flr.	Finish Floor Line
FLG., FLGE., Flg., Flge.	Flange
FNB, F.N.B.	Fin Neck Bolt
FO, F.O.	Framed Opening
FRM., Frm.	Frame
FSW	Front Sidewall
GA, Ga.	Gage
GALV., Galv.	Galvanized
G.O.L.	Gage of Outstanding Leg
H, Ht.	Height
HED,HEDS	High Eave Double Slope
HES,HESS	High Eave Single Slope
Horz, Horiz	Horizontal
HSB, H.S.B.	High Strength Bolt
HSS	Hollow Structural Section
INT., Int.	Interior, Intermediate
I/S	Inside
LED,LEDS	Low Eave Double Slope
LES,LESS	Low Eave Single Slope
LEW	Left Endwall
LHI	Left Hand In
LHO	Left Hand Out
LL	Long Life
LLH	Long Leg Horizontal
LLV	Long Leg Vertical
LT	Lean-To
LT COL	Lean-To Column
LT RAF	Lean-To Rafter
LG., Lg.	Long
L, Lt.	Length
L x W x H	Length x Width x Height
MAX., max.	Maximum
MIN., min.	Minimum
MKD., MK'D.	Marked
MB, M.B.	Machine Bolt
MEZZ., Mezz.	Mezzanine
N.A., N/A	Not Applicable
NO., No.	Number
NS/FS, NS&FS	Near Side and Far Side
O.C.	On Center
O/S	Outside
OH, Opp Hand	Opposite Hand (Mirror Image)
OHD, O.H.D.	Over-Head Door
O/O	Out to Out
PF COL	Portal Frame Column (Wind Bent Column)
PF RAF	Portal Frame Rafter (Wind Bent Rafter)
PL, ℔	Plate
QTY., Qty.	Quantity
REF., Ref.	Refer, Reference
REW	Right Endwall

DRAWINGS:

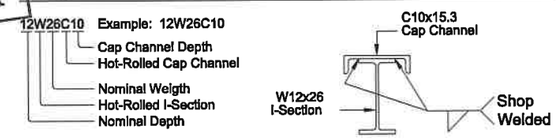
RHB	Round Head Bolt
RHI	Right Hand In
RHO	Right Hand Out
REINF.	Reinforced
REQ'D., REQD.,	Required
REV., Rev.	Revised, Revision
RF, R.F.	Rigid Frame
RF COL	Rigid Frame Column
RF RAF	Rigid Frame Rafter
RUD, R.U.D.	Roll-Up Door
SC	Slip Critical
SDS	Self-Drilling Screws
SECT., Sect.	Section
SHTG., Shtg.	Sheeting
Sol Col	Soldier Column
SP	Splice Plate
SSR	Standing Seam Roof
SST	Stainless Steel
ST COL	Straight Column
STIFF.	Stiffener
STD.	Standard
STS	Self-Tapping Screws
SW	Sidewall
SYM., Sym., SYMM., Symm.	Symmetry, Symmetrical
TBE	To Be Established
TBD	To Be Determined
TC	Tension Control
THK., Thk.	Thick
TOC, T.O.C.	Top Of Concrete
TOS, T.O.S.	Top Of Steel
T & B, TOP & BOTT	Top and Bottom
TYP., Typ., typ.	Typical
UN, U.N.O.	Unless Noted, Unless Noted Otherwise
Vert.	Vertical
WD	Walk Door
W, Wd.	Width
W.P.	Work Point, Working Point

FOR PERMIT

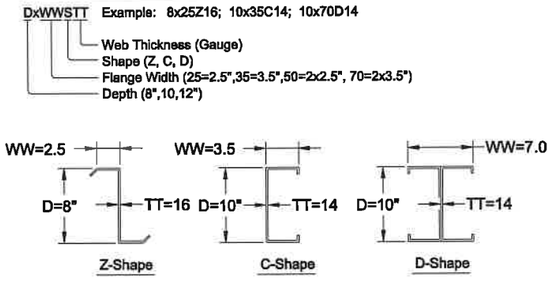
BUILT-UP SECTIONS: USED FOR FRAMES, BEAMS, COLUMNS



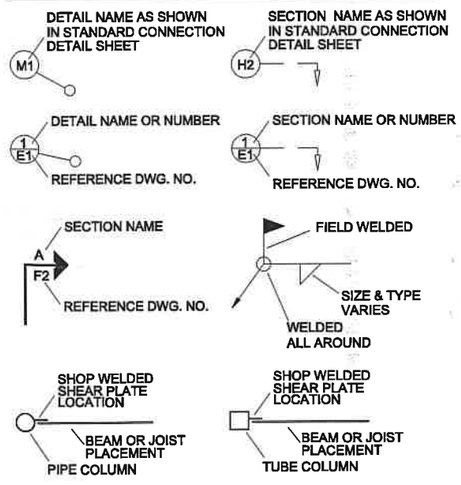
CRANE BEAM SECTIONS: USED FOR RUNWAY BEAMS, MONORAILS



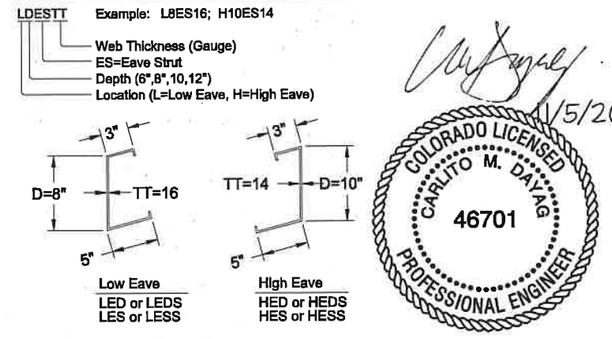
COLD-FORMED SECTIONS Z,C,D: USED FOR PURLINS,GIRTS,JAMBS,JOISTS



SECTIONS AND DETAILS:



COLD-FORMED SECTION ES: USED FOR EAVE STRUTS



11/5/20

COLORADO LICENSED
CARLITO M. DAYAG
46701
PROFESSIONAL ENGINEER

SEALING OF THIS DRAWING DOES NOT IMPLY OR CONSTITUTE THAT GENERAL STEEL ENGINEER IS THE ENGINEER OF RECORD OR THE DESIGN PROFESSIONAL FOR THIS PROJECT. ONLY THE DESIGN OF THE METAL BUILDING SYSTEM AS FURNISHED BY G.S.C. IS INCLUDED. FOUNDATION ANALYSIS, ELECTRICAL, AND MECHANICAL SYSTEMS, AND/OR OTHER PARTS SUPPLIED BY ANYONE OTHER THAN G.S.C. ARE SPECIFICALLY EXCLUDED. NO INSPECTION OR SUPERVISION IS IMPLIED.

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
A	PERMIT	11/05/20	JAP	AGP	BKD

GENERAL STEEL CORPORATION

DESCRIPTION	LEGEND & ABBREVIATIONS			
CUSTOMER	AMMC Industries			
END USER	AMMC Industries			
END USE	Shop	BUILDING	A	
STREET	19911 Hwy. 550			
CITY ST ZIP	Montrose, CO 81403			
SALES NO. 68726	DWG. NO. 148759	SCALE N.T.S.	ENG. NO. C003	ISSUE A

DRAWING INDEX

DWG.NO.	ISSUE	DRAWING TITLE	DWG.NO.	ISSUE	DRAWING TITLE	DWG.NO.	ISSUE	DRAWING TITLE
C001	A	COVER SHEET						
C002	A	UNLOADING, HANDLING & STORING OF MATERIALS						
C003	A	LEGENDS & ABBREVIATIONS						
C004	A	DRAWING INDEX						
F001	0	COLUMN LAYOUT PLAN						
F002	0	ANCHOR BOLT DETAILS						
F003	0	ANCHOR BOLT REACTIONS						
E001	A	ROOF FRAMING PLAN						
E002	A	ROOF SHEETING PLAN						
E003	A	RIGID FRAME ELEVATION						
E004	A	ENDWALL FRAMING & SHEETING ELEVATION						
E005	A	ENDWALL FRAMING & SHEETING ELEVATION						
E006	A	SIDEWALL FRAMING & SHEETING ELEVATION						
E007	A	SIDEWALL FRAMING & SHEETING ELEVATION						
E008	A	DETAIL DRAWINGS						
E009	A	DETAIL DRAWINGS						
E010	A	PANEL PROFILE, TRIMS AND ACCESSORIES						

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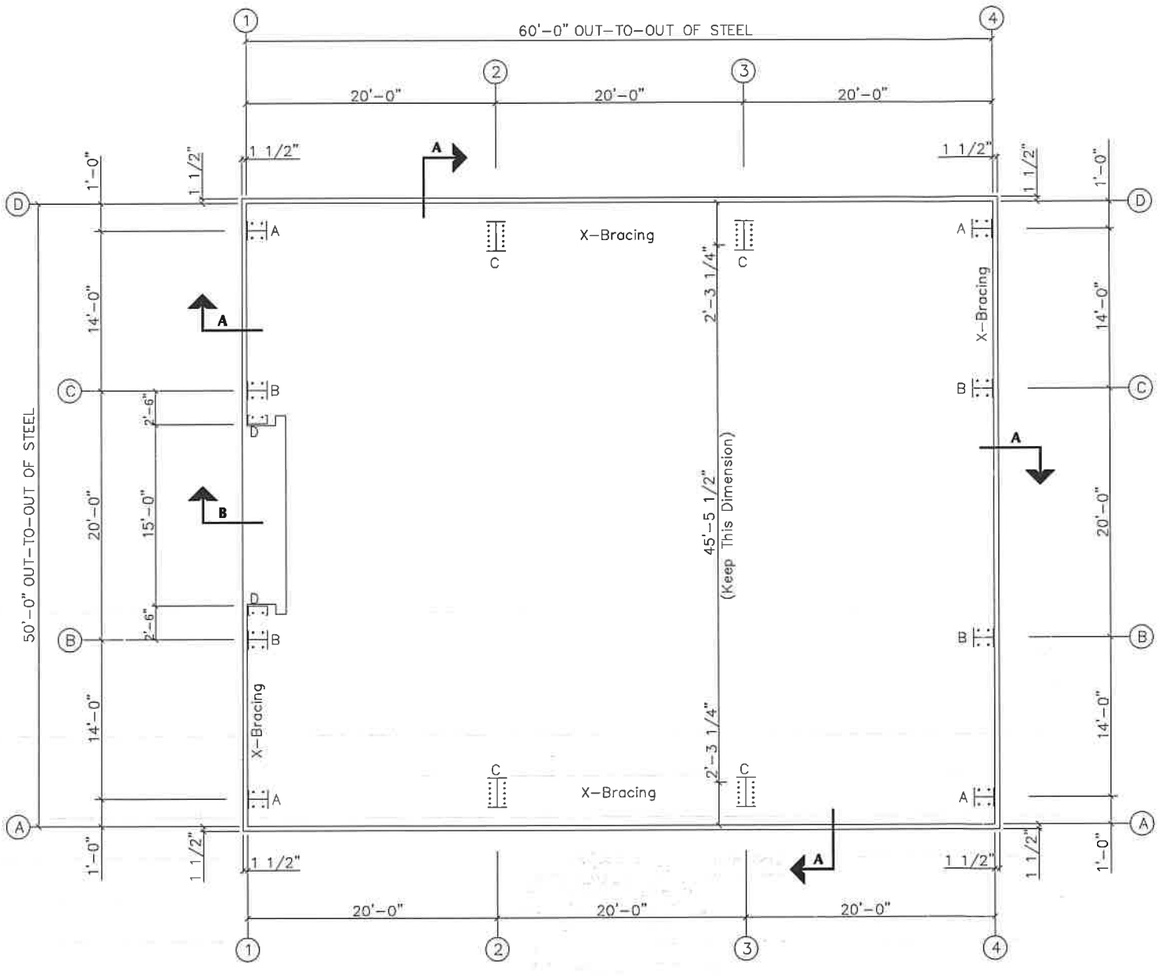
ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
A	PERMIT	11/05/20	JAP	AGP	BKD

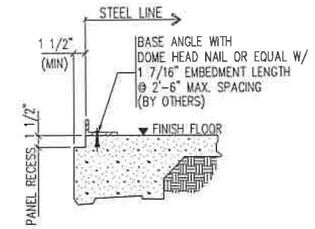
GENERAL STEEL CORPORATION

DESCRIPTION	DRAWING INDEX		
CUSTOMER	AMMC Industries		
END USER	AMMC Industries		
END USE	Shop	BUILDING	A
STREET	19911 Hwy. 550		
CITY ST ZIP	Montrose, CO 81403		
SALES NO. 58726	JOB NO. 148759	SCALE N.T.S.	DRWG NO. C004 REV. A



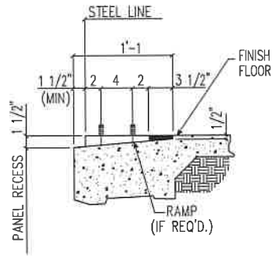
COLUMN LAYOUT PLAN

NOTE: All Base Plates @ 100'-0" (U.N.)

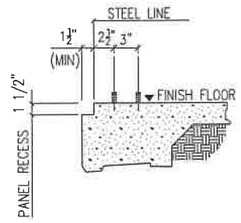


IMPORTANT NOTES:
NOTCH AREA INDICATES 1 1/2" INCH BY 1 1/2" INCH RECESS FOR METAL WALL SHEETS. THE SHEETS SHOULD NOT TOUCH THE BOTTOM OF THE NOTCH, WHICH WOULD VOID THE WARRANTY!

A SECTION



B SECTION



FIELD LOCATE WALKDOOR

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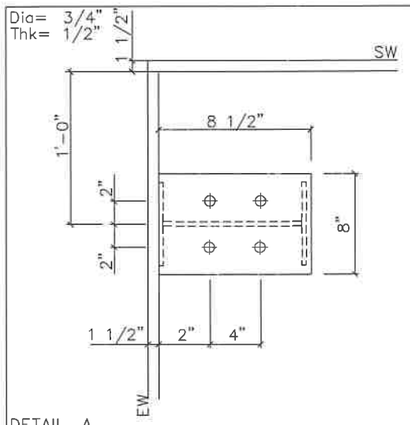
ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
0	CONSTRUCTION/PERMIT	11/05/20	JAP	AGP	BKD

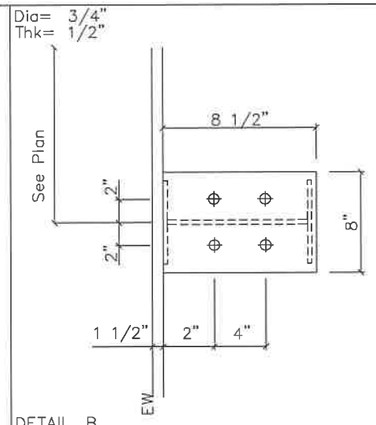
GENERAL STEEL CORPORATION

DESCRIPTION	COLUMN LAYOUT PLAN		
CUSTOMER	AMMC Industries		
END USER	AMMC Industries		
END USE	Shop	BUILDING	A
STREET	19911 Hwy. 550		
CITY ST ZIP	Montrose, CO 81403		
SALES NO. 68726	JOB NO. 148759	SCALE N.T.S.	DWG. NO. F001

REV. 0



DETAIL A



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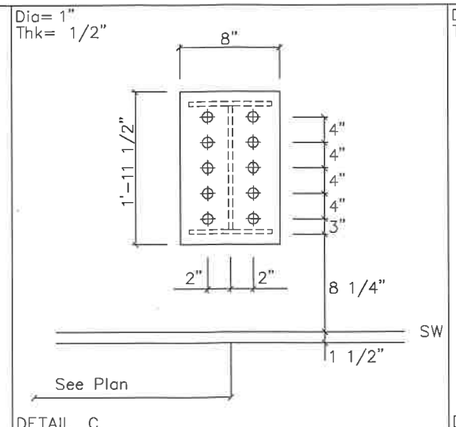
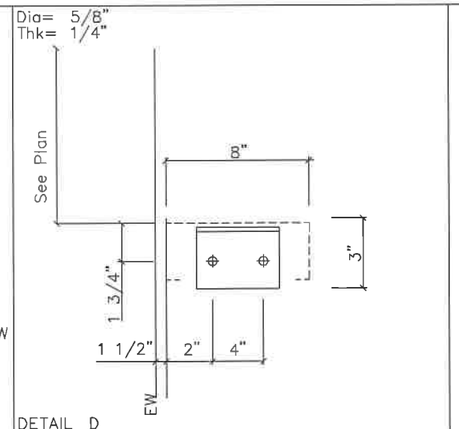


DETAIL C



DETAIL D

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PROFESSIONAL ENGINEER

GENERAL NOTES:

1. THE ANCHOR BOLT DETAILS SHOWN ON THIS DRAWING LOCATE THE ANCHOR BOLTS IN REFERENCE TO BOTH THE BUILDING STEEL LINE AND THE OUTSIDE OF RIGID'S SUGGESTED PANEL RECESS OF 1-1/2".
2. THE ANCHOR BOLT SETTING PLAN LOCATES ANCHOR BOLTS IN REFERENCE TO THE OUTSIDE OF THE PANEL RECESS SHOWN. IF THE ACTUAL PANEL RECESS IS DIFFERENT FROM WHAT IS SHOWN ON THE ANCHOR BOLT SETTING PLAN, THEN ALL REFERENCE DIMENSIONS FROM THE OUTSIDE OF THE PANEL RECESS MUST BE DETERMINED BY THE CUSTOMER.
3. BOTTOM OF ALL BASE PLATES ARE AT THE SAME ELEVATION. (UNLESS NOTED)

NOTE:

ONLY ANCHOR BOLTS SETTING PLAN ISSUED & STAMPED "FOR CONSTRUCTION" SHALL BE USED IN SETTING ANCHOR BOLTS. 'RIGID GLOBAL BUILDINGS' SHALL NOT BE RESPONSIBLE FOR ERROR OR DISCREPANCY IF THE DRAWING USED IS NOT VALID FOR CONSTRUCTION.

QTY.	SYMBOL	DIA.	PROJ.
0	+	1/2"	1"
4	⊕	5/8"	2"
32	⊕	3/4"	2 1/2"
0	⊕	7/8"	2 3/4"
40	⊕	1"	3"
0	⊕	1 1/8"	3 1/2"
0	⊕	1 1/2"	3 1/2"

ANCHOR BOLT DETAIL

ANCHOR BOLT PROJECTION "PROJ." IS MEASURED FROM BOTTOM OF BASE PLATE

LENGHT OF "PROJ." SHOWN IS FOR ONE NUT + ONE WASHER

DETAIL OF ANCHOR BOLT AS PER THE SUPPLIER

NUTS & WASHERS BY SUPPLIER

ANCHOR BOLTS NOT BY RIGID GLOBAL BUILDINGS

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
0	CONSTRUCTION/PERMIT	11/05/20	JAP	AGP	BKD

GENERAL STEEL CORPORATION

DESCRIPTION	ANCHOR BOLT DETAILS				
CUSTOMER	AMMC Industries				
END USER	AMMC Industries				
END USE	Shop	BUILDING	A		
STREET	19911 Hwy. 550				
CITY ST ZIP	Montrose, CO 81403				
SHEET NO. 68726	JOB NO. 148759	SCALE N.T.S.	ENG. NO. F002	REV. 0	

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	Dead Horiz	Dead Vert	Live Horiz	Live Vert	Snow Horiz	Snow Vert	Wind_Left1 Horiz	Wind_Left1 Vert	Wind_Right1 Horiz	Wind_Right1 Vert	Wind_Left2 Horiz	Wind_Left2 Vert
2*	D	1.5	3.6	5.0	10.0	30.4	60.6	−6.0	−8.9	0.1	−5.4	−5.9	−5.4
2*	A	−1.5	3.6	−5.0	10.0	−30.4	60.5	−0.1	−5.4	6.0	−8.9	−0.3	−1.8

Frame Line	Column Line	Wind_Right2 Horiz	Wind_Right2 Vert	Wind_Long1 Horiz	Wind_Long1 Vert	Wind_Long2 Horiz	Wind_Long2 Vert	Seismic_Left Horiz	Seismic_Left Vert	Seismic_Right Horiz	Seismic_Right Vert	Seismic_Long Horiz	Seismic_Long Vert
2*	D	0.3	−1.8	−0.9	−10.3	−1.5	−9.1	−1.7	−1.1	1.7	1.1	0.0	−4.0
2*	A	5.9	−5.4	1.5	−9.1	0.9	−10.3	−1.7	1.1	1.7	−1.1	0.0	−4.0

Frame Line	Column Line	MIN_SNOW Horiz	MIN_SNOW Vert	F1UNB_SL_L Horiz	F1UNB_SL_L Vert	F1UNB_SL_R Horiz	F1UNB_SL_R Vert
2*	D	5.0	10.0	24.7	61.6	24.5	32.7
2*	A	−5.0	10.0	−24.5	32.7	−24.7	61.6

2* Frame lines: 2 3

RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k) Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin	Bolt(in) Qty	Bolt(in) Dia	Base_Plate(in) Width	Base_Plate(in) Length	Base_Plate(in) Thick	Grout (in)
2*	D	1	31.9	64.1	2	−2.7	−3.2	10	1.000	8.000	23.50	0.500	0.0
		6	26.1	65.2	4	0.3	−4.1						
2*	A	3	2.7	−3.2	1	−31.9	64.1	10	1.000	8.000	23.50	0.500	0.0
		7	−26.1	65.2	5	−0.3	−4.1						

2* Frame lines: 2 3

ENDWALL COLUMN: BASIC COLUMN REACTIONS (k)

Frm Line	Col Line	Dead Vert	Live Vert	Snow Vert	Wind_Left1 Horz	Wind_Left1 Vert	Wind_Right1 Horz	Wind_Right1 Vert	Wind_Left2 Horz	Wind_Left2 Vert	Wind_Right2 Horz	Wind_Right2 Vert	Wind Press Horz	Wind Suct Horz
1	D	0.5	1.1	6.8	0.0	−1.2	0.0	−1.1	0.0	−0.7	0.0	−0.5	−1.0	1.2
1	C	1.3	4.0	24.0	0.0	−4.4	0.0	−2.6	0.0	−3.2	0.0	−1.4	−2.6	2.9
1	B	1.3	4.0	24.0	1.7	−5.1	0.0	−2.2	1.7	−3.8	0.0	−1.0	−2.6	2.9
1	A	0.5	1.1	6.8	0.0	1.4	1.7	−3.4	0.0	2.0	1.7	−2.8	−1.0	1.2

Frm Line	Col Line	Wind_Long1 Horz	Wind_Long1 Vert	Wind_Long2 Horz	Wind_Long2 Vert	Seis_Left Horz	Seis_Left Vert	Seis_Right Horz	Seis_Right Vert	MIN_SNOW Horz	MIN_SNOW Vert	E1UNB_SL_L Horz	E1UNB_SL_L Vert	E1UNB_SL_R Horz	E1UNB_SL_R Vert
1	D	0.0	−1.4	0.0	−0.7	0.0	0.1	0.0	−0.1	0.0	1.1	0.0	7.9	0.0	1.5
1	C	0.0	−4.2	0.0	−2.8	0.0	−0.1	0.0	0.1	0.0	4.0	0.0	28.9	0.0	9.7
1	B	0.0	−2.3	0.4	−4.8	1.9	−2.6	0.0	2.3	0.0	4.0	0.0	9.8	0.1	28.7
1	A	0.4	−1.3	0.0	−0.8	0.0	2.6	1.9	−2.3	0.0	1.1	0.1	1.4	0.0	8.0

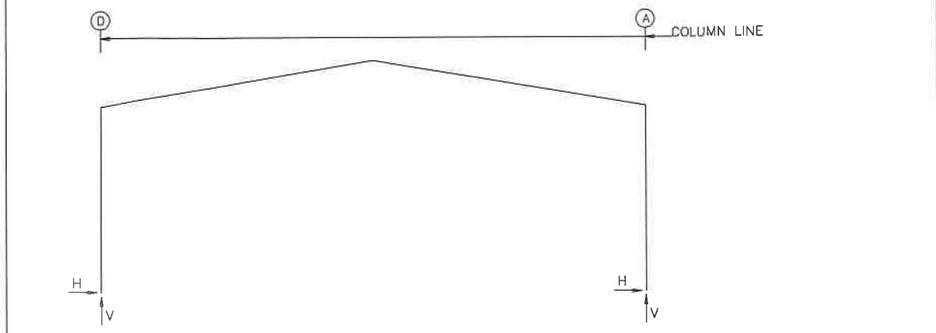
Frm Line	Col Line	Dead Vert	Live Vert	Snow Vert	Wind_Left1 Horz	Wind_Left1 Vert	Wind_Right1 Horz	Wind_Right1 Vert	Wind_Left2 Horz	Wind_Left2 Vert	Wind_Right2 Horz	Wind_Right2 Vert	Wind Press Horz	Wind Suct Horz
4	A	0.5	1.1	6.8	0.0	−1.2	0.0	−1.1	0.0	−0.7	0.0	−0.5	−1.0	1.2
4	B	1.3	4.0	24.0	0.0	−4.4	0.0	−2.6	0.0	−3.2	0.0	−1.4	−2.6	2.9
4	C	1.3	4.0	24.0	1.7	−5.1	0.0	−2.2	1.7	−3.8	0.0	−1.0	−2.6	2.9
4	D	0.5	1.1	6.8	0.0	1.4	1.7	−3.4	0.0	2.0	1.7	−2.8	−1.0	1.2

Frm Line	Col Line	Wind_Long1 Horz	Wind_Long1 Vert	Wind_Long2 Horz	Wind_Long2 Vert	Seis_Left Horz	Seis_Left Vert	Seis_Right Horz	Seis_Right Vert	MIN_SNOW Horz	MIN_SNOW Vert	E2UNB_SL_L Horz	E2UNB_SL_L Vert	E2UNB_SL_R Horz	E2UNB_SL_R Vert
4	A	0.0	−1.4	0.0	−0.7	0.0	0.1	0.0	−0.1	0.0	1.1	0.0	7.9	0.0	1.5
4	B	0.0	−4.2	0.0	−2.8	0.0	−0.1	0.0	0.1	0.0	4.0	0.0	28.9	0.0	9.7
4	C	0.0	−2.3	0.4	−4.8	1.9	−2.6	0.0	2.3	0.0	4.0	0.0	9.8	0.1	28.7
4	D	0.4	−1.3	0.0	−0.8	0.0	2.6	1.9	−2.3	0.0	1.1	0.1	1.4	0.0	8.0

ENDWALL COLUMN: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k) Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin	Bolt(in) Qty	Bolt(in) Dia	Base_Plate(in) Width	Base_Plate(in) Length	Base_Plate(in) Thick	Grout (in)
1	D	8	0.7	−0.6	9	−0.6	−0.6	4	0.750	8.000	8.500	0.500	0.0
		10	0.0	8.3	8	0.7	−0.6						
1	C	11	1.7	−1.9	9	−1.6	−1.8	4	0.750	8.000	8.500	0.500	0.0
		10	0.0	30.1	11	1.7	−1.9						
1	B	11	1.7	−2.3	12	−1.6	−2.1	4	0.750	8.000	8.500	0.500	0.0
		13	0.0	30.0	11	1.7	−2.3						
1	A	14	0.7	−1.8	9	−0.6	−0.5	4	0.750	8.000	8.500	0.500	0.0
		13	0.0	8.4	14	0.7	−1.8						
4	A	8	0.7	−0.6	9	−0.6	−0.6	4	0.750	8.000	8.500	0.500	0.0
		15	0.0	8.3	8	0.7	−0.6						
4	B	11	1.7	−1.9	9	−1.6	−1.8	4	0.750	8.000	8.500	0.500	0.0
		15	0.0	30.1	11	1.7	−1.9						
4	C	11	1.7	−2.3	12	−1.6	−2.1	4	0.750	8.000	8.500	0.500	0.0
		16	0.0	30.0	11	1.7	−2.3						
4	D	14	0.7	−1.8	9	−0.6	−0.5	4	0.750	8.000	8.500	0.500	0.0
		16	0.0	8.4	14	0.7	−1.8						

FRAME LINES: 2 3



FOR PERMIT

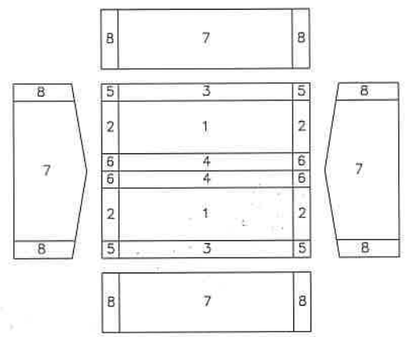
NOTES FOR REACTIONS

1. All loading conditions are examined and only maximum/minimum H or V and the corresponding H or V are reported.
2. Positive reactions are as shown in the sketch. Foundation loads are in opposite directions.
3. Bracing reactions are in the plane of the brace with the H pointing away from the braced bay. The vertical reaction is downward.
4. Building reactions are based on the following building data.

Width (ft)	: 50
Length (ft)	: 60
Eave Height (ft)	: 17 / 17
Roof Slope (rise/12)	: 2.0:12 / 2.0:12
Design Code	: IBC 18
Enclosure	: Closed
Dead Load (psf)	: 4.00
Collateral Load (psf)	: 0.00
Basic Design Wind Speed (mph)	: V (3 sec. gust) = 115.00 mph
Allowable Stress Wind Speed (mph)	: Vasd (3 sec. gust) = 89.08 mph
Wind Importance Factor	: 1.000
Wind Exposure	: C
Live Load (psf)	: 20.00
Frame Live Load (psf)	: 20.00
Ground Snow Load (psf)	: 173.00
Roof Snow Load (psf)	: 121.10
Snow Exposure	: 1.000
Snow Importance Factor	: 1.000
Thermal Factor	: 1.000
Seismic Importance Factor	: 1.000
Spectral Response Accel.	: Ss=0.330 : S1=0.075
Spectral Response Coeff.	: Sds=0.337 : Sd1=0.120
Seismic Coeff. (Fa*Ss)	: 0.506 : Fa=1.537
Seismic Design Category	: C

5. Loading conditions are:
   1 Dead+Collateral+Snow+Slide_Snow
   2 0.6Dead+0.6Wind_Left1
   3 0.6Dead+0.6Wind_Right1
   4 0.6Dead+0.6Wind_Long1L
   5 0.6Dead+0.6Wind_Long2L
   6 Dead+Collateral+F1UNB_SL_L
   7 Dead+Collateral+F1UNB_SL_R
   8 0.6Dead+0.6Wind_Suction+0.6Wind_Long1L
   9 0.6Dead+0.6Wind_Pressure+0.6Wind_Long1L
   10 Dead+Collateral+E1UNB_SL_L
   11 0.6Dead+0.6Wind_Left1+0.6Wind_Suction
   12 0.6Dead+0.6Wind_Pressure+0.6Wind_Long2L
   13 Dead+Collateral+E1UNB_SL_R
   14 0.6Dead+0.6Wind_Right1+0.6Wind_Suction
   15 Dead+Collateral+E2UNB_SL_L
   16 Dead+Collateral+E2UNB_SL_R

Zone	Width (ft)	Length (ft)	Components & Cladding (Factored) Pressure (psf) Member	Pressure (psf) Panel	Suction (psf) Member	Suction (psf) Panel
1			10.00	10.00	−10.00	−25.81
2		5.00	10.00	10.00	−24.67	−37.69
3	5.00		10.00	10.00	−10.00	−25.81
4	5.00		10.00	10.00	−24.67	−37.69
5	5.00	5.00	10.00	10.00	−24.67	−37.69
6	5.00	5.00	10.00	10.00	−27.94	−44.65
7			10.92	12.78	−12.00	−13.66
8	5.00		10.92	12.78	−13.32	−17.05



Design Calculation Wind

BUILDING BRACING REACTIONS

Wall Loc	Wall Line	Col Line	± Reactions(k) Wind Horz	Wind Vert	Seismic Horz	Seismic Vert	Panel_Shear (lb/ft) Wind	Panel_Shear (lb/ft) Seis
L_EW	1	B,A	1.7	2.1	1.9	2.3		
F_SW	A	2,3	3.5	2.7	5.3	4.0		
R_EW	4	C,D	1.7	2.1	1.9	2.3		
B_SW	D	3,2	3.5	2.7	5.3	4.0		

COLORADO LICENSED CARLITO M. DAYAG 46701 PROFESSIONAL ENGINEER

11/5/20

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ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
0	CONSTRUCTION/PERMIT	11/05/20	JAP	MBS	BKD

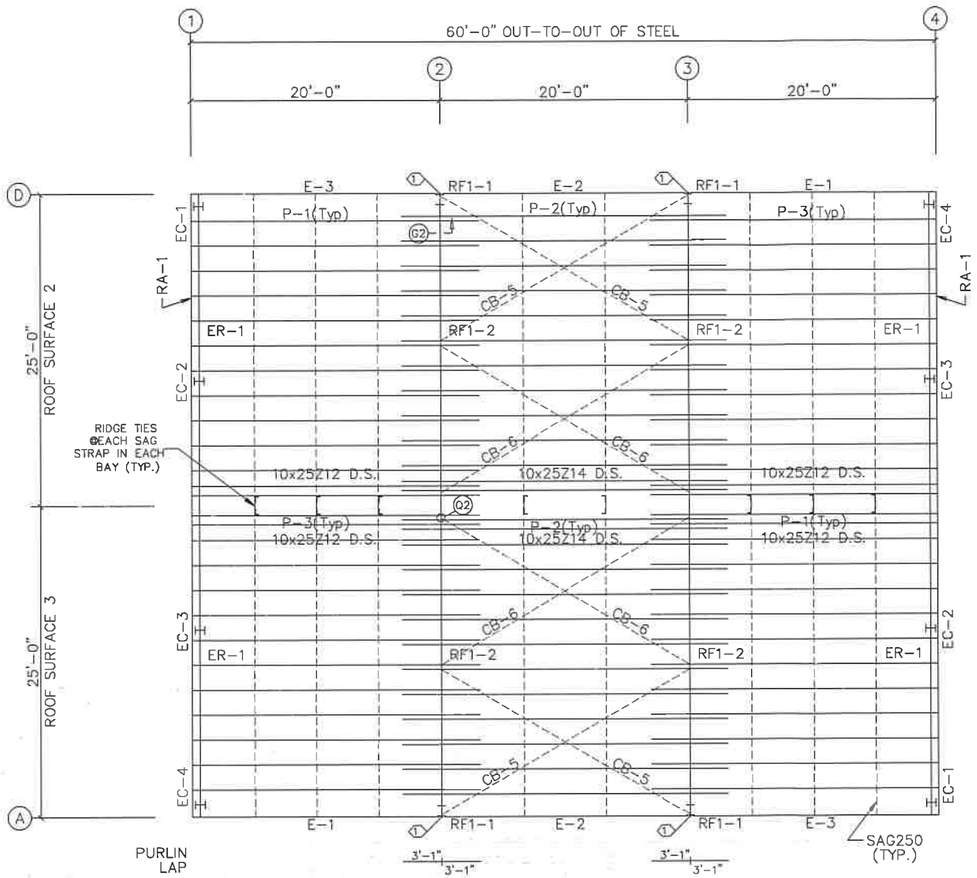
GENERAL STEEL CORPORATION

DESCRIPTION	ANCHOR BOLT REACTIONS
CUSTOMER	AMMC Industries
END USER	AMMC Industries
END USE	Shop BUILDING A
STREET	19911 Hwy. 550
CITY ST ZIP	Montrose, CO 81403
ORDER NO.	68726
SEQ. NO.	148759
SCALE	N.T.S.
DWG. NO.	F003
ISSUE	0

SPECIAL BOLTS ROOF PLAN					
O ID	QUAN	TYPE	DIA	LENGTH	WASH
1	4	A307	1/2"	1 1/4"	0

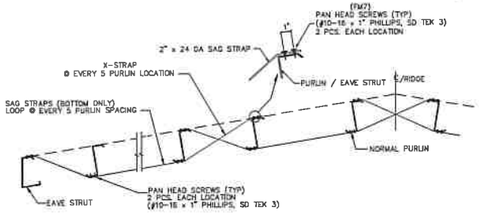
MEMBER TABLE ROOF PLAN	
MARK	PART
P-1	10x25Z12
P-2	10x25Z14
P-3	10x25Z12
E-1	L10x5x3ES14
E-2	L10x5x3ES14
E-3	L10x5x3ES14
CB-5	CB0313
CB-6	CB0250

FOR PERMIT

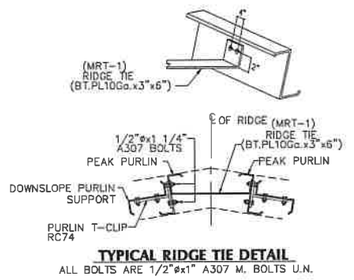


ROOF FRAMING PLAN

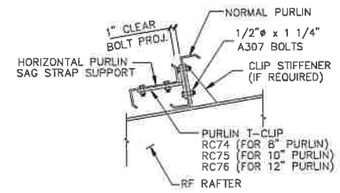
NOTE: 1. USE (4) 1/2" DIAMETER A307 M. BOLTS AT PURLIN TO RAFTER CONNECTION ALONG FRAME LINE 1, 2, 3 & 4
2. WITH 6" THICK VR TYPE ROOF INSULATION BY OTHERS



TYPICAL SAG STRAP DETAIL @ ROOF (BOTTOM ONLY)



TYPICAL RIDGE TIE DETAIL
ALL BOLTS ARE 1/2"ø x1" A307 M. BOLTS U.N.



1 SF2.4 HORIZONTAL PURLIN CONNECTION TO RF RAFTER
ALL BOLTS ARE 1/2"ø x 1" A307 U.N.

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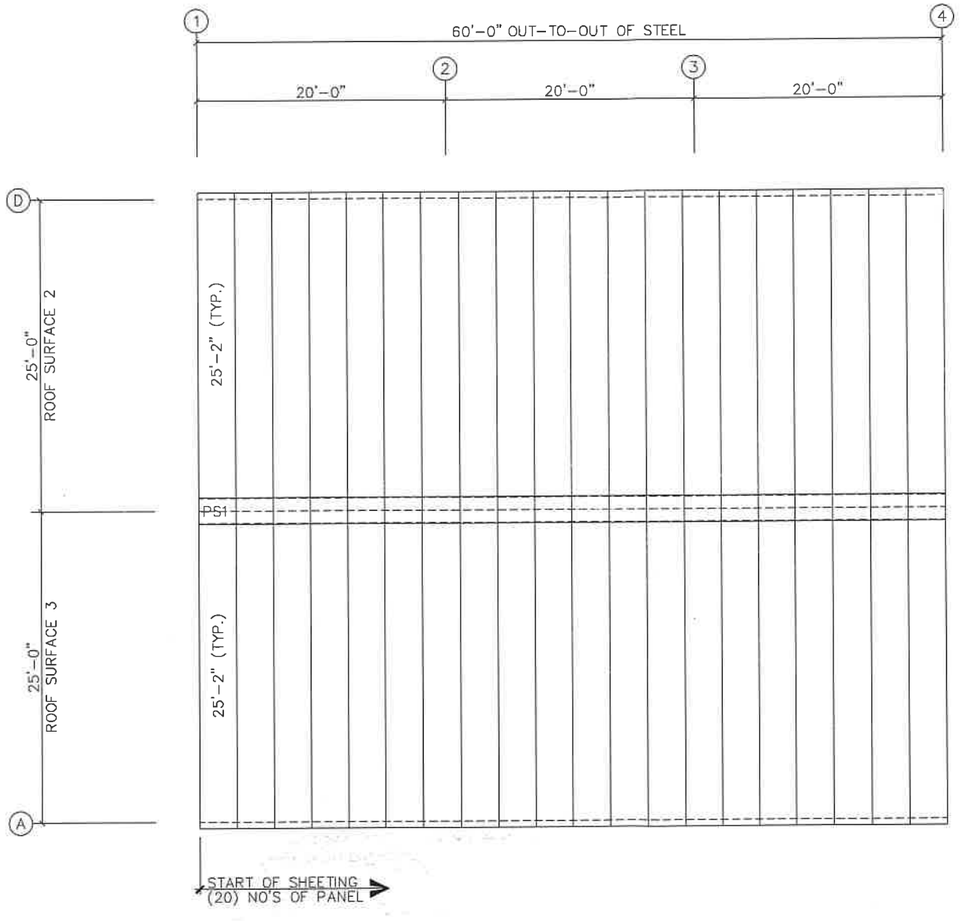
ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
A	PERMIT	11/05/20	JAP	AGP	BKD

GENERAL STEEL CORPORATION

DESCRIPTION	ROOF FRAMING PLAN			
CUSTOMER	AMMC Industries			
END USER	AMMC Industries			
END USE	Shop	BUILDING	A	
STREET	19911 Hwy. 550			
CITY ST ZIP	Montrose, CO 81403			
SALES ORD. 68726	JOB NO. 148759	SCALE N.T.S.	DWG. NO. E001	REV. A

FOR PERMIT



ROOF SHEETING PLAN

PANELS: 26 Ga. PBR – Glvm.Plus

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ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
A	PERMIT	11/05/20	JAP	AGP	BKD

GENERAL STEEL CORPORATION

DESCRIPTION	ROOF SHEETING PLAN		
CUSTOMER	AMMC Industries		
END USER	AMMC Industries		
END USE	Shop	BUILDING	A
STREET	19911 Hwy. 550		
CITY ST ZIP	Montrose, CO 81403		
PROJECT NO. 68726	SALES NO. 148759	SCALE N.T.S.	DWG. NO. E002 REV. A

SPLICE PLATE & BOLT TABLE									
Mark	Qty Top	Bot	Int	Type	Dia	Length	Width	Thick	Length
SP-1	4	4	2	A325	0.875	2.50	8"	5/8"	3'-10 7/16"
SP-2	4	4	0	A325	0.750	2.25	8"	1/2"	1'-6 15/16"

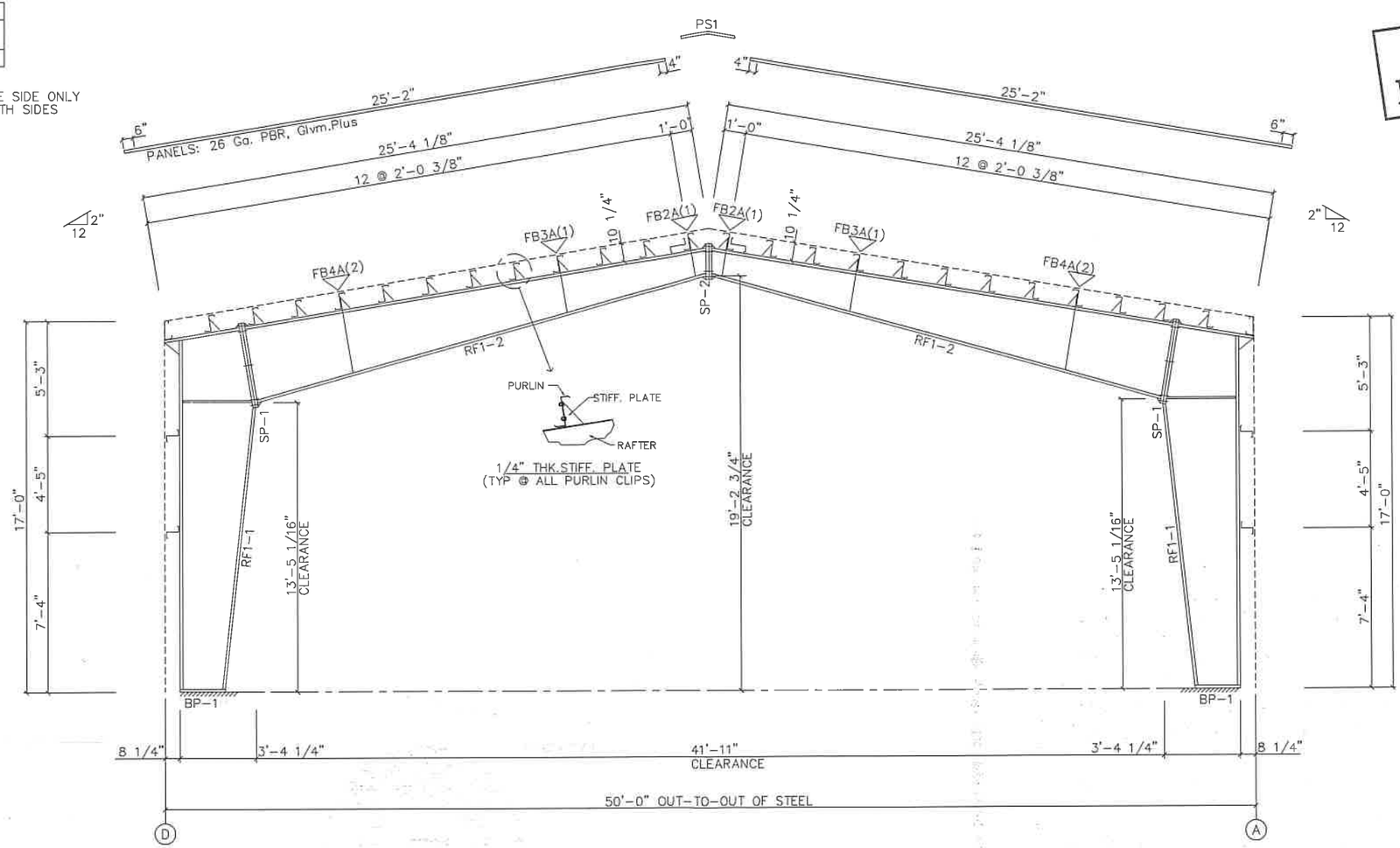
STIFFENER TABLE				
Mark	Stiff Mark	Plate Size Width	Thick	Length
RF1-1	St- 1	3.750	0.250	38.47

BASE PLATE TABLE			
Col Mark	Plate Size Width	Thick	Length
BP-1	8"	1/2"	1'-11 1/2"

FBXXA(1)=FLANGE BRACE AT ONE SIDE ONLY
FBXXA(2)=FLANGE BRACE AT BOTH SIDES
A - L2x2x14

MEMBER TABLE					
Mark	Web Depth Start/End	Web Plate Thick	Length	Outside Flange W x Thk x Length	Inside Flange W x Thk x Length
RF1-1	21.5/39.5	0.188	157.3	8 x 1/4" x 194.2	8 x 1/2" x 158.2
	39.5/32.4	0.313	42.4	8 x 1/4" x 41.5	
RF1-2	39.5/14.0	0.250	240.0	8 x 5/16" x 260.9	8 x 3/8" x 260.2
	14.0/12.0	0.250	20.9		

FOR PERMIT



RIGID FRAME ELEVATION: FRAME LINE 2 3

NOTE: A325 high strength bolt shall be tightened with one washer. Refer to general notes 1.5 and 1.6 on cover sheet for tightening methods and installation inspections.

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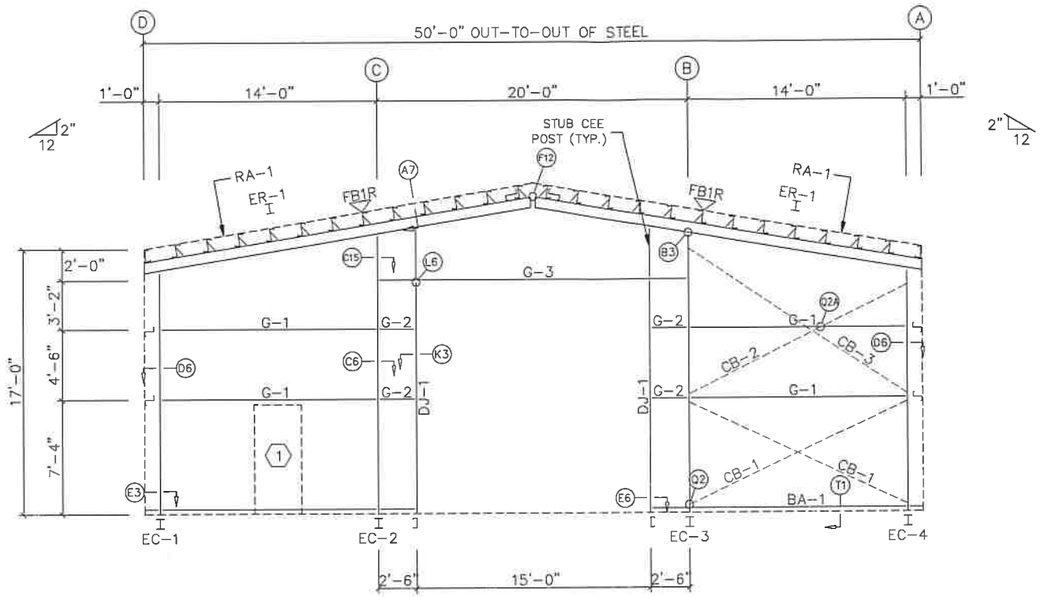
A	PERMIT	11/05/20	JAP	AGP	BKD

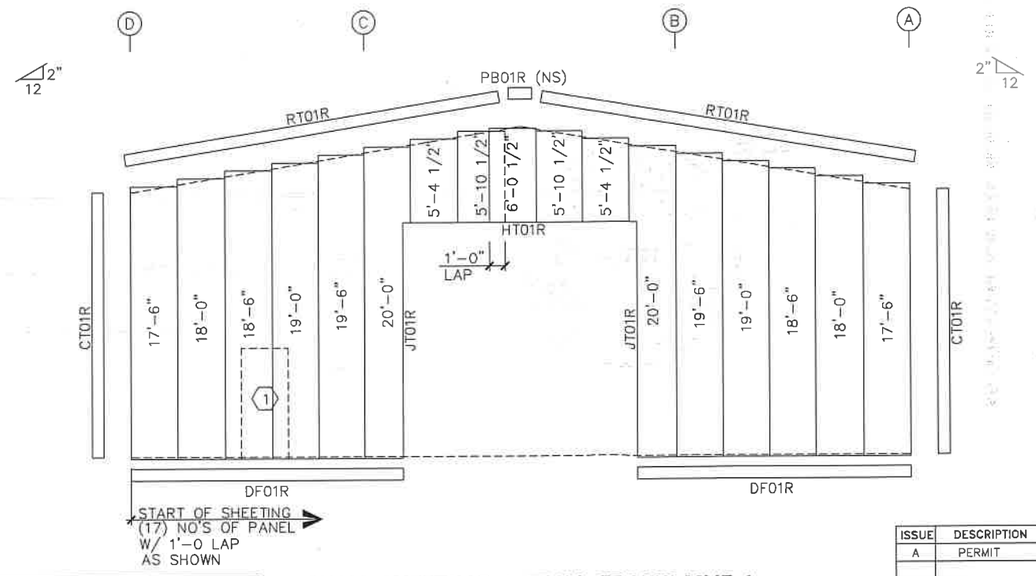
GENERAL STEEL CORPORATION

DESCRIPTION	RIGID FRAME ELEVATION		
CUSTOMER	AMMC Industries		
END USER	AMMC Industries		
END USE	Shop	BUILDING	A
STREET	19911 Hwy. 550		
CITY ST ZIP	Montrose, CO 81403		
PROJ. NO. 68726	ARC NO. 148759	SCALE N.T.S.	DWG. NO. E003 REV. A



ENDWALL FRAMING: FRAME LINE 1

(1) DENOTES WALKDOOR FIELD LOCATED WITH 6" THICK VR TYPE WALL INSULATION BY OTHERS



ENDWALL SHEETING & TRIM: FRAME LINE 1

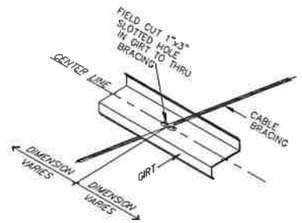
PANELS: 26 Go. PR – KOKO BROWN

NOTE: A325 high strength bolt shall be tightened with one washer. Refer to general notes 1.5 and 1.6 on cover sheet for tightening methods and installation inspections.

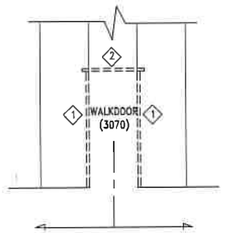
BOLT TABLE FRAME LINE 1				
LOCATION	QUAN	TYPE	DIA	LENGTH
ER-1/ER-1	8	A325	5/8"	1 3/4"
Columns/Raf	6	A325	1/2"	1 1/4"

FLANGE BRACE TABLE FRAME LINE 1		
▽ID	MARK	LENGTH
1	FB1R	1'-6"

MEMBER TABLE FRAME LINE 1	
MARK	PART
EC-1	W8x10
EC-2	W8x10
EC-3	W8x10
EC-4	W8x10
ER-1	W12542
DJ-1	8x25C16
G-1	8x25Z16
G-2	8x25Z16
G-3	8x35C12
CB-1	CB0313
CB-2	CB0313
CB-3	CB0313



Q2A **CABLE BRACING THRU GIRT**



F.O TRIM DETAILS

(FOR FIELD LOCATE WALKDOOR/ 1 PC.)
(ERECTOR TO FIELD VERIFY LOCATION)

TRIM TABLE		
▽ID	MARK	LENGTH
1	JT01R	7'-3"
2	HT01R	3'-6"

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ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
A	PERMIT	11/05/20	JAP	AGP	BKD

GENERAL STEEL CORPORATION

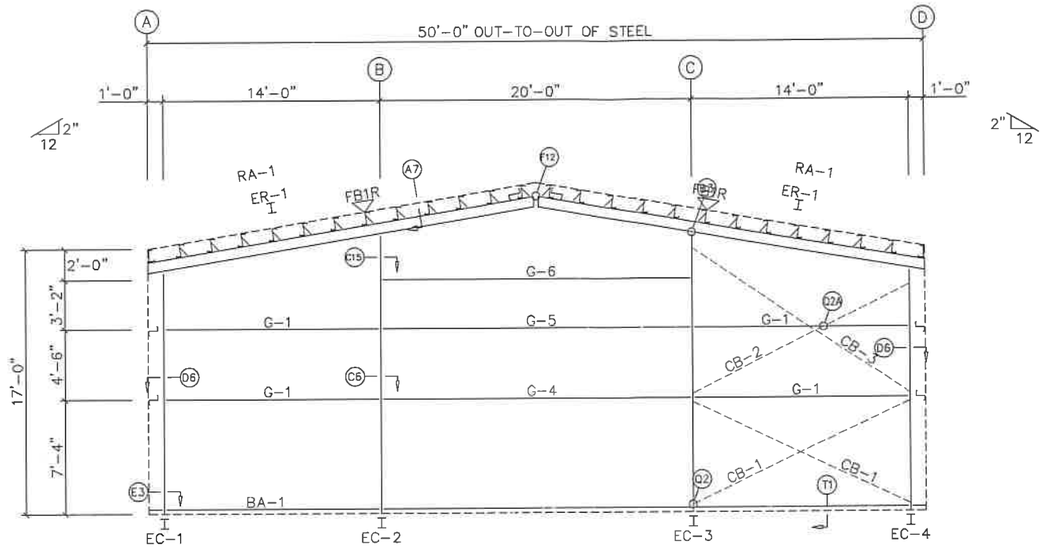
DESCRIPTION	ENDWALL FRAMING, SHEETING & TRIMS
CUSTOMER	AMMC Industries
END USER	AMMC Industries
END USE	Shop / BUILDING / A
STREET	19911 Hwy. 550
CITY ST ZIP	Montrose, CO 81403
MASTER NO.	68726
JOB NO.	148759
SCALE	N.T.S.
DWG. NO.	E004
ISSUE	A

BOLT TABLE FRAME LINE 4				
LOCATION	QUAN	TYPE	DIA	LENGTH
ER-1/ER-1	8	A325	5/8"	1 3/4"
Columns/Raf	6	A325	1/2"	1 1/4"

FLANGE BRACE TABLE FRAME LINE 4		
VID	MARK	LENGTH
1	FB1R	1'-6"

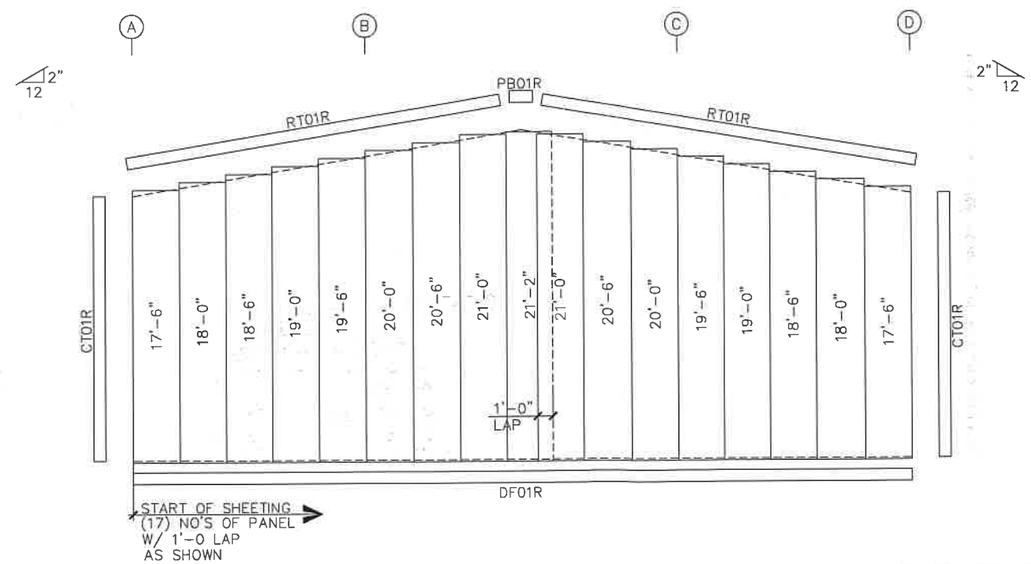
MEMBER TABLE FRAME LINE 4	
MARK	PART
EC-1	W8x10
EC-2	W8x10
EC-3	W8x10
EC-4	W8x10
ER-1	W12542
G-1	8x25Z16
G-4	8x35Z14
G-5	8x25Z16
G-6	8x25Z14
CB-1	CB0313
CB-2	CB0313
CB-3	CB0313

FOR PERMIT



ENDWALL FRAMING: FRAME LINE 4

WITH 6" THICK VR TYPE WALL INSULATION BY OTHERS



ENDWALL SHEETING & TRIM: FRAME LINE 4

PANELS: 26 Ga. PR – KOKO BROWN

NOTE: A325 high strength bolt shall be tightened with one washer. Refer to general notes 1.5 and 1.6 on cover sheet for tightening methods and installation inspections.

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ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
A	PERMIT	11/05/20	JAP	AGP	BKD

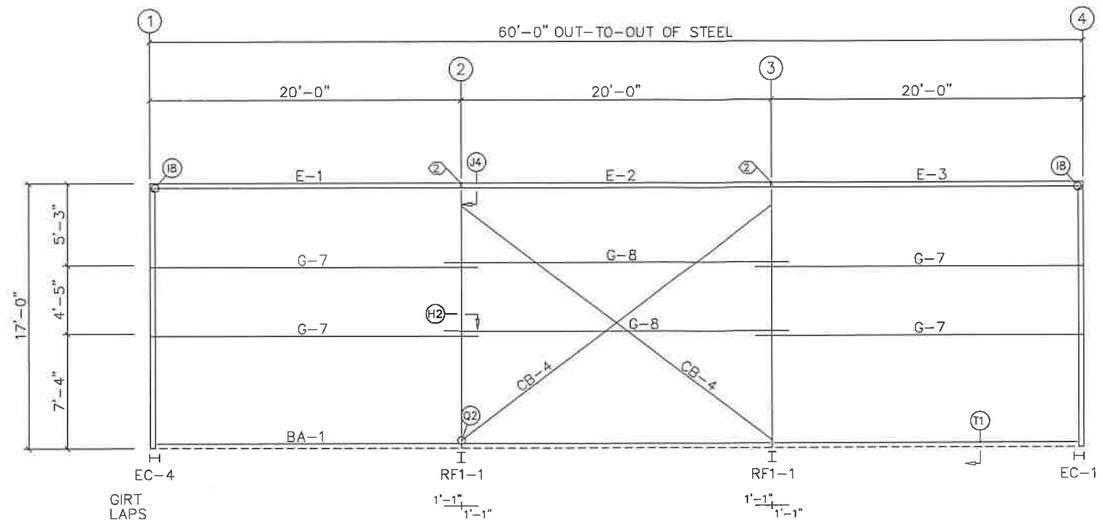
GENERAL STEEL CORPORATION

DESCRIPTION	ENDWALL FRAMING, SHEETING & TRIMS			
CUSTOMER	AMMC Industries			
END USER	AMMC Industries			
END USE	Shop	BUILDING	A	
STREET	19911 Hwy. 550			
CITY ST ZIP	Montrose, CO 81403			
SALES NO. 68726	JOB NO. 148759	SCALE N.T.S.	DWG. NO. E005	REV. A

SPECIAL BOLTS					
O ID	QUAN	TYPE	DIA	LENGTH	WASH
2	4	A307	1/2"	1 1/4"	0

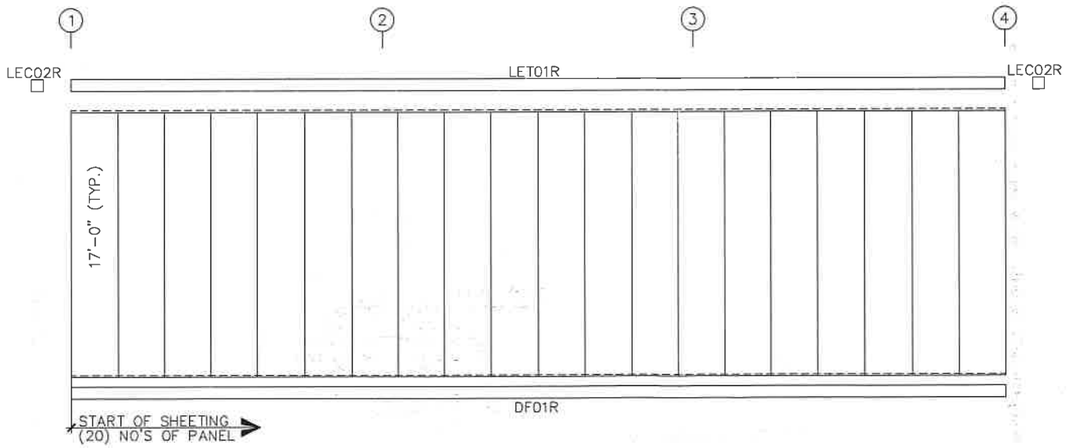
MEMBER TABLE FRAME LINE A	
MARK	PART
E-1	L10x5x3ES14
E-2	L10x5x3ES14
E-3	L10x5x3ES14
G-7	8x25Z16
G-8	8x25Z16
CB-4	CB0500

FOR PERMIT



SIDEWALL FRAMING: FRAME LINE A

WITH 6" THICK VR TYPE WALL INSULATION BY OTHERS



SIDEWALL SHEETING & TRIM: FRAME LINE A

PANELS: 26 Ga. PBR – KOKO BROWN

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ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
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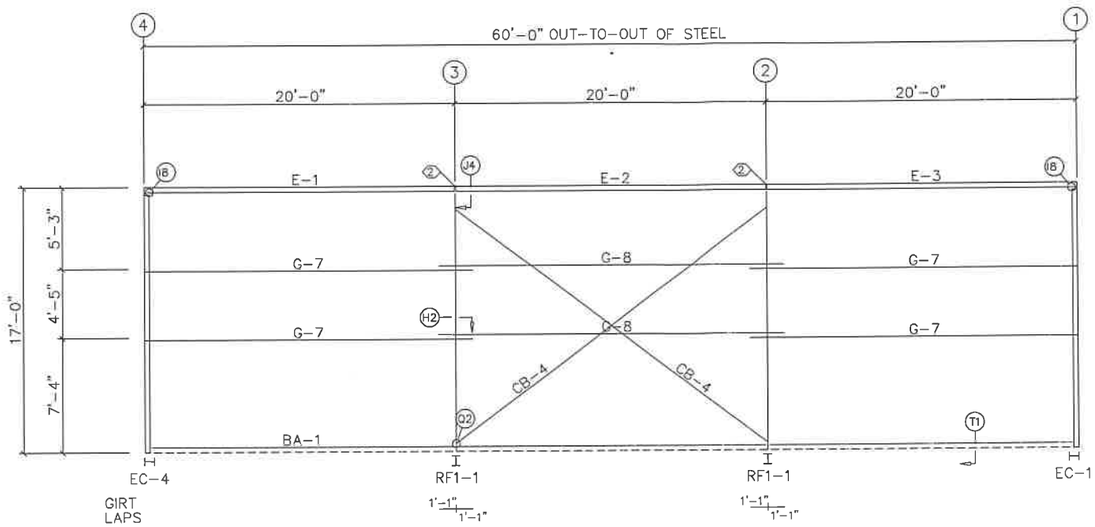
GENERAL STEEL CORPORATION

DESCRIPTION	SIDEWALL FRAMING, SHEETING & TRIMS			
CUSTOMER	AMMC Industries			
END USER	AMMC Industries			
END USE	Shop	BUILDING	A	
STREET	19911 Hwy. 550			
CITY ST ZIP	Montrose, CO 81403			
SALES NO. 68726	JOB NO. 148759	SCALE N.T.S.	DWG. NO. E006	REV. A

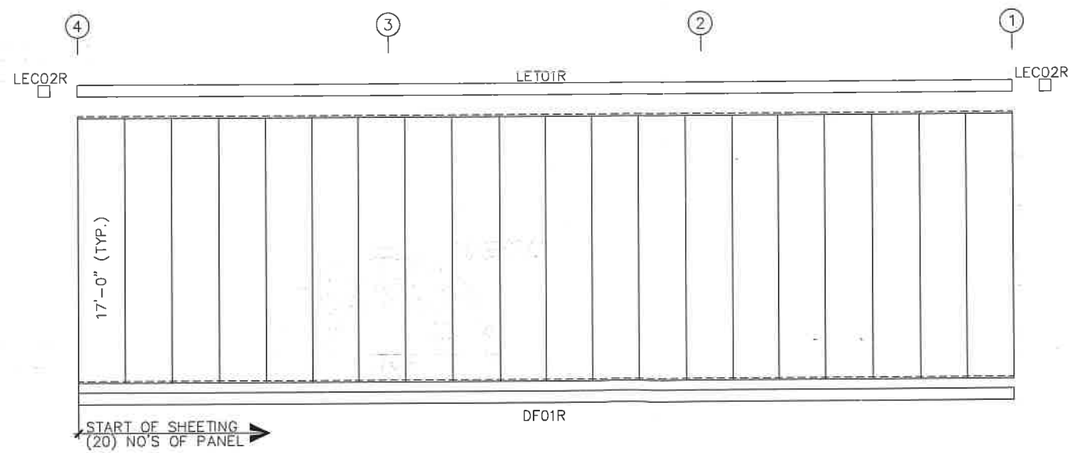
SPECIAL BOLTS					
O ID	QUAN	TYPE	DIA	LENGTH	WASH
2	4	A307	1/2"	1 1/4"	0

MEMBER TABLE FRAME LINE D	
MARK	PART
E-1	L10x5x3ES14
E-2	L10x5x3ES14
E-3	L10x5x3ES14
G-7	8x25Z16
G-8	8x25Z16
CB-4	CB0500



SIDEWALL FRAMING: FRAME LINE D

WITH 6" THICK VR TYPE WALL INSULATION BY OTHERS



SIDEWALL SHEETING & TRIM: FRAME LINE D

PANELS: 26 Ga. PBR – KOKO BROWN

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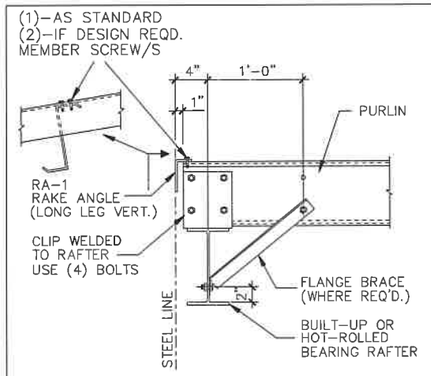
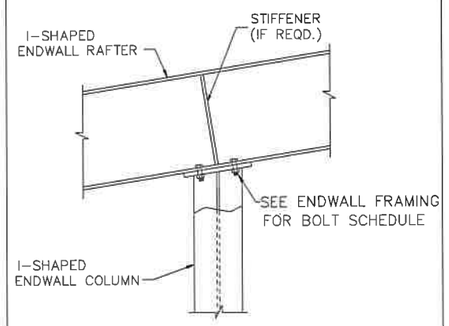
ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
A	PERMIT	11/05/20	JAP	AGP	BKD

GENERAL STEEL CORPORATION

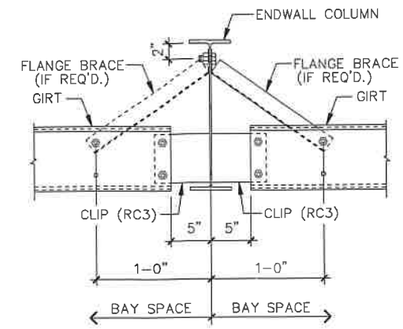
DESCRIPTION	SIDEWALL FRAMING, SHEETING & TRIMS			
CUSTOMER	AMMC Industries			
END USER	AMMC Industries			
END USE	Shop	BUILDING	A	
STREET	19911 Hwy. 550			
CITY ST ZIP	Montrose, CO 81403			
SALES NO. 68726	DR NO. 148759	SCALE N.T.S.	DWG NO. E007	ISSUE A



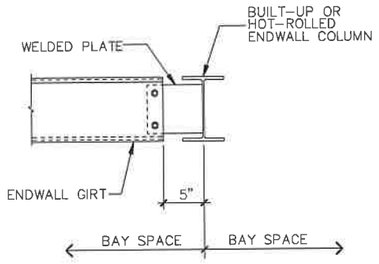
A7 **BEARING FRAME TO FLUSH ENDWALL**
ALL BOLTS ARE 1/2"ø x 1" A307 U.N.



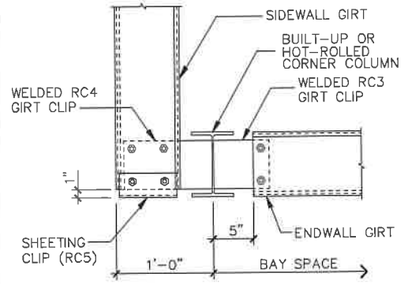
B3 **ENDWALL RAFTER TO COLUMN**
ALL BOLTS AS NOTED



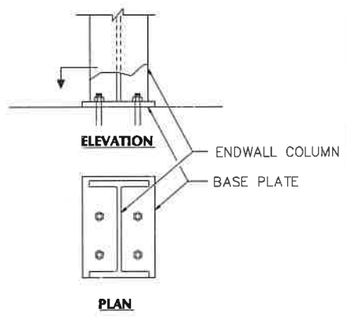
C6 **FLUSH GIRT TO ENDWALL COLUMN WITH WELDED CLIP**
ALL BOLTS ARE 1/2"ø x 1" A307 U.N.



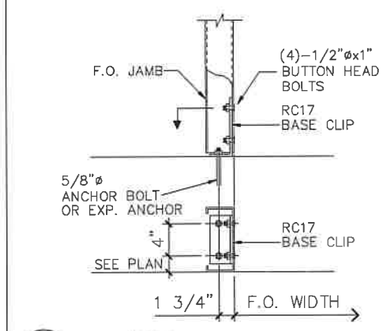
C15 **ENDWALL COLUMN TO GIRT**
ALL BOLTS ARE 1/2"ø x 1" A307 U.N.



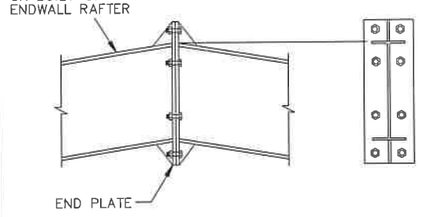
D6 **BUILT-UP CORNER COLUMN OR HOT-ROLLED CORNER COLUMN**
ALL BOLTS ARE 1/2"ø x 1" A307 U.N.



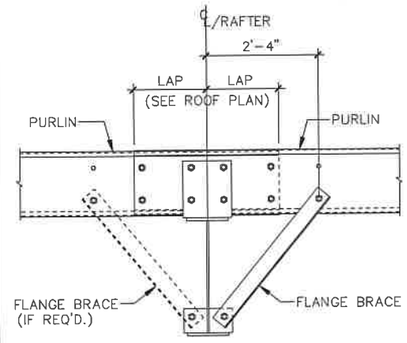
E3 **BASE PLATE FOR ENDWALL COLUMN**
SEE ANCHOR BOLT PLAN & DETAILS



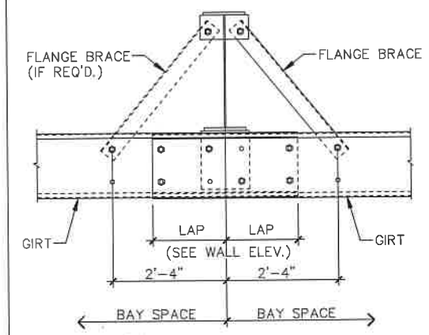
E6 **"FO" JAMB BASE DETAIL**
WITH BOLTED BASE CLIP



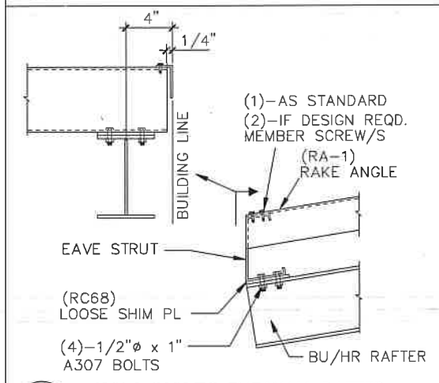
F12 **RAFTER SPLICE ALONG SURFACE HOT-ROLLED OR BUILT-UP RAFTER**
SEE ENDWALL FRAMING ELEV.
FOR BOLT DIA AND TYPE.



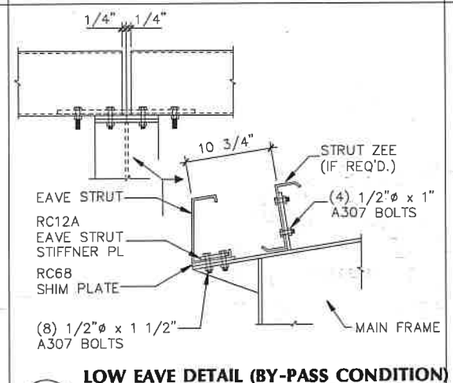
G2 **BY-PASS PURLIN TO RAFTER DETAIL**
ALL BOLTS ARE 1/2"ø x 1" A307 U.N.



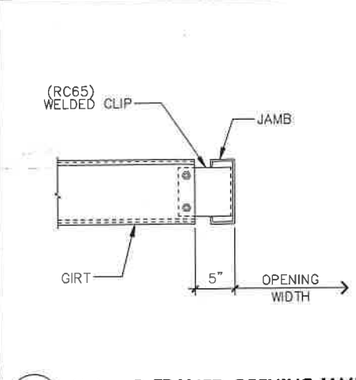
H2 **WALL GIRT TO RF COLUMN**
ALL BOLTS ARE 1/2"ø x 1" A307 U.N.



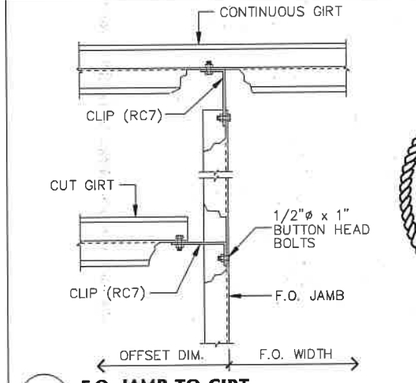
I8 **EAVE STRUT TO ENDWALL RAFTER**
LEDS



J4 **LOW EAVE DETAIL (BY-PASS CONDITION) WITH EAVE STRUT STIFFENER PLATE**
AT INTERIOR FRAME



K3 **GIRT TO FRAMED OPENING JAMB**
ALL BOLTS ARE 1/2"ø x 1" A307 U.N.



L6 **F.O. JAMB TO GIRT**
ALL BOLTS ARE 1/2"ø x 1" A307 U.N.

11/5/20

COLORADO LICENSED
CARLITO M. DAYAG
46701
PROFESSIONAL ENGINEER

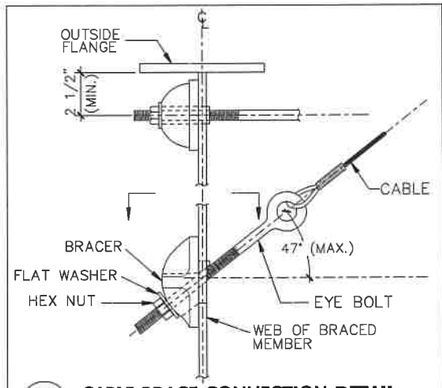
SEALING OF THIS DRAWING DOES NOT IMPLY OR CONSTITUTE THAT GENERAL STEEL ENGINEER IS THE ENGINEER OF RECORD OR THE DESIGN PROFESSIONAL FOR THIS PROJECT. ONLY THE DESIGN OF THE METAL BUILDING SYSTEM AS FURNISHED BY G.S.C. IS INCLUDED. FOUNDATION ANALYSIS, ELECTRICAL, AND MECHANICAL SYSTEMS, AND/OR OTHER PARTS SUPPLIED BY ANYONE OTHER THAN G.S.C. ARE SPECIFICALLY EXCLUDED. NO INSPECTION OR SUPERVISION IS IMPLIED.

FOR PERMIT

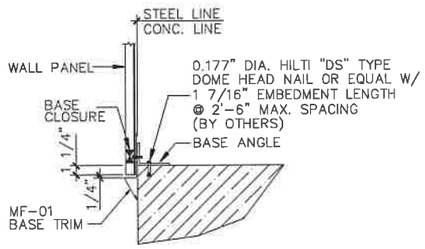
ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
A	PERMIT	11/05/20	JAP	AGP	BKD

GENERAL STEEL CORPORATION

DESCRIPTION	DETAIL DRAWINGS 1		
CUSTOMER	AMMC Industries		
END USER	AMMC Industries		
END USE	Shop	BUILDING	A
STREET	19911 Hwy. 550		
CITY ST ZIP	Montrose, CO 81403		
68726	148759	N.T.S.	E008 A



Q2 **CABLE BRACE CONNECTION DETAIL**



T1 **SECTION THRU WALL PANEL AND CONCRETE FOUNDATION**

FOR PERMIT

11/5/20

COLORADO LICENSED
CARLITO M. DAYAG
46701
PROFESSIONAL ENGINEER

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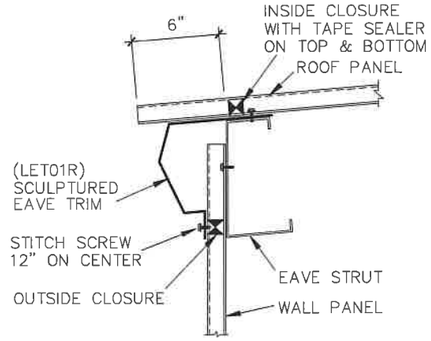
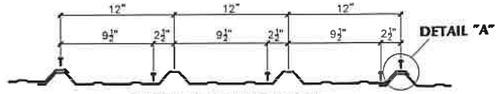
ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
A	PERMIT	11/05/20	JAP	AGP	BKD

GENERAL STEEL CORPORATION

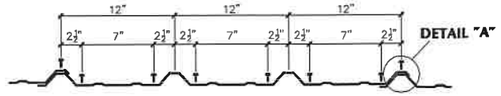
DESCRIPTION	DETAIL DRAWINGS 2
CUSTOMER	AMMC Industries
END USER	AMMC Industries
END USE	Shop / BUILDING: A
STREET	19911 Hwy. 550
CITY ST ZIP	Montrose, CO 81403
SHEET NO. 68726	JOB NO. 148759 / SCALE: N.T.S. / DWG. NO. E009 / REV. A



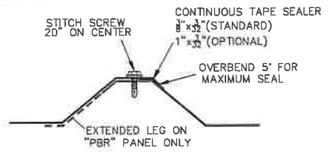
LOW EAVE DETAIL WITH SCULPTURED EAVE TRIM
SHEETED WALL



MEMBER SCREW LOCATION
FOR ALL MEMBERS UNLESS NOTED OTHERWISE

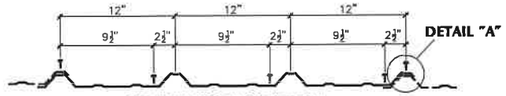


MEMBER SCREW LOCATION
FOR PEAK PURLIN, EAVE STRUT, AND PANEL END LAPS ONLY

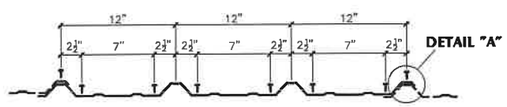


DETAIL "A"
TYP. SIDE LAP DETAIL

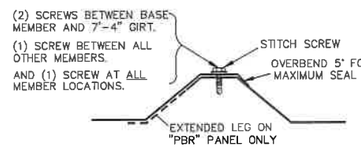
"R" and "PBR" ROOF PANEL SCREW LAYOUT



MEMBER SCREW LOCATION
FOR ALL MEMBERS UNLESS NOTED OTHERWISE



MEMBER SCREW LOCATION
FOR BASE, EAVE STRUT, AND PANEL END LAPS ONLY



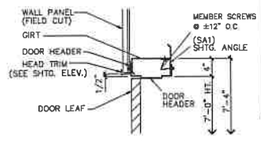
DETAIL "A"
TYP. SIDE LAP DETAIL

"R" and "PBR" WALL PANEL SCREW LAYOUT

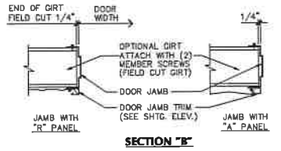
NOTES:
1. **FOR 3078, 4678, 6078 WALK DOORS ONLY**
2. **ALL DOORS ARE FIELD LOCATED UNLESS SHOWN IN A.B. PLAN**
3. **a - DIMENSION VARIES. SEE WALL ELEVATION IF REQUIRED**

INSTALLATION PROCEDURE

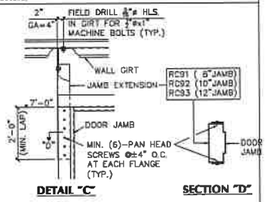
1. Place head section and jambs on flat surface (floor) with door side up. Install bolts and nuts connecting head to jambs. Be sure that head is tight to jambs so that the proper door opening is obtained.
2. Install door leaf in frame, check for 1/8" clearance at head and 3/32" clearance at stricker jamb.
3. Tilt up the entire assembly and anchor hinge jamb to floor. Plumb hinge jamb and assembly. Field cut girts if required.
4. Anchor head and striker jamb to building structure, floor and entire frame to panel skins (field cut). Install optional threshold anchor if desired. Install jamb extensions (if req'd).
5. Install lockset. Install (optional) weatherstrip, head member first. Adjust so that vinyl just contacts door when in the closed position. Do not force vinyl against door as this will interfere with the latching and will not improve the weather seal.
6. Refer also to Door Manufacturer Installation Manual for more details.



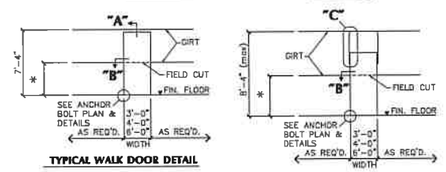
SECTION "A"



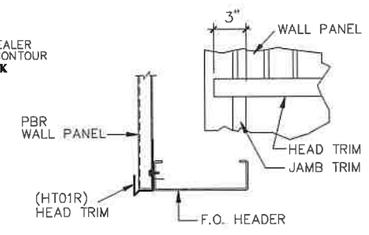
SECTION "B"



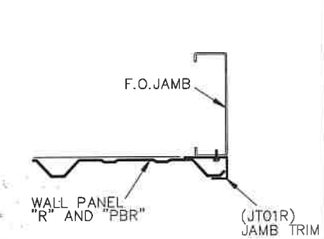
DETAIL "C" **SECTION "D"**



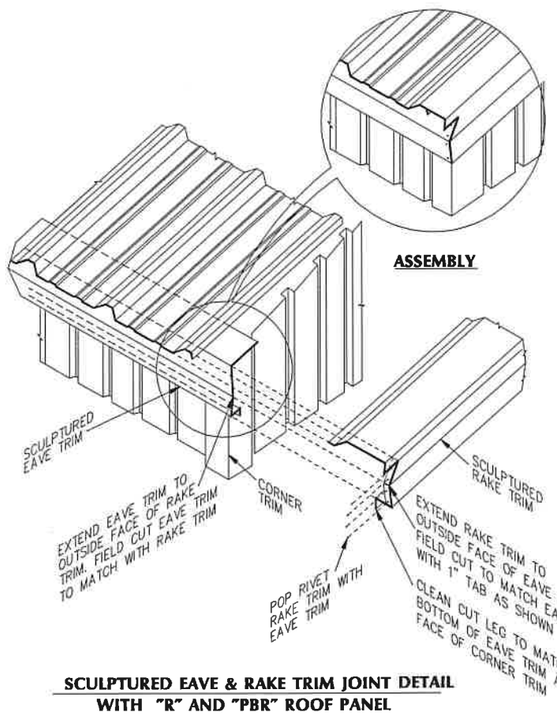
TYPICAL WALK DOOR DETAIL



F.O. HEADER TRIM DETAIL

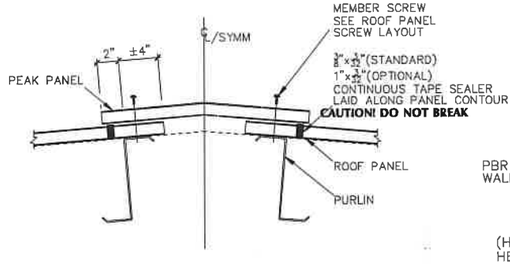


F.O. JAMB TRIM DETAIL

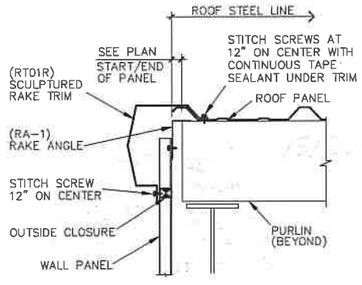


SCULPTURED EAVE & RAKE TRIM JOINT DETAIL
WITH "R" AND "PBR" ROOF PANEL

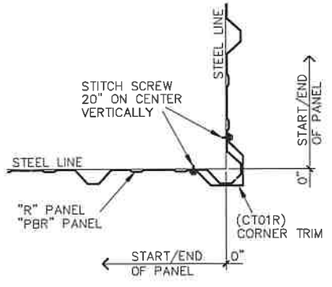
FOR PERMIT



PEAK PANEL DETAIL



SCULPTURED RAKE DETAIL WITH SHEETED WALL
"R" AND "PBR" ROOF PANEL



OUTSIDE CORNER DETAIL
ON MODULE

11/5/20

COLORADO LICENSED
CARLITO M. DAYAG
46701
PROFESSIONAL ENGINEER

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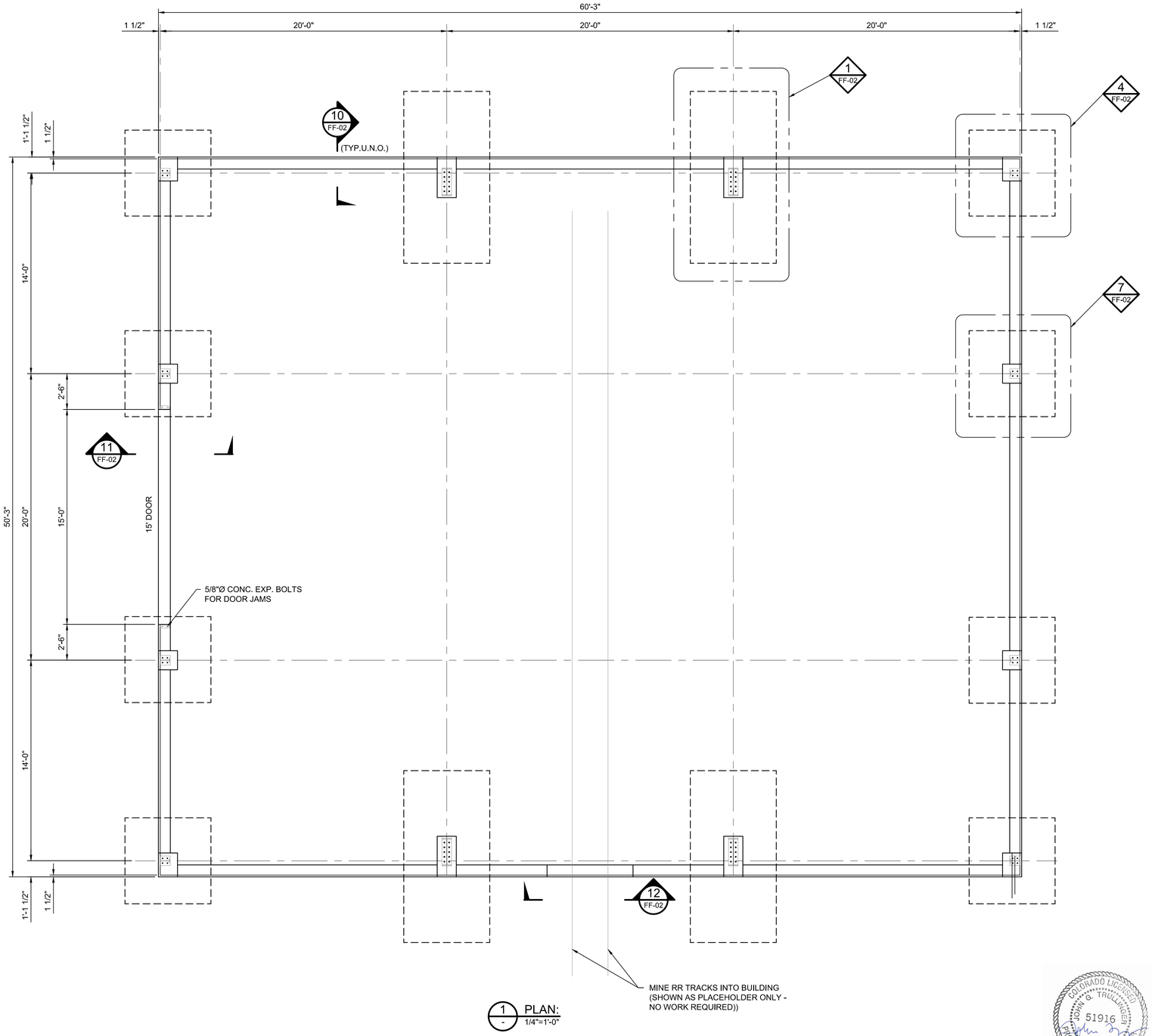
ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
A	PERMIT	11/05/20	JAP	AGP	BKD

GENERAL STEEL CORPORATION

DESCRIPTION	PANEL PROFILE, TRIMS & ACCESSORIES		
CUSTOMER	AMMC Industries		
END USER	AMMC Industries		
END USE	Shop	BUILDING	A
STREET	19911 Hwy. 550		
CITY ST ZIP	Montrose, CO 81403		
SALES ORD. 68726	JOB NO. 148759	SCALE N.T.S.	DWG. NO. E010
			REV. A



1 - PLAN: 1/4"=1'-0"

CONCRETE NOTES

1. REINFORCED CONCRETE DESIGN AND CONSTRUCTION SHALL BE IN CONFORMANCE WITH THE "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE, ACI 318-14", AS PUBLISHED BY THE AMERICAN CONCRETE INSTITUTE.
2. CONCRETE PROPERTIES:

	f'c (PSI) 28 DAYS	MAX. AGGR.	ENTR. AIR (%)	MAX. W/C	EXP CAT
FOOTINGS AND PIERS:.....	4,500	3/4"	6±1.5	0.45	F2

3. f'c OF 4500PSI IS SPECIFIED TO MEET ACI 301 REQUIREMENTS FOR DURABILITY.
4. REINFORCING STEEL - ASTM A615 GRADE 60
5. IF CONCRETE SUPPORT BLOCKS ARE USED, THEIR STRENGTH SHALL BE EQUAL TO OR GREATER THAN THAT OF THE CONCRETE BEING PLACED.
6. THE FOLLOWING MATERIALS SHALL NOT EXCEED THE FOLLOWING PERCENT OF TOTAL CEMENTITIOUS MATERIAL BY WEIGHT:

   FLY ASH:.....15%
   SOLUBLE CHLORIDE:.....0.1%

7. CONCRETE COMPONENTS SHALL MEET THE FOLLOWING ASTM:

   PORTLAND CEMENT:.....C150, TYPE I/II
   FINE AND COARSE AGGREGATES:.....C33
   FLYASH:.....C618, CLASS F
   AIR ENTRAINING ADMIXTURES:.....C260
   OTHER CHEMICAL ADMIXTURES:.....C494, TYPE A-G
   WATER, CLEAN & NOT DETRIMENTAL TO CONCRETE:.....N/A

8. PROVIDE A 3/4" CHAMFER ON ALL EXPOSED CONCRETE CORNERS.
9. PROVIDE LAP SPLICES AT ALL CORNERS AND INTERSECTIONS. SAME SIZE AND SPACING AS HORIZONTAL REINFORCING. CORNER BARS MAY BE REQUIRED.
10. PROVIDE SUPPORTS AND SPACERS FOR ALL REINFORCING
11. CONCRETE COVER OVER REBAR SHALL BE AS FOLLOWS UNLESS OTHERWISE NOTED ON DRAWINGS:

    CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH - 3"
    CONCRETE EXPOSED TO EARTH OR WEATHER - 2"
    CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND -1 1/2"

12. CONCRETE SHALL NOT BE ALLOWED TO FREE FALL DURING PLACEMENT, USE TREMIE OR OTHER MEANS NECESSARY TO PREVENT SEGREGATION.
13. CONSOLIDATE ALL CONCRETE, INCLUDING SLABS, BY VIBRATING.
14. ALL CONCRETE SHOWN SHALL BE REINFORCED. PLANS, SECTIONS AND DETAILS SHOWN WITHOUT REINFORCEMENT ARE INTENDED TO SHOW DIMENSIONS AND DETAILS OF CONSTRUCTION ONLY. REINFORCEMENT OF THESE SECTIONS SHALL BE PROVIDED IN ACCORDANCE WITH THE DETAILS SHOWING REINFORCEMENT.
15. ALL REINFORCING SHALL BE DETAILED, FABRICATED AND PLACED IN ACCORDANCE WITH CRSI "MANUAL OF STANDARD PRACTICE".
16. SUBMIT DESCRIPTION OF PLANNED PROTECTIVE MEASURES FOR HOT OR COLD WEATHER CONCRETING. HOT AND COLD WEATHER CONCRETING SHALL BE DONE IN ACCORDANCE WITH ACI 305R AND 306.1 RESPECTIVELY U.N.O.

FASTENING AND ANCHORAGE

A. GENERAL
1. COORDINATE THE FASTENING AND ANCHORAGE WITH THE CONCRETE AND STEEL COMPONENTS SHOWN ON THE DRAWINGS.

B. SUBMITTALS
1. PRODUCT SPECIFICATIONS WITH RECOMMENDED DESIGN VALUES AND PHYSICAL CHARACTERISTICS FOR ADHESIVE AND MECHANICAL ANCHORS.

C. PRODUCTS
1. CAST-IN-PLACE ANCHORS:
   a. ANCHOR BOLTS: ASTM F1554, GRADE 36
   b. NUTS: ASTM 1563 HEAVY HEX (GRADE DH WHEN GALVANIZED)
   c. WASHERS: ASTM F436

D. EXECUTION
1. CAST-IN-PLACE ANCHORS: USE TEMPLATES TO LOCATE ANCHORS ACCURATELY AND SECURELY IN FORMWORK.
2. INSTALL ANCHORS PER THE MANUFACTURER'S INSTRUCTIONS.
3. INSTALL ANCHORS IN ACCORDANCE WITH SPACING AND EDGE CLEARANCES INDICATED ON THE DRAWINGS.
4. REINFORCING BARS IN THE CONCRETE STRUCTURE MAY CONFLICT WITH SPECIFIC ANCHOR LOCATIONS. ADJUST THE REINFORCING BARS TO AVOID ANCHOR RODS.

COLORADO LICENSED
JOHN G. TRULLINGER
51916
PROFESSIONAL ENGINEER
January 22, 2021
Exp Date 10/31/21

ISSUED FOR CONSTRUCTION

NO.	BY	CHK.	APP.	DATE	REVISION DESCRIPTION
0	KAL2	KAK	JGT	01/22/21	ISSUED FOR CONSTRUCTION

CLIENT
BID
CONSTRUCTION 01/22/21
RELEASED TO/FOR
A B C 0 1 2 3
DATE RELEASED

BARR
Project Office:
BARR ENGINEERING CO.
170 SOUTH MAIN STREET
SUITE 500
SALT LAKE CITY, UT 84101
Ph: 1-800-632-2277
Fax: (952) 832-2601
www.barr.com
Corporate Headquarters:
Minneapolis, Minnesota
Ph: 1-800-632-2277

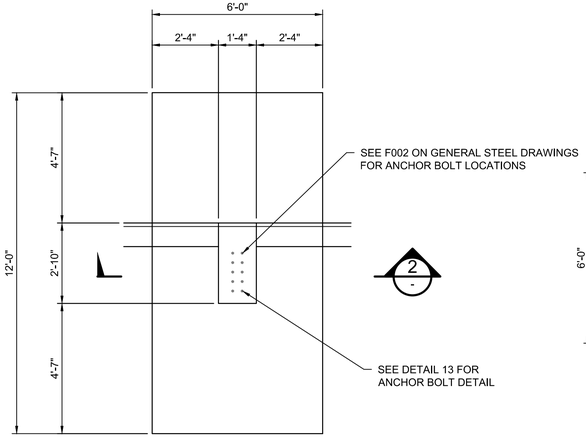
Scale	AS SHOWN
Date	01/14/21
Drawn	KAL2
Checked	KAK
Designed	KAK
Approved	JGT

OURAY SILVER MINE, INC
OURAY, COLORADO

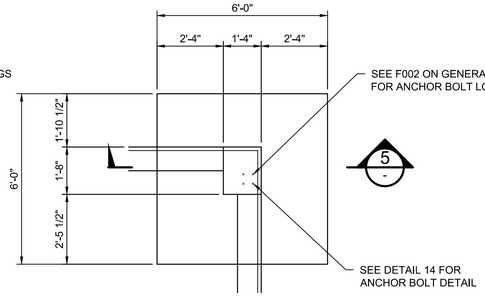
WAREHOUSE BUILDING
REVENUE VIRGINIUS MINE

WAREHOUSE BUILDING FOUNDATION
PLAN AND NOTES

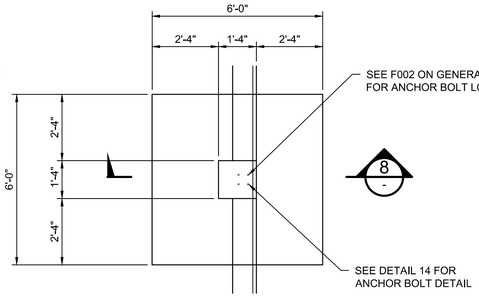
BARR PROJECT No. 06/46-1001.04
CLIENT PROJECT No.
DWG. No. 965-FF-01
REV. No. 0



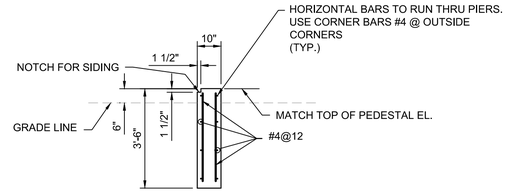
1 DETAIL: 965-FF-01 3/8"=1'-0" (SIMILAR 4 PLC'S)



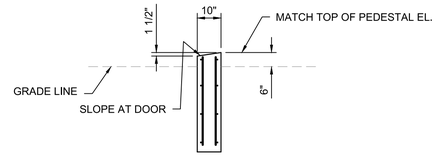
4 DETAIL: 3/8"=1'-0" (SIMILAR 4 PLC'S)



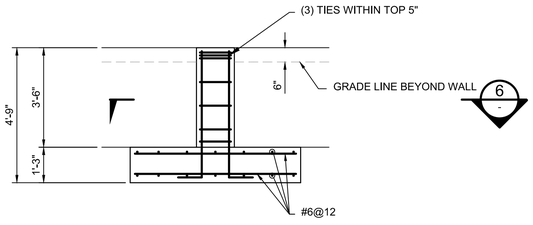
7 DETAIL: 965-FF-01 3/8"=1'-0" (SIMILAR 4 PLC'S)



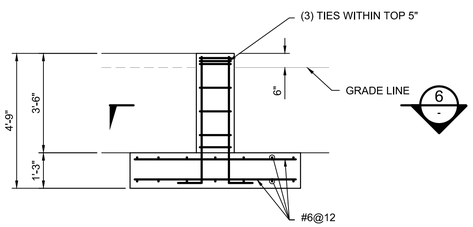
10 SECTION: 965-FF-01 3/8"=1'-0"



11 SECTION: 965-FF-01 3/8"=1'-0"



2 SECTION: 3/8"=1'-0"



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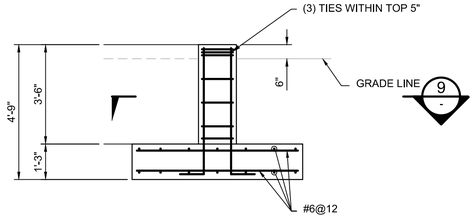

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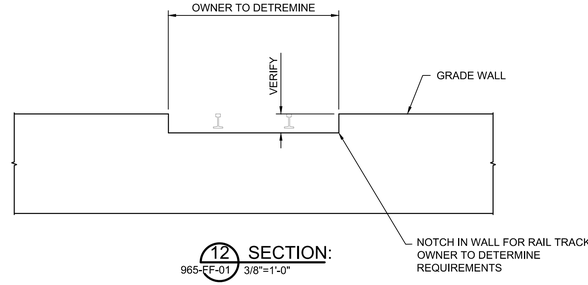
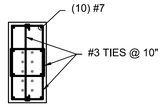
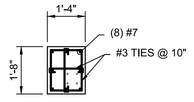


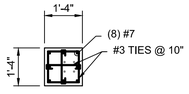
12 SECTION: 965-FF-01 3/8"=1'-0"



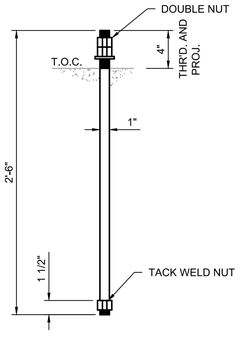
3 SECTION: 3/8"=1'-0"



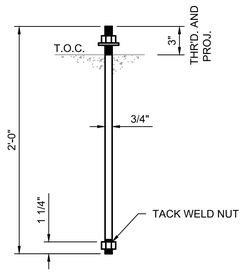
6 SECTION: 3/8"=1'-0"



9 SECTION: 3/8"=1'-0"



13 DETAIL: 1 1/2"=1'-0"



14 DETAIL: 1 1/2"=1'-0"

ISSUED FOR CONSTRUCTION

NO.	BY	CHK.	APP.	DATE	REVISION DESCRIPTION
0	KAL2	KAK	JGT	01/22/21	ISSUED FOR CONSTRUCTION

RELEASED TO/FOR

	A	B	C	0	1	2	3
CLIENT							
BID							
CONSTRUCTION				01/22/21			

DATE RELEASED

BARR
Corporate Headquarters: Minneapolis, Minnesota Ph: 1-800-632-2277

Project Office:
BARR ENGINEERING CO.
170 SOUTH MAIN STREET
SUITE 500
SALT LAKE CITY, UT 84101
Ph: 1-800-632-2277
Fax: (952) 832-2601
www.barr.com

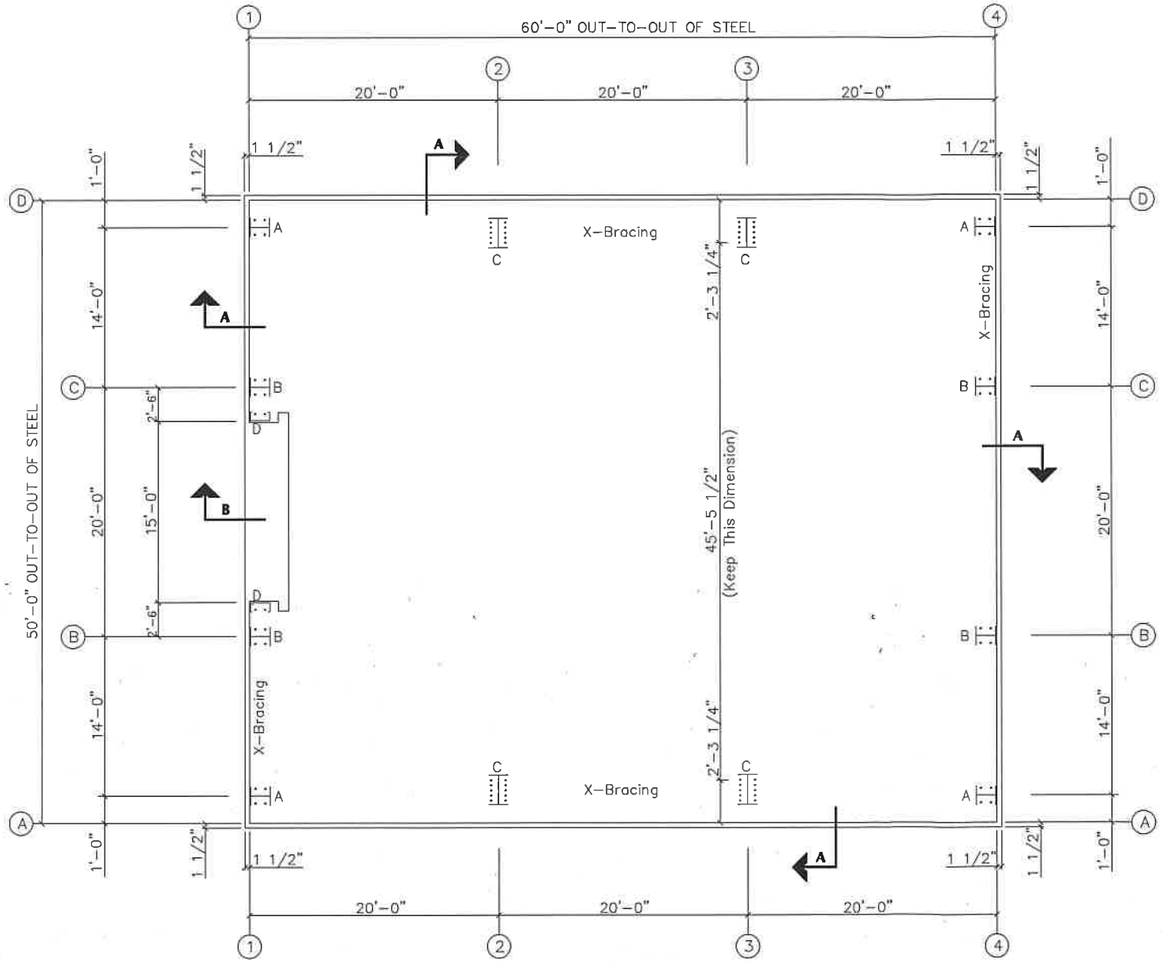
Scale	AS SHOWN
Date	01/14/21
Drawn	KAL2
Checked	KAK
Designed	KAK
Approved	JGT

OURAY SILVER MINE, INC
OURAY, COLORADO

WAREHOUSE BUILDING
REVENUE VIRGINIUS MINE

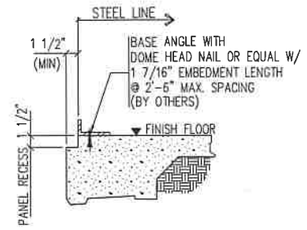
WAREHOUSE BUILDING FOUNDATION SECTIONS AND DETAILS

BARR PROJECT No. 06/46-1001.04
CLIENT PROJECT No.
DWG. No. 965-FF-02
REV. No. 0



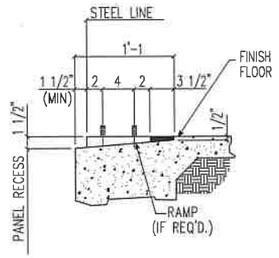
COLUMN LAYOUT PLAN

NOTE: All Base Plates @ 100'-0" (U.N.)



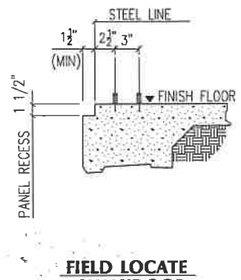
IMPORTANT NOTES:
NOTCH AREA INDICATES 1 1/2" INCH BY 1 1/2" INCH RECESS FOR METAL WALL SHEETS. THE SHEETS SHOULD NOT TOUCH THE BOTTOM OF THE NOTCH, WHICH WOULD VOID THE WARRANTY!

A SECTION



B SECTION

FOR CONSTRUCTION

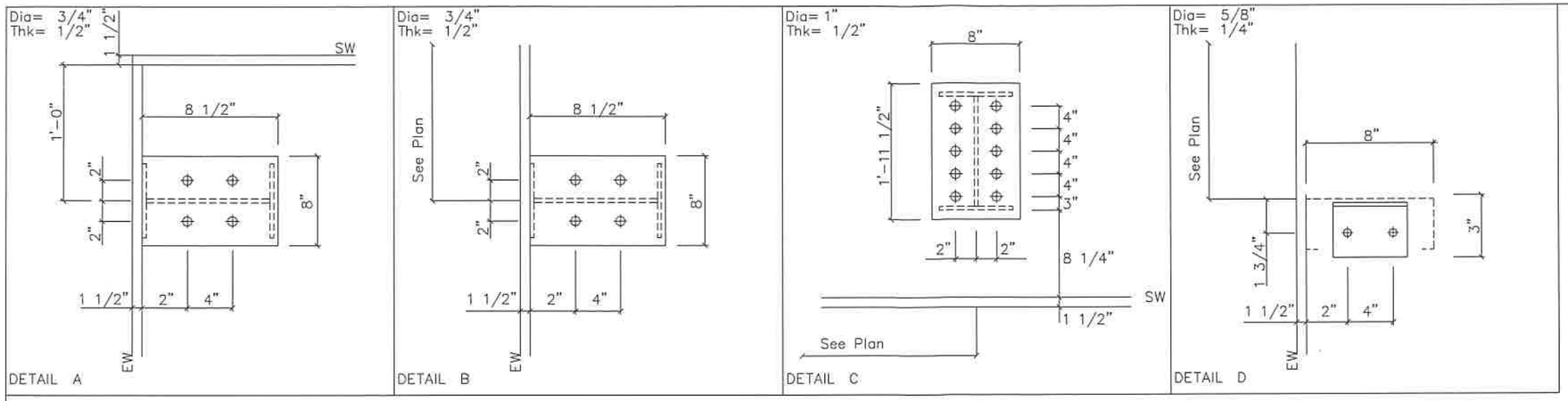


FIELD LOCATE WALKDOOR

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
0	CONSTRUCTION/PERMIT	11/05/20	JAP	AGP	BKD

GENERAL STEEL CORPORATION

DESCRIPTION	COLUMN LAYOUT PLAN		
CUSTOMER	AMMC Industries		
END USER	AMMC Industries		
END USE	Shop	BUILDING	A
STREET	19911 Hwy. 550		
CITY ST ZIP	Montrose, CO 81403		
SALES REP. 68726	JOB NO. 148759	SCALE N.T.S.	DWG NO. F001 REV. 0



FOR CONSTRUCTION

SEALING OF THIS DRAWING DOES NOT IMPLY OR CONSTITUTE THAT GENERAL STEEL ENGINEER IS THE ENGINEER OF RECORD OR THE DESIGN PROFESSIONAL FOR THIS PROJECT. ONLY THE DESIGN OF THE METAL BUILDING SYSTEM AS FURNISHED BY G.S.C. IS INCLUDED. FOUNDATION ANALYSIS, ELECTRICAL, AND MECHANICAL SYSTEMS, AND/OR OTHER PARTS SUPPLIED BY ANYONE OTHER THAN G.S.C. ARE SPECIFICALLY EXCLUDED. NO INSPECTION OR SUPERVISION IS IMPLIED.

GENERAL NOTES:

1. THE ANCHOR BOLT DETAILS SHOWN ON THIS DRAWING LOCATE THE ANCHOR BOLTS IN REFERENCE TO BOTH THE BUILDING STEEL LINE AND THE OUTSIDE OF RIGID'S SUGGESTED PANEL RECESS OF 1-1/2".
2. THE ANCHOR BOLT SETTING PLAN LOCATES ANCHOR BOLTS IN REFERENCE TO THE OUTSIDE OF THE PANEL RECESS SHOWN. IF THE ACTUAL PANEL RECESS IS DIFFERENT FROM WHAT IS SHOWN ON THE ANCHOR BOLT SETTING PLAN, THEN ALL REFERENCE DIMENSIONS FROM THE OUTSIDE OF THE PANEL RECESS MUST BE DETERMINED BY THE CUSTOMER.
3. BOTTOM OF ALL BASE PLATES ARE AT THE SAME ELEVATION. (UNLESS NOTED)

NOTE:

ONLY ANCHOR BOLTS SETTING PLAN ISSUED & STAMPED "FOR CONSTRUCTION" SHALL BE USED IN SETTING ANCHOR BOLTS. 'RIGID GLOBAL BUILDINGS' SHALL NOT BE RESPONSIBLE FOR ERROR OR DISCREPANCY IF THE DRAWING USED IS NOT VALID FOR CONSTRUCTION.

QTY.	SYMBOL	DIA.	PROJ.
0	+	1/2"	1"
4	⊕	5/8"	2"
32	⊕	3/4"	2 1/2"
0	⊕	7/8"	2 3/4"
40	⊕	1"	3"
0	⊕	1 1/8"	3 1/2"
0	⊕	1 1/2"	3 1/2"

ANCHOR BOLT DETAIL

ANCHOR BOLT PROJECTION "PROJ." IS MEASURED FROM BOTTOM OF BASE PLATE

LENGHT OF "PROJ." SHOWN IS FOR ONE NUT + ONE WASHER

DETAIL OF ANCHOR BOLT AS PER THE SUPPLIER

NUTS & WASHERS BY SUPPLIER

ANCHOR BOLTS NOT BY RIGID GLOBAL BUILDINGS

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
0	CONSTRUCTION/PERMIT	11/05/20	JAP	AGP	BKD

GENERAL STEEL CORPORATION

DESCRIPTION	ANCHOR BOLT DETAILS		
CUSTOMER	AMMC Industries		
END USER	AMMC Industries		
END USE	Shop	BUILDING	A
STREET	19911 Hwy. 550		
CITY ST ZIP	Montrose, CO 81403		
PROJ# 68726	JOB NO. 148759	SCALE N.T.S.	DWG NO. F002 REV 0

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	Dead Horiz	Dead Vert	Live Horiz	Live Vert	Snow Horiz	Snow Vert	Wind_Left1 Horiz	Wind_Left1 Vert	Wind_Right1 Horiz	Wind_Right1 Vert	Wind_Left2 Horiz	Wind_Left2 Vert
2*	D	1.5	3.6	5.0	10.0	30.4	60.6	−6.0	−8.9	0.1	−5.4	−5.9	−5.4
2*	A	−1.5	3.6	−5.0	10.0	−30.4	60.5	−0.1	−5.4	6.0	−8.9	−0.3	−1.8

Frame Line	Column Line	Wind_Right2 Horiz	Wind_Right2 Vert	Wind_Long1 Horiz	Wind_Long1 Vert	Wind_Long2 Horiz	Wind_Long2 Vert	Seismic_Left Horiz	Seismic_Left Vert	Seismic_Right Horiz	Seismic_Right Vert	Seismic_Long Horiz	Seismic_Long Vert
2*	D	0.3	−1.8	−0.9	−10.3	−1.5	−9.1	−1.7	−1.1	1.7	1.1	0.0	−4.0
2*	A	5.9	−5.4	1.5	−9.1	0.9	−10.3	−1.7	1.1	1.7	−1.1	0.0	−4.0

Frame Line	Column Line	MIN_SNOW Horiz	MIN_SNOW Vert	F1UNB_SL_L Horiz	F1UNB_SL_L Vert	F1UNB_SL_R Horiz	F1UNB_SL_R Vert
2*	D	5.0	10.0	24.7	61.6	24.5	32.7
2*	A	−5.0	10.0	−24.5	32.7	−24.7	61.6

2* Frame lines: 2 3

RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k) Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin	Bolt(in) Qty	Bolt(in) Dia	Base_Plate(in) Width	Base_Plate(in) Length	Base_Plate(in) Thick	Grout (in)
2*	D	1	31.9	64.1	2	−2.7	−3.2	10	1.000	8.000	23.50	0.500	0.0
		6	26.1	65.2	4	0.3	−4.1						
2*	A	3	2.7	−3.2	1	−31.9	64.1	10	1.000	8.000	23.50	0.500	0.0
		7	−26.1	65.2	5	−0.3	−4.1						

2* Frame lines: 2 3

ENDWALL COLUMN: BASIC COLUMN REACTIONS (k)

Frm Line	Col Line	Dead Vert	Live Vert	Snow Vert	Wind_Left1 Horz	Wind_Left1 Vert	Wind_Right1 Horz	Wind_Right1 Vert	Wind_Left2 Horz	Wind_Left2 Vert	Wind_Right2 Horz	Wind_Right2 Vert	Wind Press Horz	Wind Suct Horz
1	D	0.5	1.1	6.8	0.0	−1.2	0.0	−1.1	0.0	−0.7	0.0	−0.5	−1.0	1.2
1	C	1.3	4.0	24.0	0.0	−4.4	0.0	−2.6	0.0	−3.2	0.0	−1.4	−2.6	2.9
1	B	1.3	4.0	24.0	1.7	−5.1	0.0	−2.2	1.7	−3.8	0.0	−1.0	−2.6	2.9
1	A	0.5	1.1	6.8	0.0	1.4	1.7	−3.4	0.0	2.0	1.7	−2.8	−1.0	1.2

Frm Line	Col Line	Wind_Long1 Horz	Wind_Long1 Vert	Wind_Long2 Horz	Wind_Long2 Vert	Seis_Left Horz	Seis_Left Vert	Seis_Right Horz	Seis_Right Vert	MIN_SNOW Horz	MIN_SNOW Vert	E1UNB_SL_L Horz	E1UNB_SL_L Vert	E1UNB_SL_R Horz	E1UNB_SL_R Vert
1	D	0.0	−1.4	0.0	−0.7	0.0	0.1	0.0	−0.1	0.0	1.1	0.0	7.9	0.0	1.5
1	C	0.0	−4.2	0.0	−2.8	0.0	−0.1	0.0	0.1	0.0	4.0	0.0	28.9	0.0	9.7
1	B	0.0	−2.3	0.4	−4.8	1.9	−2.6	0.0	2.3	0.0	4.0	0.0	9.8	0.1	28.7
1	A	0.4	−1.3	0.0	−0.8	0.0	2.6	1.9	−2.3	0.0	1.1	0.1	1.4	0.0	8.0

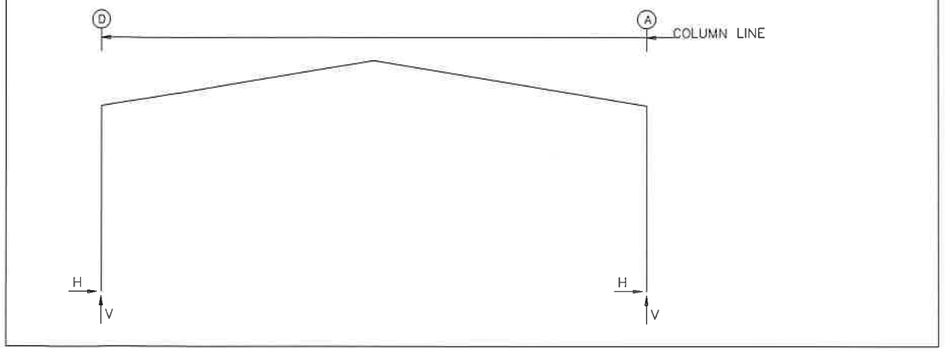
Frm Line	Col Line	Dead Vert	Live Vert	Snow Vert	Wind_Left1 Horz	Wind_Left1 Vert	Wind_Right1 Horz	Wind_Right1 Vert	Wind_Left2 Horz	Wind_Left2 Vert	Wind_Right2 Horz	Wind_Right2 Vert	Wind Press Horz	Wind Suct Horz
4	A	0.5	1.1	6.8	0.0	−1.2	0.0	−1.1	0.0	−0.7	0.0	−0.5	−1.0	1.2
4	B	1.3	4.0	24.0	0.0	−4.4	0.0	−2.6	0.0	−3.2	0.0	−1.4	−2.6	2.9
4	C	1.3	4.0	24.0	1.7	−5.1	0.0	−2.2	1.7	−3.8	0.0	−1.0	−2.6	2.9
4	D	0.5	1.1	6.8	0.0	1.4	1.7	−3.4	0.0	2.0	1.7	−2.8	−1.0	1.2

Frm Line	Col Line	Wind_Long1 Horz	Wind_Long1 Vert	Wind_Long2 Horz	Wind_Long2 Vert	Seis_Left Horz	Seis_Left Vert	Seis_Right Horz	Seis_Right Vert	MIN_SNOW Horz	MIN_SNOW Vert	E2UNB_SL_L Horz	E2UNB_SL_L Vert	E2UNB_SL_R Horz	E2UNB_SL_R Vert
4	A	0.0	−1.4	0.0	−0.7	0.0	0.1	0.0	−0.1	0.0	1.1	0.0	7.9	0.0	1.5
4	B	0.0	−4.2	0.0	−2.8	0.0	−0.1	0.0	0.1	0.0	4.0	0.0	28.9	0.0	9.7
4	C	0.0	−2.3	0.4	−4.8	1.9	−2.6	0.0	2.3	0.0	4.0	0.0	9.8	0.1	28.7
4	D	0.4	−1.3	0.0	−0.8	0.0	2.6	1.9	−2.3	0.0	1.1	0.1	1.4	0.0	8.0

ENDWALL COLUMN: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k) Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin	Bolt(in) Qty	Bolt(in) Dia	Base_Plate(in) Width	Base_Plate(in) Length	Base_Plate(in) Thick	Grout (in)
1	D	8	0.7	−0.6	9	−0.6	−0.6	4	0.750	8.000	8.500	0.500	0.0
		10	0.0	8.3	8	0.7	−0.6						
1	C	11	1.7	−1.9	9	−1.6	−1.8	4	0.750	8.000	8.500	0.500	0.0
		10	0.0	30.1	11	1.7	−1.9						
1	B	11	1.7	−2.3	12	−1.6	−2.1	4	0.750	8.000	8.500	0.500	0.0
		13	0.0	30.0	11	1.7	−2.3						
1	A	14	0.7	−1.8	9	−0.6	−0.5	4	0.750	8.000	8.500	0.500	0.0
		13	0.0	8.4	14	0.7	−1.8						
4	A	8	0.7	−0.6	9	−0.6	−0.6	4	0.750	8.000	8.500	0.500	0.0
		15	0.0	8.3	8	0.7	−0.6						
4	B	11	1.7	−1.9	9	−1.6	−1.8	4	0.750	8.000	8.500	0.500	0.0
		15	0.0	30.1	11	1.7	−1.9						
4	C	11	1.7	−2.3	12	−1.6	−2.1	4	0.750	8.000	8.500	0.500	0.0
		16	0.0	30.0	11	1.7	−2.3						
4	D	14	0.7	−1.8	9	−0.6	−0.5	4	0.750	8.000	8.500	0.500	0.0
		16	0.0	8.4	14	0.7	−1.8						

FRAME LINES: 2 3



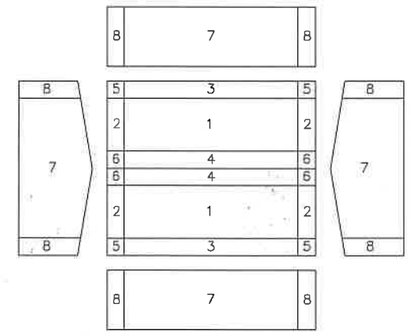
NOTES FOR REACTIONS

1. All loading conditions are examined and only maximum/minimum H or V and the corresponding H or V are reported.
2. Positive reactions are as shown in the sketch. Foundation loads are in opposite directions.
3. Bracing reactions are in the plane of the brace with the H pointing away from the braced bay. The vertical reaction is downward.
4. Building reactions are based on the following building data.

Width (ft)	: 50
Length (ft)	: 60
Eave Height (ft)	: 17 / 17
Roof Slope (rise/12)	: 2.0:12 / 2.0:12
Design Code	: IBC 18
Enclosure	: Closed
Dead Load (psf)	: 4.00
Collateral Load (psf)	: 0.00
Basic Design Wind Speed (mph)	: V (3 sec. gust) = 115.00 mph
Allowable Stress Wind Speed (mph)	: Vasd (3 sec. gust) = 89.08 mph
Wind Importance Factor	: 1.000
Wind Exposure	: C
Live Load (psf)	: 20.00
Frame Live Load (psf)	: 20.00
Ground Snow Load (psf)	: 173.00
Roof Snow Load (psf)	: 121.10
Snow Exposure	: 1.000
Snow Importance Factor	: 1.000
Thermal Factor	: 1.000
Seismic Importance Factor	: 1.000
Spectral Response Accel.	: Ss=0.330 : S1=0.075
Spectral Response Coeff.	: Sds=0.337 : Sd1=0.120
Seismic Coeff. (Fa*Ss)	: 0.506 : Fa=1.537
Seismic Design Category	: C

5. Loading conditions are:
   1 Dead+Collateral+Snow+Slide_Snow
   2 0.6Dead+0.6Wind_Left1
   3 0.6Dead+0.6Wind_Right1
   4 0.6Dead+0.6Wind_Long1L
   5 0.6Dead+0.6Wind_Long2L
   6 Dead+Collateral+F1UNB_SL_L
   7 Dead+Collateral+F1UNB_SL_R
   8 0.6Dead+0.6Wind_Suction+0.6Wind_Long1L
   9 0.6Dead+0.6Wind_Pressure+0.6Wind_Long1L
   10 Dead+Collateral+E1UNB_SL_L
   11 0.6Dead+0.6Wind_Left1+0.6Wind_Suction
   12 0.6Dead+0.6Wind_Pressure+0.6Wind_Long2L
   13 Dead+Collateral+E1UNB_SL_R
   14 0.6Dead+0.6Wind_Right1+0.6Wind_Suction
   15 Dead+Collateral+E2UNB_SL_L
   16 Dead+Collateral+E2UNB_SL_R

Zone	Width (ft)	Length (ft)	Components & Cladding (Factored) Pressure (psf) Member	Pressure (psf) Panel	Suction (psf) Member	Suction (psf) Panel
1			10.00	10.00	−10.00	−25.81
2		5.00	10.00	10.00	−24.67	−37.69
3	5.00		10.00	10.00	−10.00	−25.81
4	5.00		10.00	10.00	−24.67	−37.69
5	5.00	5.00	10.00	10.00	−24.67	−37.69
6	5.00	5.00	10.00	10.00	−27.94	−44.65
7			10.92	12.78	−12.00	−13.86
8	5.00		10.92	12.78	−13.32	−17.05



Design Calculation Wind

BUILDING BRACING REACTIONS

Wall Loc	Wall Line	Col Line	± Reactions(k) Wind Horz	Wind Vert	Seismic Horz	Seismic Vert	Panel_Shear (lb/ft) Wind	Panel_Shear (lb/ft) Seis
L_EW	1	B,A	1.7	2.1	1.9	2.3		
F_SW	A	2,3	3.5	2.7	5.3	4.0		
R_EW	4	C,D	1.7	2.1	1.9	2.3		
B_SW	D	3,2	3.5	2.7	5.3	4.0		

FOR CONSTRUCTION

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
0	CONSTRUCTION/PERMIT	11/05/20	JAP	MBS	BKD

GENERAL STEEL CORPORATION

DESCRIPTION	ANCHOR BOLT REACTIONS		
CUSTOMER	AMMC Industries		
END USER	AMMC Industries		
END USE	Shop	BUILDING	A
STREET	19911 Hwy. 550		
CITY ST ZIP	Montrose, CO 81403		
68726	148759	N.T.S.	F003 0