

Worster Dam – DAM I.D. # 030401

Dam Breach Analysis and Flood Inundation Mapping



TETRA TECH PROJECT No. 114-910349

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1.0 PROJECT OVERVIEW

This report outlines the approach used for performing dam breach analysis and flood inundation mapping for Worster Dam in northwestern Larimer County, Colorado. This analysis was performed in accordance with the Colorado State Engineer's Office (SEO) requirements for inundation mapping as defined in Rule 16 of the Rules and Regulations for Dam Safety and Construction, January 2007.

Worster Dam (DAM I.D. # 030401) is owned and operated by Divide Canal and Reservoir Company. The specific site of the dam is in Section 5, Township 11 North, Range 74 West (Latitude = 40.944391, Longitude = -105.714877). The dam is located on Sheep Creek, which is a tributary of the Cache La Poudre River. A vicinity map of the dam and the reservoir it creates, Eaton Reservoir is provided in Figure 1. Eaton Reservoir is fed through flows contributed from the upper reaches of Sheep Creek.

The State Engineer's Office (SEO) conducted routine inspections of the dam in 2013 and 2015. While the conditions observed during the inspection resulted in a recommended safe storage level of full storage, a request was made by the SEO to update the Emergency Action Plan (EAP). The current EAP for the Worster dam is over 20 years old as stated in a letter from Kallie Bauer (SEO, July 17, 2013). In response to the State Engineer's office request, Divide Canal and Reservoir Company (Divide) received grant money from the SEO to perform dam breach analysis and corresponding flood inundation mapping to update the EAP. The dam breach analysis and subsequent flood inundation mapping determines the extent of damages that may occur in the event of a dam breach, thus determining the hazard classification of the dam.

The dam is currently classified as a significant hazard dam under the State of Colorado's Dam Safety Regulations (Rules and Regulations for Dam Safety and Dam Construction, 2007). Per the State of Colorado Dam Safety Office, dam owners are required to determine the flood inundation area resulting from a dam failure as part of the Emergency Action Plan (EAP) for all significant hazard dams throughout the State. In January 2016, Divide contracted with Tetra Tech to perform dam breach analysis and map the flood inundation area to accompany the updated EAP. The development of the dam breach parameters, the modeling associated with the breach of the dam, routing of the dam breach flood wave, and mapping of the flood inundation area are described in the following sections.

2.0 DESCRIPTION OF WORSTER DAM

Based on information from the SEO, Worster Dam is comprised of a compacted earth fill embankment. The dam is classified as a large, significant hazard dam with a maximum embankment height of 72 feet and a crest length of 705 feet. The upstream and downstream slope of the dam face appear to be constructed to 1.5 feet and 2 feet horizontal, respectively, to 1-foot vertical (1.5H:1V & 2H:1V) side slopes. There is riprap on both the upstream and downstream faces of the dam. There is a shelf on the downstream face where piezometers are installed. The upper third of the downstream face appears steeper than the lower 2/3, but is stable. The dam crest is 12 feet wide and is at an elevation of 8,541 feet AMSL.

The dam was designed and constructed with a spillway and low flow outlet structure. The spillway is a fuse plug located along the western portion of the dam embankment, and conveys flow into Sheep Creek, which is tributary to the North Fork of the Cache La Poudre River. The spillway is 95 feet wide with 6 feet of freeboard and has a maximum capacity of 192 cubic feet per second (cfs).

Worster Dam was originally designed in the 1900's and completed in 1911. Improvements to the outlet structure were conducted in the early 1990's and the outlet pipe was slip-lined with solid wall HDPE pipe in 1996. The original design plans for the dam, as well as the plans for the outlet structure improvements are provided in Appendix A. Geometric parameters for Worster Dam are provided in Table 1.

Table 1: Worster Dam Parameters

| Parameter | Value |
|---------------------------|-----------------|
| Maximum Dam Height | 72 feet |
| Crest Length | 705 feet |
| Crest Width | 12 feet |
| Crest Elevation | 8,541 ft AMSL |
| Spillway Crest Elevation | 8,535 ft AMSL |
| Freeboard | 6 feet |
| Normal Storage Capacity | 3,750 acre-feet |
| Maximum Storage Capacity | 4,018 acre-feet |
| SEO Size Classification | Large |
| SEO Hazard Classification | Significant |

3.0 DAM BREACH ANALYSIS

For this analysis, Rule 5.9.1, in the State *Rules and Regulations* (SEO, 2007) was followed which requires an evaluation of the dam piping failure flood. To make reliable estimates of the piping failure flood, dam breach parameters need to be estimated from empirical methods. The State provides *Guidelines for Dam Breach Analysis* (SEO, 2010a) to help compute the respective dam breach parameters. Once dam breach parameters are calculated, they can be input into numerical models to develop the outflow dam failure hydrographs. The primary tools used for this analysis is the HEC-HMS and HEC-RAS models for determining the dam failure flood hydrograph (USACE, 2010a).

The SEO requires analysis of the sunny-day or piping failure flood for the hazard classification analysis. A sunny-day, fair-weather, or piping failure is a failure that occurs in the absence of a flood event when a reservoir is at maximum pool level. Typical piping failure modes include cracking due to settlement or seismic activity, or internal seepage erosion that creates an earthen “pipe” through the embankment. Flow through the initial opening increases the size of opening both horizontally and vertically until the section of the embankment over the top of it collapses, yielding a fully-formed breach. The outflow hydrographs from piping failures are usually not as large as outflow hydrographs from storm-related overtopping failures, but piping failures can occur with little or no warning and thus have a higher potential for loss of life.

3.1 DAM BREACH METHODS AND PARAMETERS

The first step in evaluating the outflow hydrograph is the prediction of reasonably accurate breach parameters necessary to make reliable estimates of the breach outflow and resulting downstream inundation. Selection of dam breach parameters was performed using the procedures set forth in Chapter 7 of the State Guidelines (SEO, 2010a). The State Guidelines recommend three empirical methods for predicting dam breach parameters: MacDonald & Langridge-Monopolis (MLM, 1984), Washington State (WA) (2007), and Froehlich (2008) methods. However, only the MLM-WA and Froehlich methods were used for this analysis based on the piping failure empirical method guidance chart adapted from Colorado Rules and Regulations for Dam Safety and Dam Construction, 2007, which is included in Appendix B. The State also provides spreadsheets for computing these parameters on their website. Excerpts from these spreadsheets are provided in Appendix B and each method is described below in more detail.

The overtopping breach was not evaluated as a failure mode for this analysis just the sunny-day breach or piping failure was evaluated for the hazard classification. The sunny-day breach parameters were based on a piping failure scenario in the embankment, and the initial water surface elevation is assumed to be at the spillway crest elevation of 8,535 feet AMSL.

3.2 MACDONALD AND LANGRIDGE-MONOPOLIS AND WASHINGTON STATE METHODS

MacDonald and Langridge-Monopolis (MLM, 1984) method computes a volume of embankment eroded during the breach formation, based on the product of the reservoir volume (V_w) and the maximum water depth (H_w). This product, termed the Breach Formation Factor (BFF), loosely represents the erosive potential of the water stored in the reservoir. The Washington State method (WA) (2007) took the MLM method and adjusted it based on whether the dam is made of cohesionless or cohesive material. The

State Guidelines indicate the Washington State method is more suited for large dams like Worster Dam, so it was used to calculate the volume of eroded material for this analysis.

The predictive equations for each method are presented below for the volume of earth removed during a breach (V_{er}), the time of breach formation (T_f), and average breach width (B_{avg}). The equations are shown in Table 2 below.

Table 2: MLM and WA Equations

| Breach Parameter | MLM (1984) | WA (2007) |
|------------------------------------|--|--|
| Volume Eroded (V_{er}) | $V_{er} = 3.264(BFF)^{0.77}$ (best fits all data) | $V_{er} = 3.75(BFF)^{0.77}$ (cohesionless) |
| | $V_{er} = 0.714(BFF)^{0.852}$ (rockfill) | $V_{er} = 2.5(BFF)^{0.77}$ (cohesive) |
| Breach Formation Factor (BFF) | $BFF = V_w \times H_w$ | $BFF = V_w \times H_w$ |
| Average Breach Width (B_{avg}) | $B_{avg} = V_{er}/(H_b \times W_{avg})$ | - |
| Breach Development Time (T_f) | $T_f = 0.016 (V_{er})^{0.364}$ | $T_f = 0.02 (V_{er})^{0.36}$ (cohesionless) |
| | | $T_f = 0.036 (V_{er})^{0.36}$ (cohesive) |

V_{er} is the volume of cohesionless soil material eroded (cubic yards); V_w is the volume of water released through the breach or the effective storage volume (acre-feet); H_w is the depth water behind dam (feet); H_b is the height of the breach (feet); T_f is the breach formation time (hours); B is the bottom width of the breach (feet); C is the crest width of the dam (feet); Z_b is the slope of the breach formation; and $Z_{u,d}$ is the upstream and downstream slopes, respectively (no units), expressed as X Horizontal : 1 Vertical (XH:1V). The upstream slope of the dam is 1.5H:1V, and the downstream slope is 2H:1V. The side slope of the breach (Z_b) for this earthen (non-cohesive) embankment is assumed to be 2H:1V based on Table 3 of the State Guidelines (SEO, 2010a). The input parameters for this method are listed in Table 3. C_p is the piping coefficient used to calculate peak discharge through the piping hole.

Table 3: Input and Breach Parameters for MLM-WA Methods

| Parameter | Unit | Value | Note |
|---------------|---------------|-------|---------------------------------|
| V_w | acre-ft | 3862 | At Spillway Crest |
| H_b | feet | 72 | Dam Height |
| H_w | feet | 66 | Above Base Level |
| S_a | acres | 137.5 | Reservoir Surface Area at H_w |
| Z_b | XH:1V | 2 | Breach Side Slope |
| Z_u | XH:1V | 1.5 | Slope of Upstream Dam Face |
| Z_d | XH:1V | 2 | Slope of Downstream Dam Face |
| C | feet | 12 | Dam Crest Width |
| C_p | dimensionless | 0.68 | Piping Coefficient |
| B_b | feet | 4.3 | Bottom Width of Breach |
| B_{avg} | feet | 148.3 | Average Breach Width |
| T_f | hours | 1.01 | Breach Formation Time |
| B_{avg}/H_b | -- | 2.06 | |
| ER/H_w | -- | 2.2 | ER = Erosion Rate |

3.3 FROEHLICH METHOD

The SEO recommends Froehlich's method for dams with reservoir volumes greater than 100 acre-ft. The Froehlich (2008) method is dependent only on the volume of the reservoir, height of the breach, and the assumed breach side slope. The empirical equations for breach width and time of failure (i.e., breach formation time), as presented in Table 2 of the State Guidelines (SEO, 2010a), are listed below.

$$B_{avg} = 9.5 k_o (V_r H_d)^{0.25}$$

$$T_f = 0.59 (V_r^{0.47} / H_d^{0.9})$$

In these equations, B_{avg} is average breach width (feet); T_f is the breach time (hours); k_o is a coefficient that is equal to 1.0 for piping breach and 1.3 for overtopping breach; V_r is the effective reservoir storage volume (acre-feet); and H_d is the height (feet) of water above the breach bottom. The average breach width is the average width between the bottom width and the width at the dam crest. The input parameters and breach parameters calculated using Froehlich's equations are shown in Table 4.

Table 4: Input and Breach Parameters for Froehlich's Method

| Parameter | Units | Piping Failure | Note |
|---------------|-----------|----------------|---|
| k_o | -- | 1.0 | Unitless Coefficient |
| H_w | feet | 66 | Above Base Level |
| V_r | acre-feet | 3862 | Effective Storage Volume |
| A_a | acres | 137.5 | Reservoir Surface Area at H_w |
| H_b | feet | 72 | Top of Dam Elevation = 8,541 Feet, Breach Invert = 8,469 Feet |
| Z_b | XH:1V | 0.7 | Breach Side Slope |
| B_b | feet | 87 | Bottom Width of Breach |
| B_{avg} | feet | 137.4 | $B_{avg} = 9.5 k_o (V_r H_d)^{0.25}$ |
| T_f | hours | 0.56 | $T_f = 0.59 (V_r^{0.47} / H_d^{0.9})$ |
| B_{avg}/H_b | -- | 1.91 | |
| ER/H_w | -- | 3.7 | ER = Erosion Rate |

The average breach width and time of formation were taken from Froehlich instead of parameters calculated using the MLM-WA methods as recommended by the SEO and because Froehlich method produces shorter breach formation time, which is more conservative.

As an additional check to the validity of the peak discharge dam breach estimates, the SEO recommends two checks. The first check is to determine if the average breach width divided by the height of the breach is less than 0.6. If (B_{avg}/H_b) is greater than 0.6, then full breach development is anticipated. For this analysis, the average breach width divided by the breach height is greater than 0.6 for both dam failure methods. The other check is to evaluate whether the erosion rate divided by the height of the water is in between 1.6 and 21 ($1.6 < [ER/H_w] < 21$). This is the case for both dam failure methods so the erosion rate is assumed reasonable, and the prediction of the peak discharges is also reasonable.

4.0 DAM BREACH MODELING

For this analysis, the breach of Worster Dam from Eaton Reservoir was simulated using the storage area method in HEC-RAS (USACE, 2010). HEC-RAS uses hydraulic equations to generate breach hydrographs, and the breach hydrograph is based on the reservoir stage-storage information and breach formation parameters as described in the following sections.

4.1 STAGE-STORAGE INFORMATION

A partial list of the stage-storage data for the reservoir is provided in Table 5 with a more detailed table located in Appendix C. This information was provided by the SEO. Per recommendations from the SEO, the starting piping elevation was set at the mid-height of the dam.

Table 5: Eaton Reservoir Stage-Storage Data

| Stage (ft) | Storage Volume (ac-ft) |
|------------|------------------------|
| 8,469 | 0 |
| 8,474 | 0 |
| 8,475 | 2 |
| 8,480 | 18 |
| 8,490 | 208 |
| 8,500 | 622 |
| 8,510 | 1,237 |
| 8,520 | 2,090 |
| 8,530 | 3,218 |
| 8,535 | 3,862 |
| 8,536 | 3,984 |
| 8,537 | 4,104 |
| 8,538 | 4,220 |
| 8,539 | 4,334 |
| 8,540 | 4,444 |
| 8,541 | 4,551 |

4.2 DAM BREACH PARAMETERS

Input parameters for the breach data in HEC-RAS are provided in Table 6. Selection of orifice coefficient (C_p), weir coefficient (C_w), and breach progression method were based on the SEO Guidelines for modeling a piping failure in HEC-RAS.

Table 6: Dam Breach Input Parameters for HEC-RAS

| Parameter | Value |
|-------------------------------|---------------|
| Center Station of Breach (ft) | 478.18 |
| Final Bottom Width (ft) | 87 |
| Left Side Slope (ft/ft) | 0.7 |
| Right Side Slope (ft/ft) | 0.7 |
| Breach Weir Coefficient | 3.08 |
| Breach Formation Time (hr) | 0.6 |
| Failure Mode | Piping |
| Piping Coefficient | 0.7 |
| Initial Piping Elevation (ft) | 8,469 ft AMSL |
| Starting WSEL (ft) | 8,535 ft AMSL |

5.0 HYDRAULIC ANALYSIS

The HEC-RAS model (USACE, 2010a) was used for developing flood inundation mapping downstream of the dam. HEC-RAS is a computer program that models the hydraulics of water flow through natural rivers and channels. HEC-RAS is a one-dimensional step backwater model so it does not directly model the hydraulic effect of cross section shape changes, bends, and other two- and three-dimensional aspects of flow. However, HEC-RAS (version 4.1) can model steady, gradually varying or unsteady flow, and both were performed for this study.

The outflow hydrograph for the piping failure was determined from the HEC-RAS analysis described above in Section 4 and used as input for the HEC-RAS unsteady flow model. Unsteady flow analysis was performed for the channel downstream of the dam. There are two cross sections upstream of the dam crest and downstream cross sections were cut starting approximately 163 feet downstream of the dam crest, ending at the inlet of Halligan Reservoir. These cross sections were cut using the USGS topography, which is a 10-meter Digital Elevation Map (DEM) with contours generated at 0.5-meter intervals using ArcGIS.

A total of four hundred and ninety-two (492) cross sections were obtained at an interval of approximately 300 feet. These cross sections were numbered beginning at the downstream end of the reach where the North Fork of the Cache La Poudre River enters Halligan Reservoir. The river stations are in meters and generated using the HEC-GeoRAS extension in ArcGIS. The cross sections stop approximately 160 feet upstream of the dam crest.

Cross-sections were subdivided into a main-channel reach, a left overbank, and a right overbank. The flow paths in the main channel and the overbanks differ because of bends in the valley and localized geomorphic features. Manning's n-value of 0.05 was observed for the main channel representing a sand bottom creek bed, and a value of 0.08 was noted for the overbank throughout the creek representing native vegetation that exists along the channel. These n-values were determined from the field reconnaissance. However, due to model instabilities the n-values were increased to 0.07 for the channel and 0.1 for the overbanks, which more closely represent flow conditions during a dam failure flood. Tetra Tech performed field reconnaissance of the flood inundation area downstream of the Worster Dam on April 23, 2016 and took photos of some of the structures, which are included in Appendix E. Due to presence of snow on the roads above certain elevation, only certain structures near the downstream areas were accessible.

The stream is conveyed beneath road crossings through culverts/bridges downstream of the dam. These culverts/bridges do not have the capacity to convey the breach flood, so the culverts and bridges were not modeled. It is assumed that the culverts or bridges will be washed out during the dam failure flood and will not attenuate flood water.

The unsteady flow analysis was performed as a mixed flow regime with an upstream boundary condition of constant lateral inflow of 192 cfs, which is equal to the maximum Worster Dam spillway capacity, and a downstream boundary condition was set at Halligan Reservoir existing spillway crest elevation of 6357.6 feet. The results from the HEC-RAS model are included in Appendix D.

6.0 FLOOD INUNDATION MAPPING

The flood hydrograph from the piping failure flood was routed downstream and the subsequent flood inundation limits were mapped using ArcGIS. The peak flow from this event is approximately 65,709 cfs. HEC-RAS model results were used to create the inundation maps of Sheep Creek and the North Fork of the Cache La Poudre River. The inundation maps are provided as Figure 2 through Figure 10. Flood hydrographs at critical locations/sections are provided at the end of Appendix D. The inundation maps were created by exporting the HEC-RAS model flood inundation limits into ArcGIS. The flood inundation limits for each flood were mapped over the USGS DEM base map and current aerial photography to provide a background image for reference. Also shown on these figures is the location of fifty-four (54) structures and critical sections (i.e., bridges, culverts, or narrow areas) that were determined based on aerial photography.

Worster Dam is currently classified as a “Significant Hazard Dam” by the SEO. Rule 4.2.14 states the “Hazard Classification of a Dam” is the placement of a dam into one of four categories based on the hazard potential derived from an evaluation of the probable incremental adverse consequences due to failure or improper operation of the dam. The hazard potential classification does not reflect the current condition of the dam with regard to safety, structural integrity, or flood routing capacity. The purpose of a hazard classification is to evaluate the potential consequences of the subject dam failure on persons and property below the dam and to determine the adverse impacts of a dam failure flood at critical downstream locations where hazards may exist (SEO, 2010b). Hazard classification does not consider the potential impacts of future downstream or upstream development or changes in land use. It is based on current downstream conditions.

The criteria for determining expected loss of human life as outlined in the Guidelines (SEO, 2010b) are as follows:

1. No loss of life is expected to occur if the depth of flow is two feet or less and the product of the flow depth and the flow velocity at the critical location is less than seven.
2. If the flow velocity cannot be calculated with confidence at the critical location, then the average cross-section flow velocity may be used in calculating the product of depth and velocity.
3. Judgment and sound reason must be used in determining the potential for loss of life at locations where the flow depth is less than two feet and the product of depth and velocity is greater than seven, or where the depth of flow is greater than two feet and the product of depth and velocity is less than seven. An example location may be the basement of a habitable structure.

As seen in Table 7 below, there is a potential for significant damage to 38 of the 54 structures because either the depth of flow (d) is more than two (2) feet or the product of the flow velocity and the flow depth ($V \times d$) is more than seven (7) at those structures due to the dam failure flood. However, at least 3 of the 38 structures are potentially inhabited all year around and most structures are access roads, storage buildings, barns, or buildings inhabited seasonally (i.e. they may be used as cabins during spring/summer seasons) based on field observation.

Table 7: Summary Data at Critical Sections/Structures

| Structure ID ¹ | Critical Section ID | Approx. Distance From Dam (mi) | Approx. Structure Base Floor Elev (ft) ² | Peak WSEL (ft) | Peak Flow (cfs) | Depth of Water above Base Elev, d (ft) | Ave Velocity, V (fps) | Velocity and Depth product (V x d) | Potentially Significant Damage? ³ | Potentially Inhabited Structure? ⁴ | Initial Flood Wave Arrival Time (hh:mm) | Peak Flood Wave Arrival Time (hh:mm) |
|---------------------------|---------------------|--------------------------------|---|----------------|-----------------|--|-----------------------|------------------------------------|--|---|---|--------------------------------------|
| 1 | 45242.44 | 0.04 | 8486.8 | 8495.66 | 65708.9 | 8.9 | 18.7 | 165.9 | YES | NO | 00:01 | 00:23 |
| 2 | 45204.38 | 0.06 | 8488.8 | 8494.3 | 65652.2 | 5.5 | 15.7 | 86.3 | YES | NO | 00:01 | 00:23 |
| 3 | 45169.73 | 0.09 | 8480.5 | 8491.05 | 65599.9 | 10.5 | 18.4 | 193.8 | YES | NO | 00:01 | 00:23 |
| 4 | 45169.73 | 0.09 | 8520.4 | 8491.05 | 65599.9 | -- | 18.4 | -- | NO | NO | 00:01 | 00:23 |
| 5 | 43803.54 | 0.93 | 8449.8 | 8446.91 | 63111.3 | -- | 10.1 | -- | NO | NO | 00:14 | 00:33 |
| 6 | 39669.85 | 3.50 | 8337.6 | 8359.91 | 52997.7 | 22.3 | 10.0 | 222.4 | YES | NO | 00:42 | 00:53 |
| 7 | 36070.57 | 5.74 | 8166.0 | 8180.94 | 51158.7 | 14.9 | 17.6 | 262.2 | YES | NO | 00:58 | 01:06 |
| 8 | 35980.86 | 5.79 | 8180.9 | 8177.99 | 50781.6 | -- | 9.9 | -- | NO | NO | 00:59 | 01:06 |
| 9 | 35980.86 | 5.79 | 8195.5 | 8177.99 | 50781.6 | -- | 9.9 | -- | NO | NO | 00:59 | 01:06 |
| 10 | 35901.62 | 5.84 | 8186.6 | 8175.01 | 50309.6 | -- | 11.7 | -- | NO | NO | 00:59 | 01:07 |
| 11 | 34924.11 | 6.45 | 8151.5 | 8126.73 | 49877.1 | -- | 12.1 | -- | NO | NO | 01:03 | 01:11 |
| 12 | 34924.11 | 6.45 | 8103.7 | 8126.73 | 49877.1 | 23.1 | 12.1 | 279.9 | YES | NO | 01:03 | 01:11 |
| 13 | 33815.75 | 7.14 | 8053.5 | 8047.36 | 49175.1 | -- | 13.7 | -- | NO | NO | 01:07 | 01:14 |
| 14 | 30011.71 | 9.50 | 7700.2 | 7708.04 | 46084.0 | 7.8 | 20.1 | 157.2 | YES | NO | 01:21 | 01:25 |
| 15 | 28705.96 | 10.32 | 7596.9 | 7608.33 | 45571.6 | 11.4 | 10.5 | 119.4 | YES | NO | 01:25 | 01:29 |
| 16 | 21080.7 | 15.05 | 7128.5 | 7130.52 | 34584.2 | 2.0 | 7.1 | 14.4 | YES | YES | 01:57 | 02:02 |
| 17 | 21004.7 | 15.10 | 7123.9 | 7129.42 | 34487.0 | 5.5 | 5.9 | 32.8 | YES | NO | 01:57 | 02:03 |
| 18 | 21004.7 | 15.10 | 7150.3 | 7129.42 | 34487.0 | -- | 5.9 | -- | NO | NO | 01:57 | 02:03 |
| 19 | 20955.1 | 15.13 | 7127.1 | 7128.27 | 34428.4 | 1.2 | 7.1 | 8.3 | YES | NO | 01:57 | 02:03 |
| 20 | 20955.1 | 15.13 | 7129.8 | 7128.27 | 34428.4 | -- | 7.1 | -- | NO | NO | 01:57 | 02:03 |
| 21 | 20955.1 | 15.13 | 7144.3 | 7128.27 | 34428.4 | -- | 7.1 | -- | NO | NO | 01:57 | 02:03 |
| 22 | 20955.1 | 15.13 | 7149.1 | 7128.27 | 34428.4 | -- | 7.1 | -- | NO | NO | 01:57 | 02:03 |
| 23 | 20600.91 | 15.35 | 7094.8 | 7112.72 | 34099.8 | 17.9 | 9.4 | 167.9 | YES | NO | 02:00 | 02:05 |
| 24 | 19772.65 | 15.87 | 7076.2 | 7083.98 | 29978.5 | 7.8 | 5.4 | 41.9 | YES | NO | 02:04 | 02:14 |
| 25 | 18882.93 | 16.42 | 7038.1 | 7044.34 | 29856.7 | 6.2 | 9.0 | 56.0 | YES | NO | 02:09 | 02:17 |
| 26 | 18882.93 | 16.42 | 7038.9 | 7044.34 | 29856.7 | 5.4 | 9.0 | 48.8 | YES | NO | 02:09 | 02:17 |
| 27 | 18882.93 | 16.42 | 7040.6 | 7044.34 | 29856.7 | 3.7 | 9.0 | 33.5 | YES | NO | 02:09 | 02:17 |
| 28 | 18882.93 | 16.42 | 7043.1 | 7044.34 | 29856.7 | 1.2 | 9.0 | 11.1 | YES | NO | 02:09 | 02:17 |
| 29 | 18882.93 | 16.42 | 7044.0 | 7044.34 | 29856.7 | 0.3 | 9.0 | 3.0 | NO | NO | 02:09 | 02:17 |
| 30 | 18882.93 | 16.42 | 7046.8 | 7044.34 | 29856.7 | -- | 9.0 | -- | NO | NO | 02:09 | 02:17 |
| 31 | 18843.7 | 16.44 | 7033.8 | 7042.8 | 29819.1 | 9.0 | 7.9 | 70.9 | YES | NO | 02:09 | 02:17 |
| 32 | 18776.4 | 16.49 | 7032.6 | 7040.42 | 29719.3 | 7.8 | 7.7 | 60.1 | YES | YES | 02:10 | 02:18 |
| 33 | 18776.4 | 16.49 | 7033.1 | 7040.42 | 29719.3 | 7.3 | 7.7 | 56.3 | YES | NO | 02:10 | 02:18 |
| 34 | 18776.4 | 16.49 | 7035.5 | 7040.42 | 29719.3 | 4.9 | 7.7 | 37.8 | YES | NO | 02:10 | 02:18 |
| 35 | 18776.4 | 16.49 | 7037.6 | 7040.42 | 29719.3 | 2.8 | 7.7 | 21.7 | YES | NO | 02:10 | 02:18 |
| 36 | 18776.4 | 16.49 | 7037.6 | 7040.42 | 29719.3 | 2.8 | 7.7 | 21.7 | YES | NO | 02:10 | 02:18 |
| 37 | 18776.4 | 16.49 | 7037.7 | 7040.42 | 29719.3 | 2.7 | 7.7 | 20.9 | YES | NO | 02:10 | 02:18 |
| 38 | 18776.4 | 16.49 | 7040.5 | 7040.42 | 29719.3 | -- | 7.7 | -- | NO | NO | 02:10 | 02:18 |
| 39 | 18701.43 | 16.53 | 7026.3 | 7038.28 | 29603.4 | 12.0 | 7.5 | 90.1 | YES | NO | 02:10 | 02:18 |
| 40 | 18701.43 | 16.53 | 7026.7 | 7038.28 | 29603.4 | 11.6 | 7.5 | 87.1 | YES | NO | 02:10 | 02:18 |

| Structure ID ¹ | Critical Section ID | Approx. Distance From Dam (mi) | Approx. Structure Base Floor Elev (ft) ² | Peak WSEL (ft) | Peak Flow (cfs) | Depth of Water above Base Elev, d (ft) | Ave Velocity, V (fps) | Velocity and Depth product (V x d) | Potentially Significant Damage? ³ | Potentially Inhabited Structure? ⁴ | Initial Flood Wave Arrival Time (hh:mm) | Peak Flood Wave Arrival Time (hh:mm) |
|---------------------------|---------------------|--------------------------------|---|----------------|-----------------|--|-----------------------|------------------------------------|--|---|---|--------------------------------------|
| 41 | 18701.43 | 16.53 | 7027.5 | 7038.28 | 29603.4 | 10.8 | 7.5 | 81.1 | YES | NO | 02:10 | 02:18 |
| 42 | 18701.43 | 16.53 | 7028.2 | 7038.28 | 29603.4 | 10.1 | 7.5 | 75.8 | YES | NO | 02:10 | 02:18 |
| 43 | 18701.43 | 16.53 | 7028.9 | 7038.28 | 29603.4 | 9.4 | 7.5 | 70.5 | YES | NO | 02:10 | 02:18 |
| 44 | 18625.6 | 16.58 | 7025.1 | 7036.62 | 29547.7 | 11.5 | 6.9 | 79.1 | YES | NO | 02:11 | 02:19 |
| 45 | 17627.52 | 17.20 | 6993.3 | 6999.3 | 29155.6 | 6.0 | 7.1 | 42.6 | YES | NO | 02:16 | 02:24 |
| 46 | 17627.52 | 17.20 | 6993.4 | 6999.3 | 29155.6 | 5.9 | 7.1 | 41.9 | YES | NO | 02:16 | 02:24 |
| 47 | 17627.52 | 17.20 | 6993.9 | 6999.3 | 29155.6 | 5.4 | 7.1 | 38.3 | YES | YES | 02:16 | 02:24 |
| 48 | 17071.63 | 17.54 | 6971.8 | 6988.67 | 26620.6 | 16.9 | 5.1 | 86.1 | YES | NO | 02:21 | 02:33 |
| 49 | 16937.91 | 17.63 | 6994.8 | 6987.16 | 26588.7 | -7.6 | 6.4 | N/A | NO | NO | 02:21 | 02:33 |
| 50 | 16697.82 | 17.78 | 6961.0 | 6975.48 | 26557.6 | 14.5 | 8.8 | 127.4 | YES | NO | 02:22 | 02:34 |
| 51 | 16642.46 | 17.81 | 6957.3 | 6974.18 | 26553.0 | 16.9 | 9.7 | 163.1 | YES | NO | 02:23 | 02:34 |
| 52 | 16564.27 | 17.86 | 6966.2 | 6970.81 | 26548.9 | 4.6 | 12.3 | 56.9 | YES | NO | 02:23 | 02:34 |
| 53 | 15176.83 | 18.72 | 6959.4 | 6918.4 | 26037.9 | -41.0 | 7.0 | N/A | NO | NO | 02:30 | 02:41 |
| 54 | 14994.68 | 18.83 | 6893.4 | 6911.12 | 25835.7 | 17.7 | 10.8 | 191.7 | YES | NO | 02:32 | 02:42 |

¹Yellow shaded structure IDs correspond to road crossings²Elevations are based on USGS DEM with 10 meter (~33 feet) accuracy³Structures at gray shaded cells have potential for significant damage per SEO criteria⁴Structures at gray shaded cells are potentially inhabited based on field observation.

7.0 CONCLUSIONS

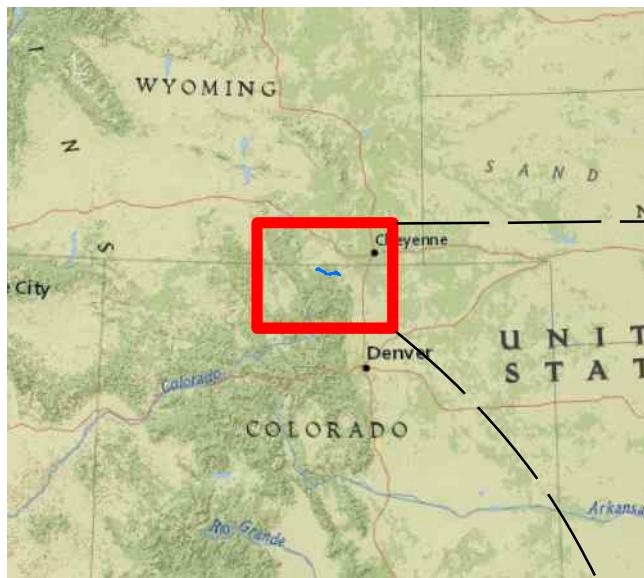
The Worster Dam is currently classified as a significant hazard dam by the SEO. Results from the dam breach analysis and inundation mapping show that there is a potential for significant damage to 38 structures as a result of possible piping breach of the Worster Dam. Based on a site visit on April 23, 2016, it is believed that there are at least three (3) human-occupied structures located within the Worster Dam flood inundation area. There are potentially more human-occupied structures upstream; however, Tetra Tech personnel were not able to evaluate anything upstream of section 21080.7 because of snow on the road that made the area inaccessible.

It is important to note that the flood inundation mapping in this report is based on using a 10-meter Digital Elevation Map (DEM); thus, there could be up to 30 feet discrepancy between the DEM elevations and the actual field survey elevations. It is recommended that this study be updated when a more accurate topography such as LiDAR or field survey tied to well-established benchmarks (i.e. USGS or Larimer County control points) is available to better represent actual field conditions. Also, finished floor elevations of all structures in the inundation area will need to be tied to this survey.

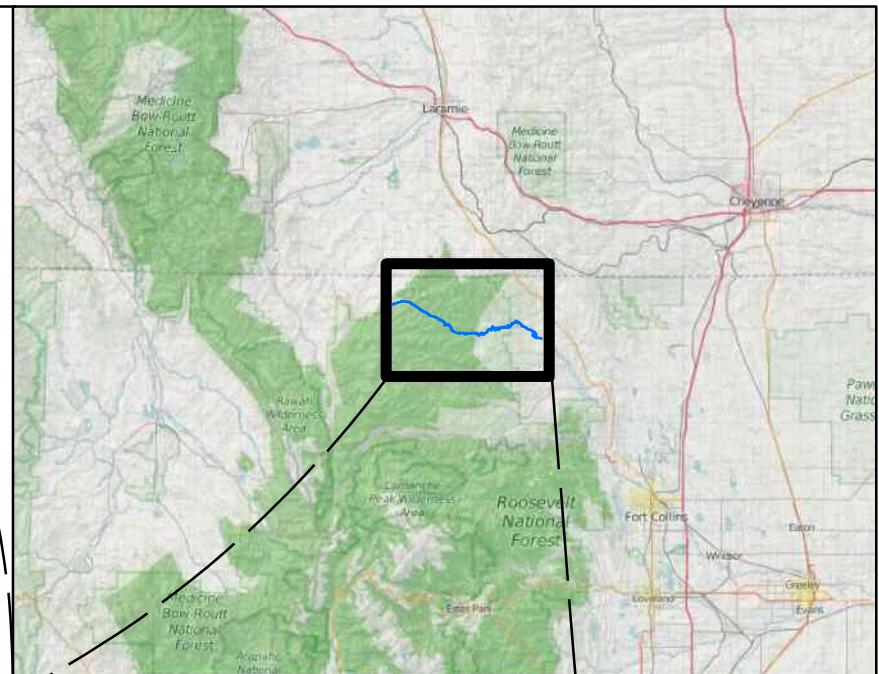
8.0 REFERENCES

- Froehlich, D.C., 2008. Embankment Dam Breach Parameters and Their Uncertainties. *Journal of Hydraulic Engineering*, 134 (12), 1708-1720.
- MacDonald TC, Langridge-Monopolis J. [MLM] 1984. Breaching Characteristics of Dam Failures. *Journal of Hydraulic Engineering*, 110(5), 567-586.
- SEO 2007. Rules and Regulations for Dam Safety and Dam Construction. Colorado Department of Natural Resources, Office of the State Engineer, Dam Safety Branch. 76p.
- SEO. 2010a. Guidelines for Dam Breach Analysis. Colorado Department of Natural Resources, Office of the State Engineer, Dam Safety Branch. 68p.
- SEO. 2010b. Guidelines for Hazard Classification. Colorado Department of Natural Resources, Office of the State Engineer, Dam Safety Branch.
- SEO, 2013. Letter from Kallie Bauer to Dale Leach, Subject: 2013 Engineer's Inspection Report – Worster Dam (Eaton Dam), DAMID 030401, Water Division 1, Water District 3. Dated July 17, 2013.
- United States Army Corps of Engineers [USACE] Hydraulic Engineering Center. 2010. Hydraulic Engineering Center – River Analysis System (HEC-RAS) [Software]. Version 4.1.

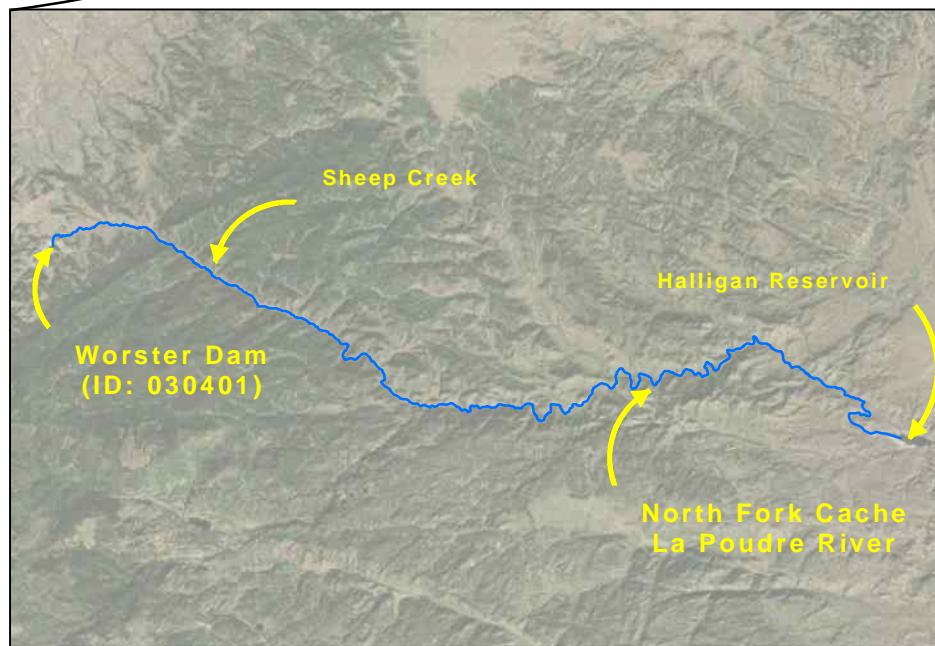
FIGURES



STATE MAP
NOT TO SCALE



REGIONAL LOCATION MAP
NOT TO SCALE



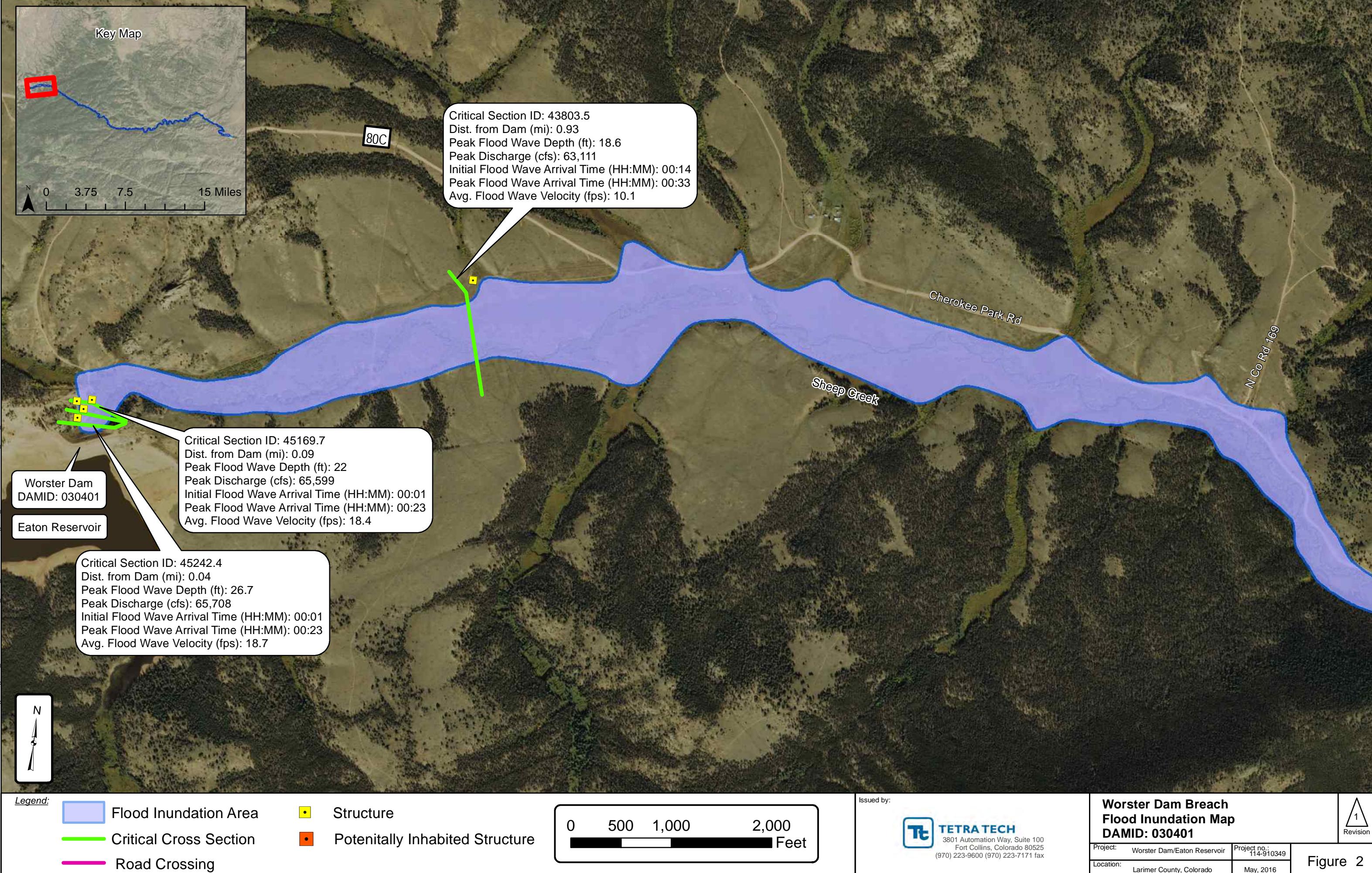
LOCATION MAP
NOT TO SCALE



| | | | | |
|------------------------|--|-----------------------------------|----------------------------|----------|
| Creek/River Centerline | Prepared for: Divide Canal and Reservoir Co. | WORSTER DAM SITE LOCATION MAP | | |
| | Prepared By: TETRA TECH 3801 Automation Way Suite 100 Fort Collins, Colorado 80525 (970) 223-9600 (970) 223-7171 fax | Project: WORSTER DAM/EATON RES | Project no.: 114-910349 | Figure 1 |

Location:
LARIMER COUNTY, CO

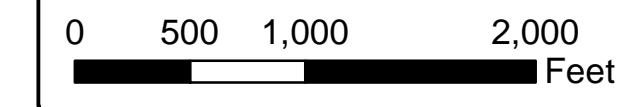
Date:
MAY 2016





Legend:

- Flood Inundation Area
- Structure
- Critical Cross Section
- Potentially Inhabited Structure
- Road Crossing



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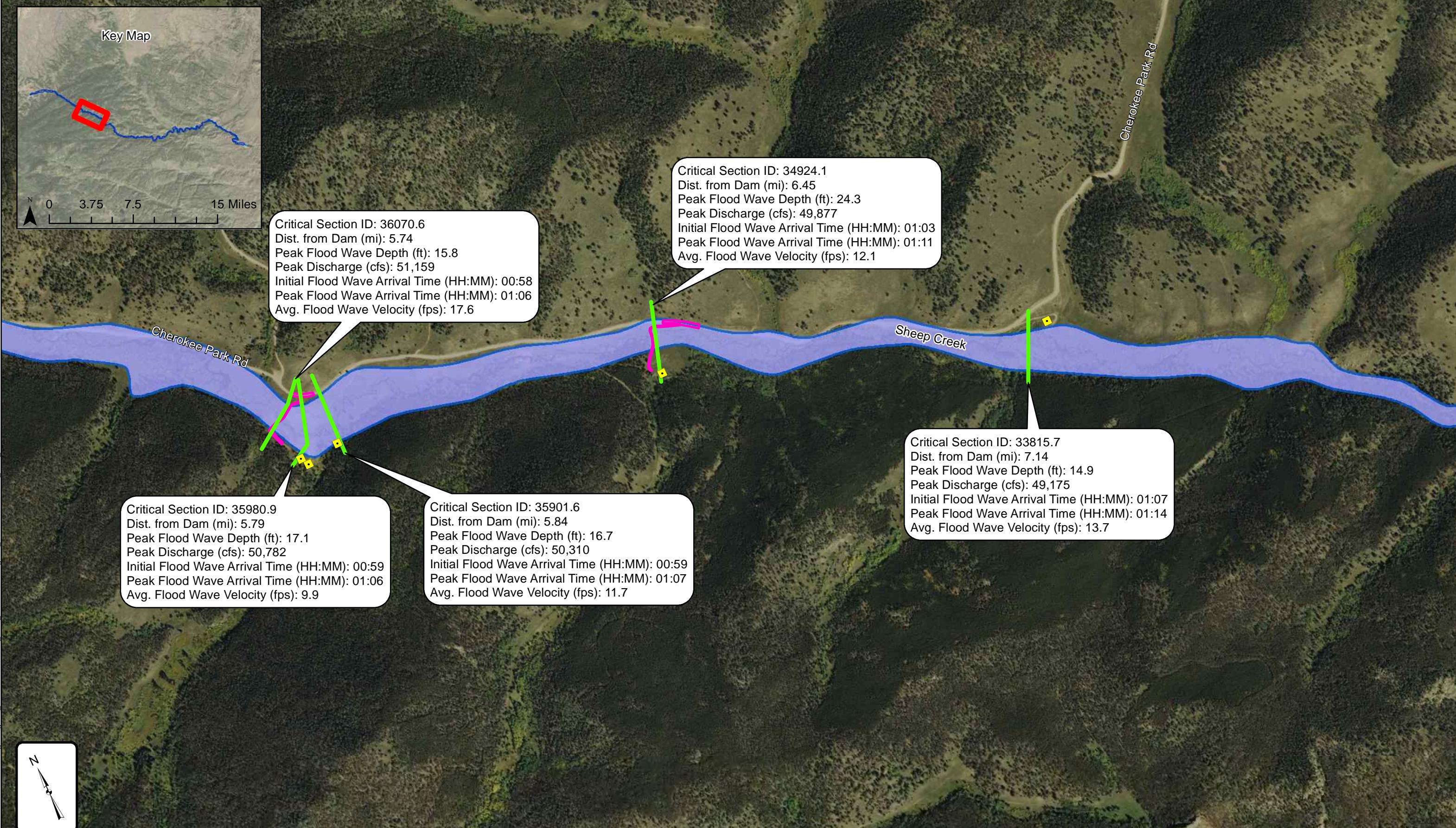
**Worster Dam Breach
Flood Inundation Map
DAMID: 030401**

Project: Worster Dam/Eaton Reservoir
Location: Larimer County, Colorado

Project no.: 114-910349
May, 2016

1
Revision

Figure 3

Legend:

- Flood Inundation Area
- Structure
- Potentially Inhabited Structure
- Critical Cross Section
- Road Crossing

0 500 1,000 2,000
Feet

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Fort Collins, Colorado 80525
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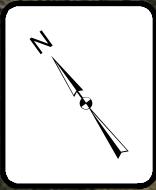
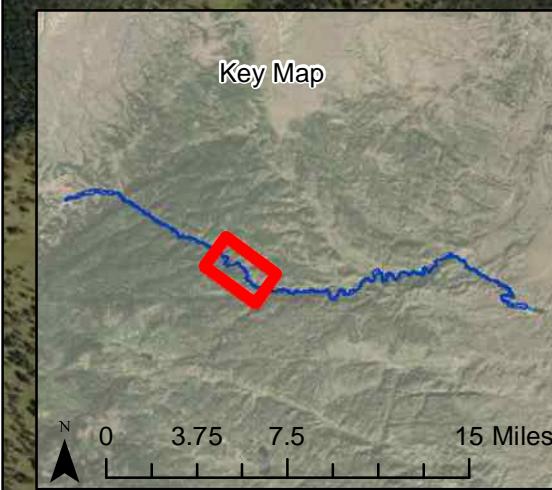
**Worster Dam Breach
Flood Inundation Map
DAMID: 030401**

Project: Worster Dam/Eaton Reservoir
Location: Larimer County, Colorado

Project no.: 114-910349
May, 2016

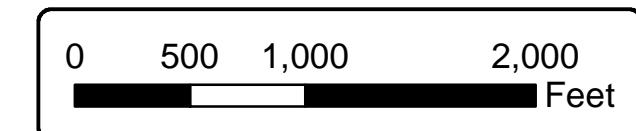
1
Revision

Figure 4

Legend:

- Flood Inundation Area
- Critical Cross Section
- Road Crossing

- Structure
- Potentially Inhabited Structure



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Fort Collins, Colorado 80525
(970) 223-9600 (970) 223-7171 fax

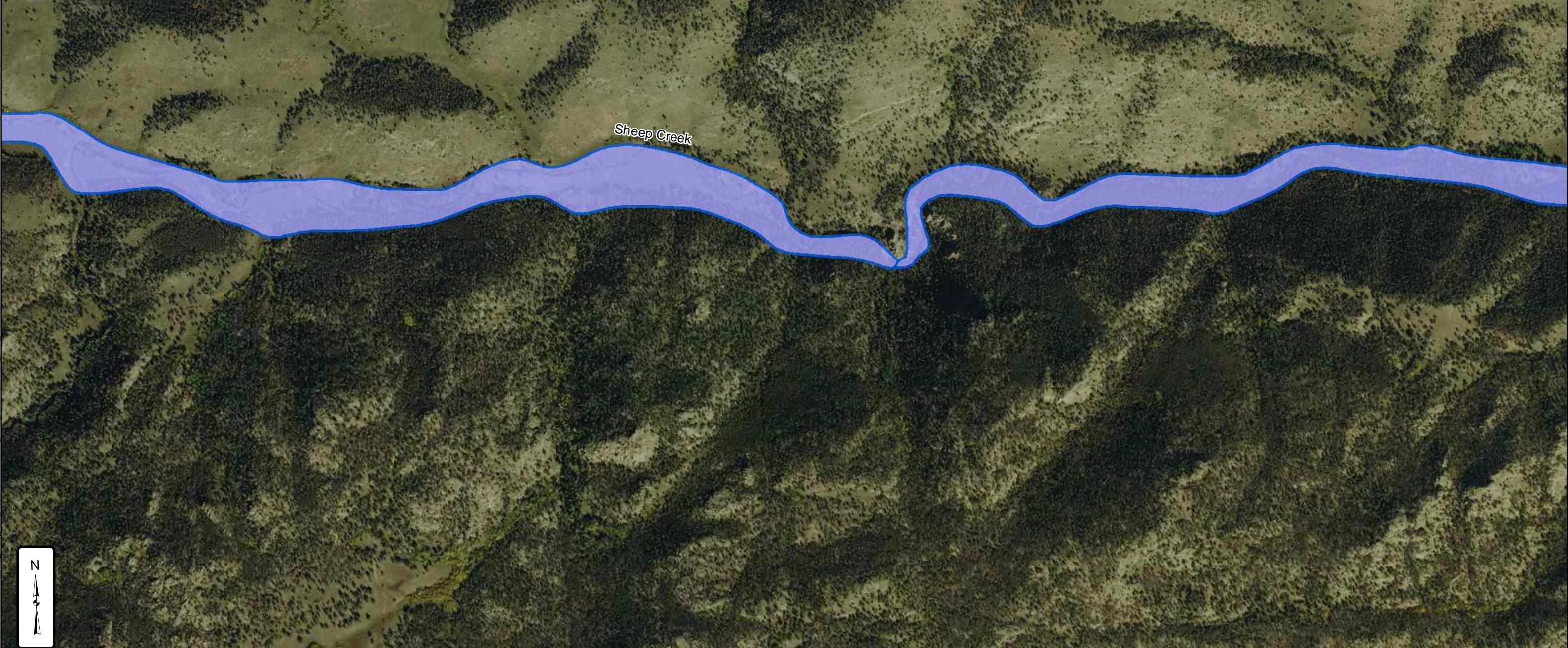
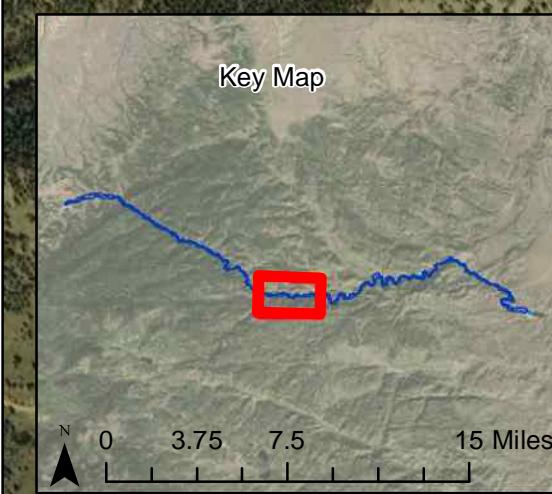
**Worster Dam Breach
Flood Inundation Map
DAMID: 030401**

Project: Worster Dam/Eaton Reservoir
Location: Larimer County, Colorado

Project no.: 114-910349
May, 2016

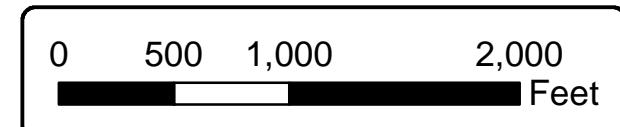
1
Revision

Figure 5



Legend:

- Flood Inundation Area
- Structure
- Critical Cross Section
- Potentially Inhabited Structure
- Road Crossing



Issued by:



Worster Dam Breach
Flood Inundation Map
DAMID: 030401

1

Revision

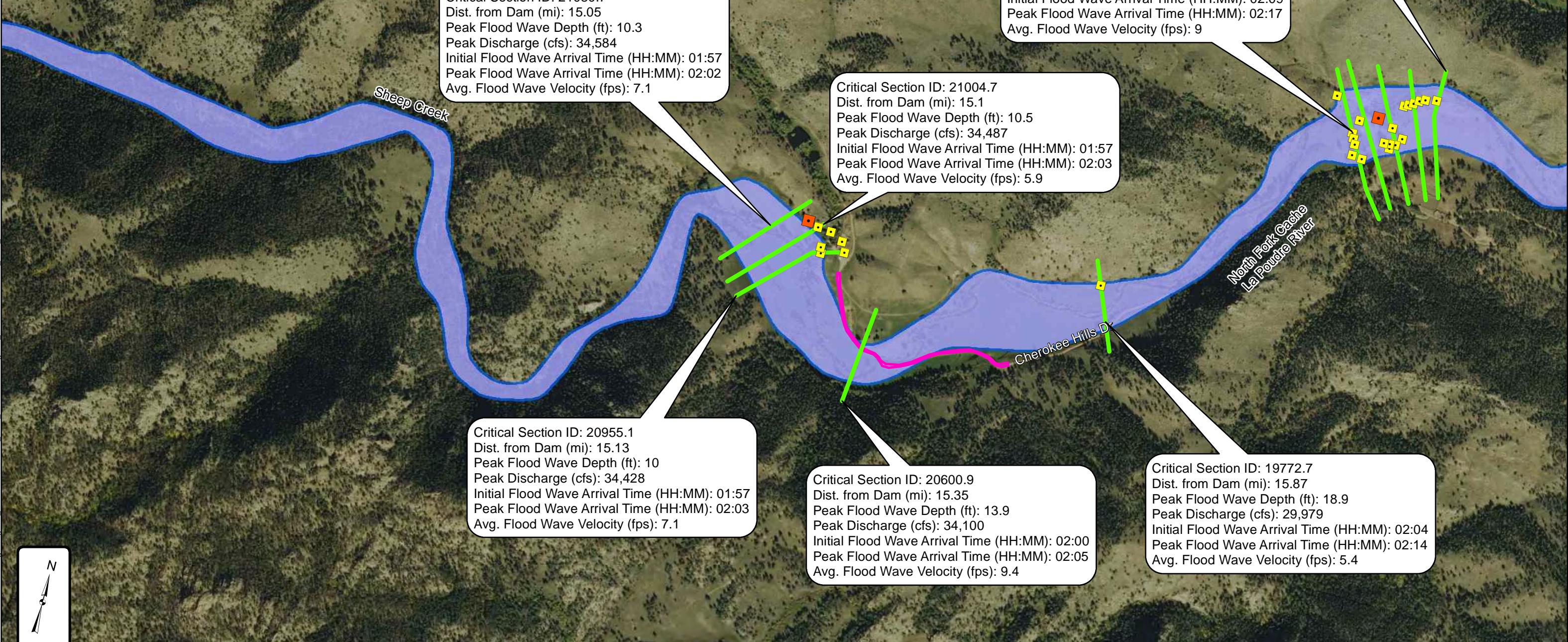
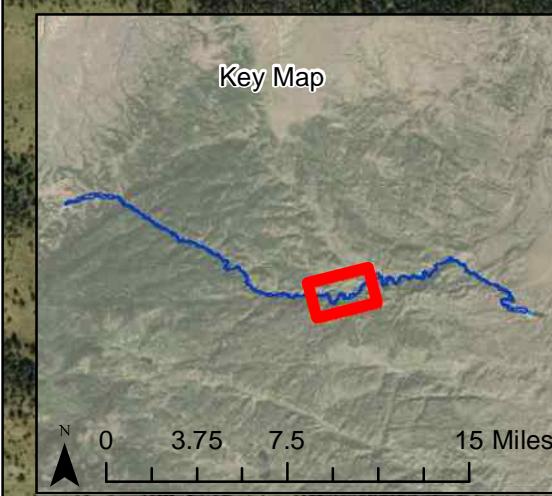
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Project no.: 114-910349

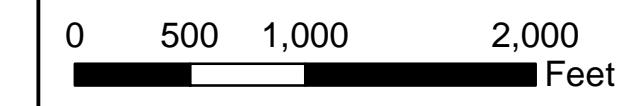
Location: Larimer County, Colorado

May, 2016

Figure 6

Legend:

- Flood Inundation Area
- Structure
- Critical Cross Section
- Potentially Inhabited Structure
- Road Crossing



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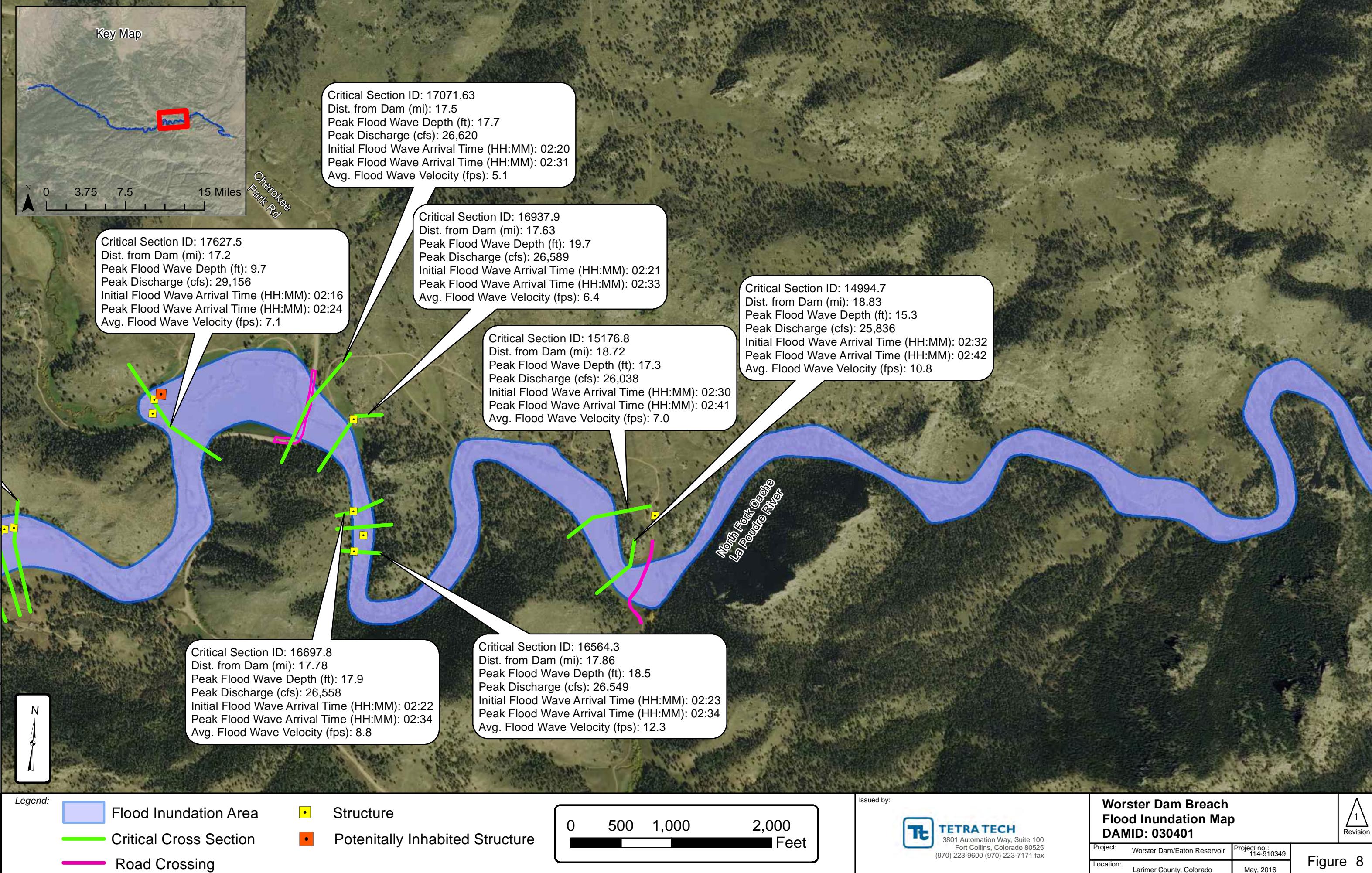
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Flood Inundation Map
DAMID: 030401**

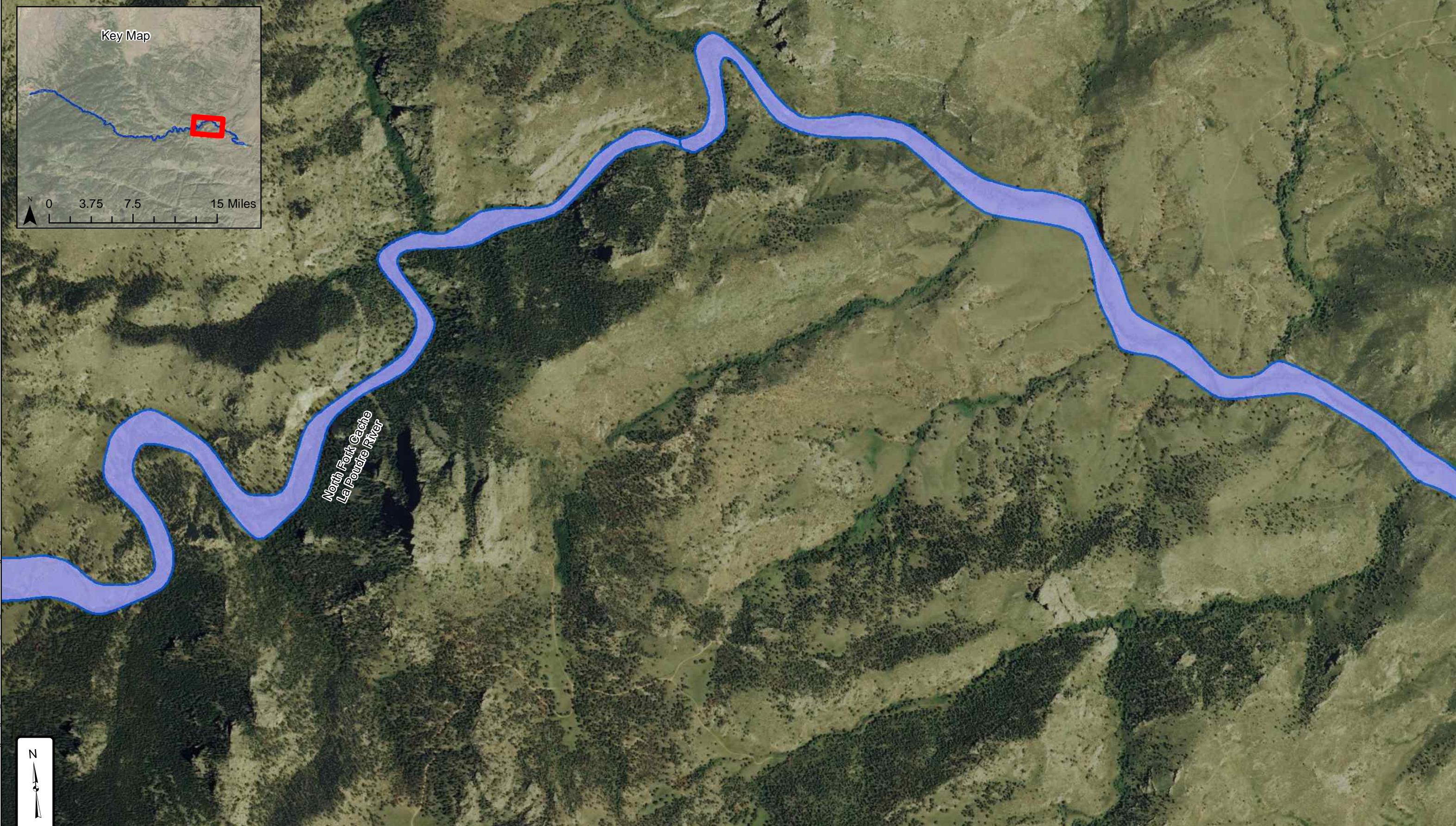
Project: Worster Dam/Eaton Reservoir
Location: Larimer County, Colorado

Project no.: 114-910349
May, 2016

1
Revision

Figure 7





Legend

- Flood Inundation Area
- Critical Cross Section
- Road Crossing

- Structure
- Potentially Inhabited Structure

A scale bar indicating distance in feet. The bar is divided into three segments: a black segment on the left labeled '0', a white segment in the middle, and another black segment on the right labeled '2,000'. Below the bar, the word 'Feet' is written.

Issued by



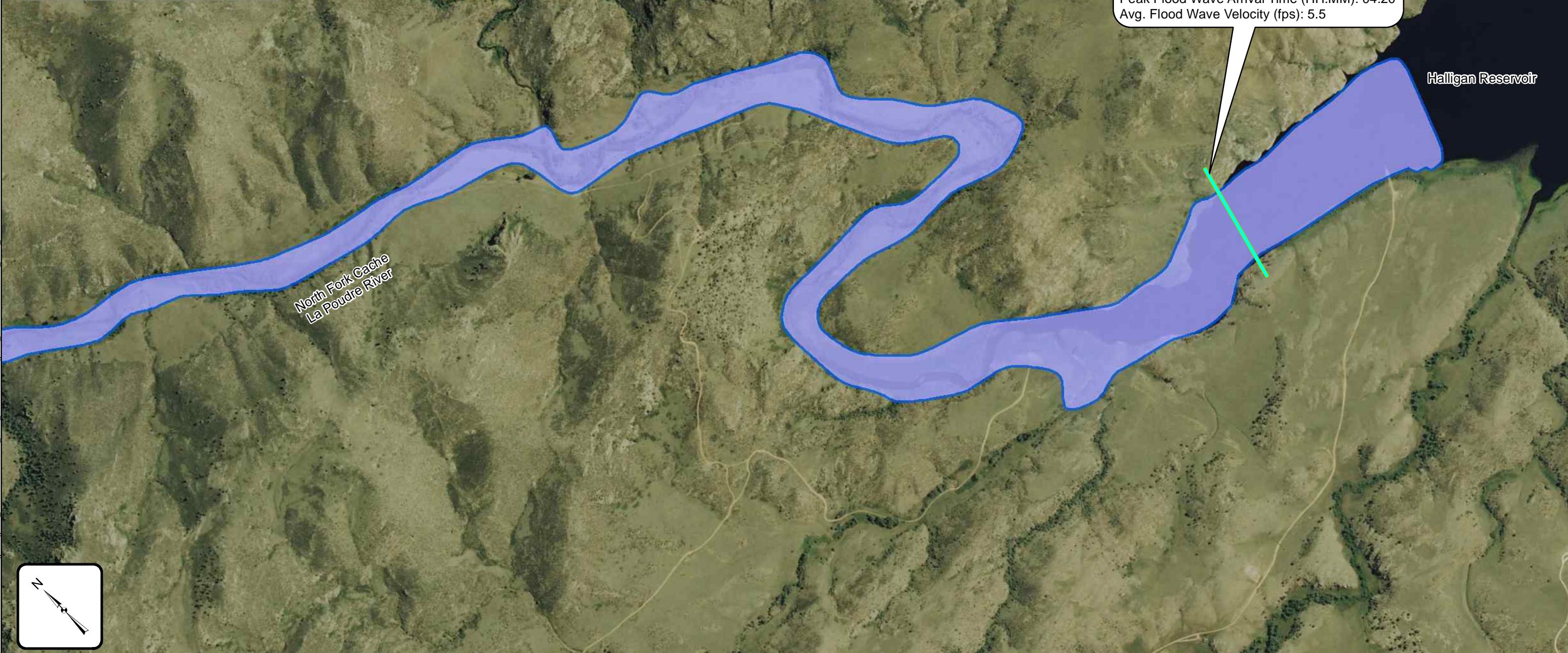
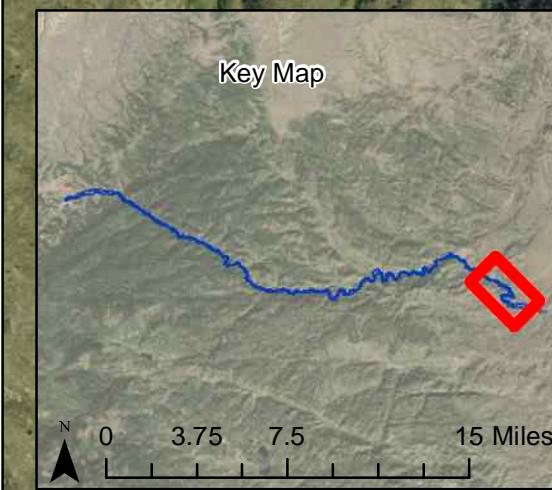
TETRA TECH
3801 Automation Way, Suite 100
Fort Collins, Colorado 80525
(970) 223-9600 (970) 223-7171 fax

Worster Dam Breach Flood Inundation Map DAMID: 030401

A

1

Figure 9



Legend:

- Flood Inundation Area
- Structure
- Critical Cross Section
- Potentially Inhabited Structure
- Road Crossing



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Fort Collins, Colorado 80525
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Worster Dam Breach
Flood Inundation Map
DAMID: 030401

1

Revision

Project: Worster Dam/Eaton Reservoir

Project no.: 114-910349

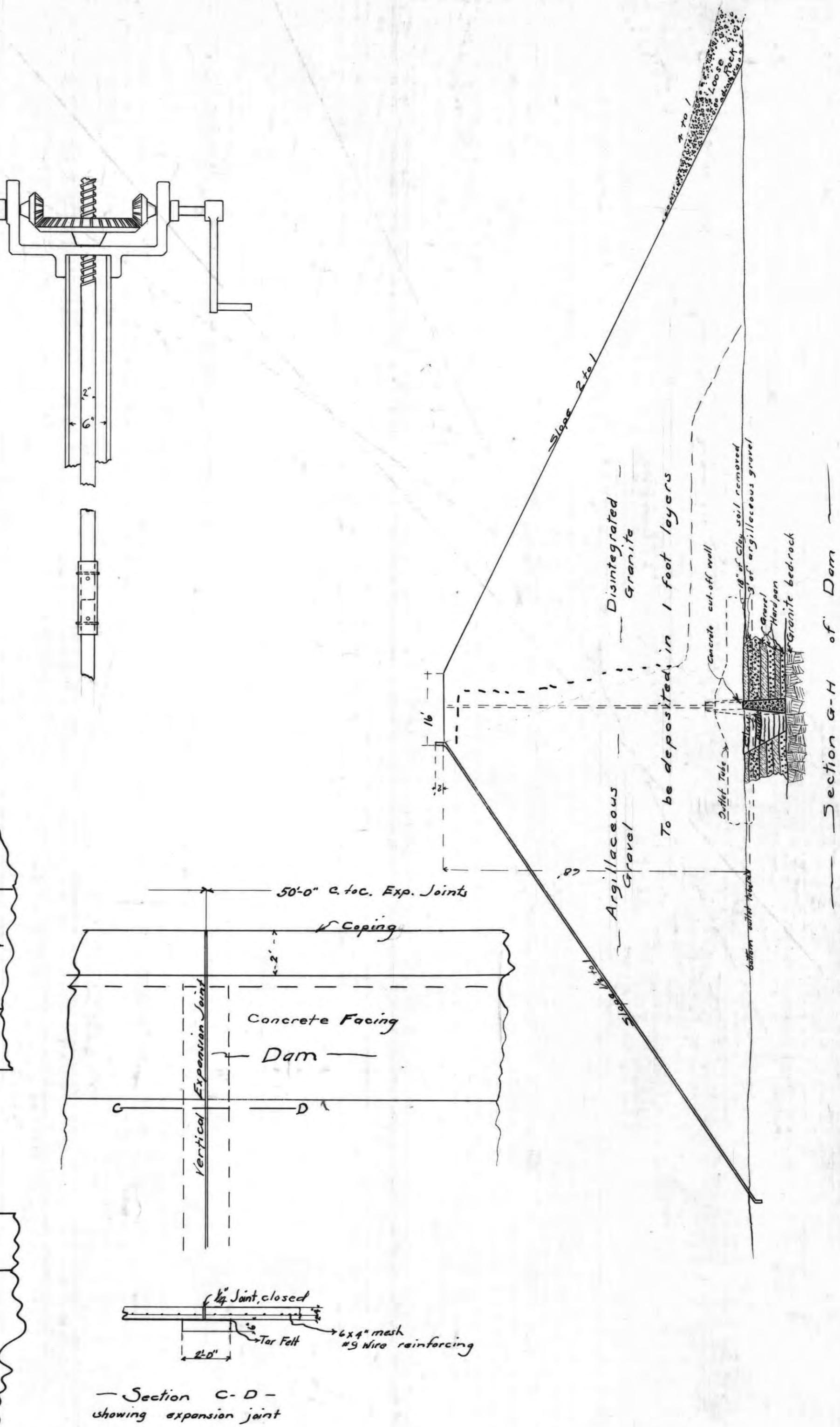
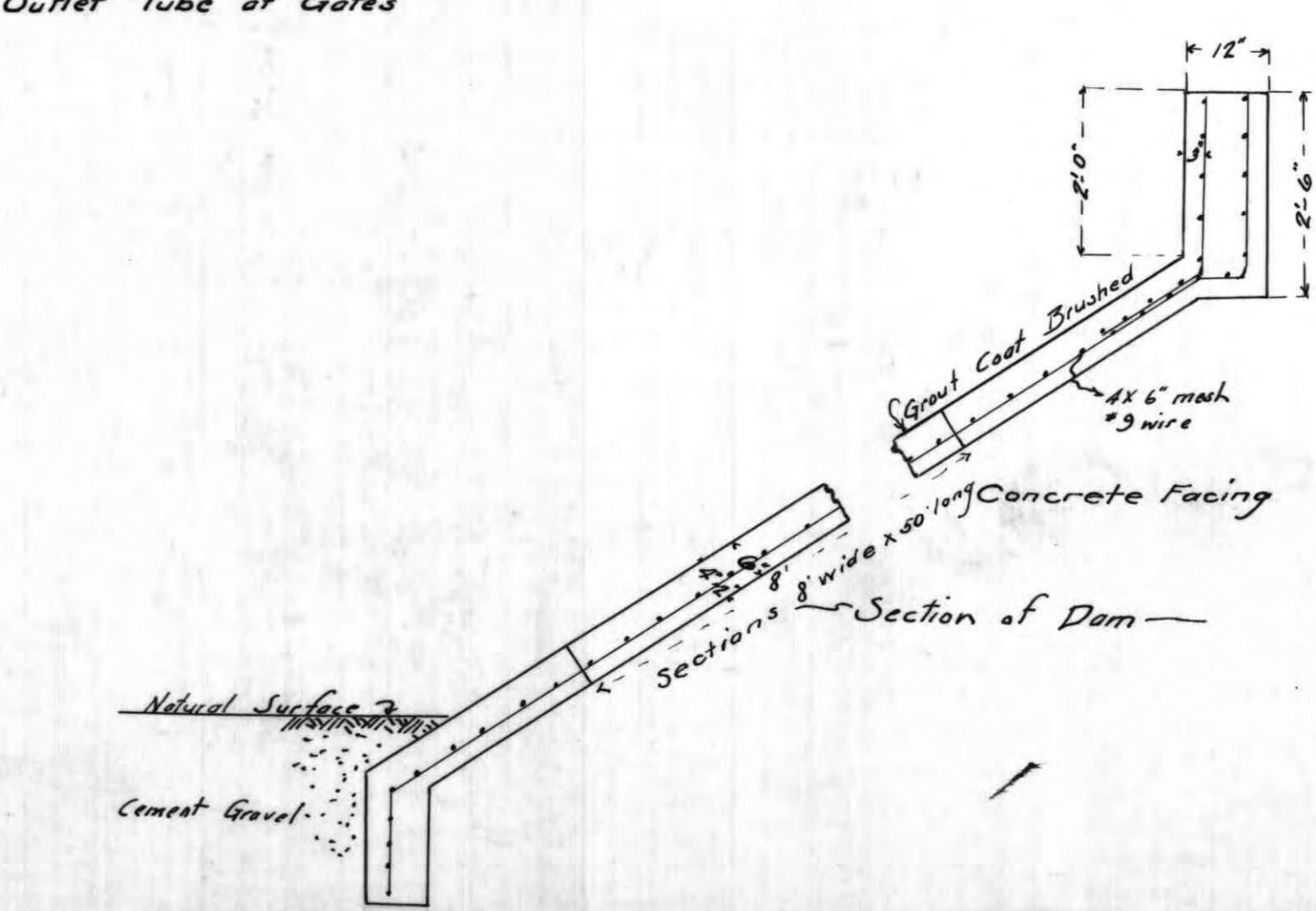
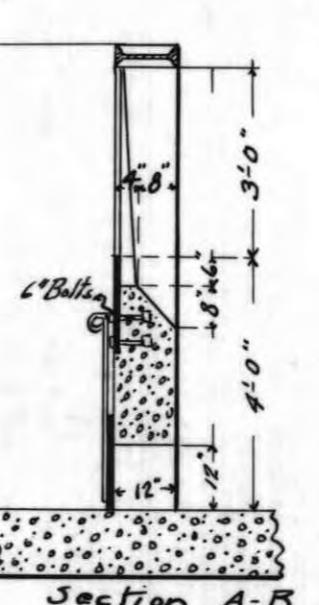
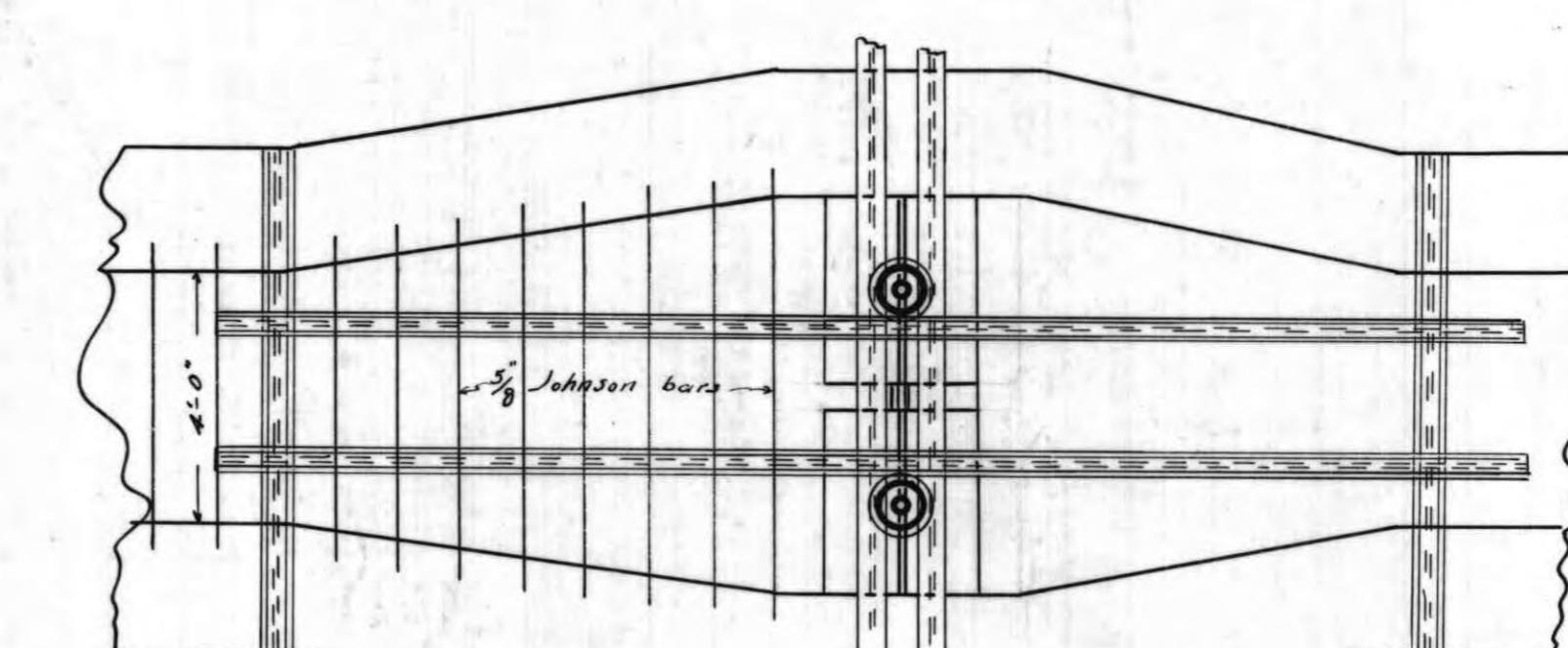
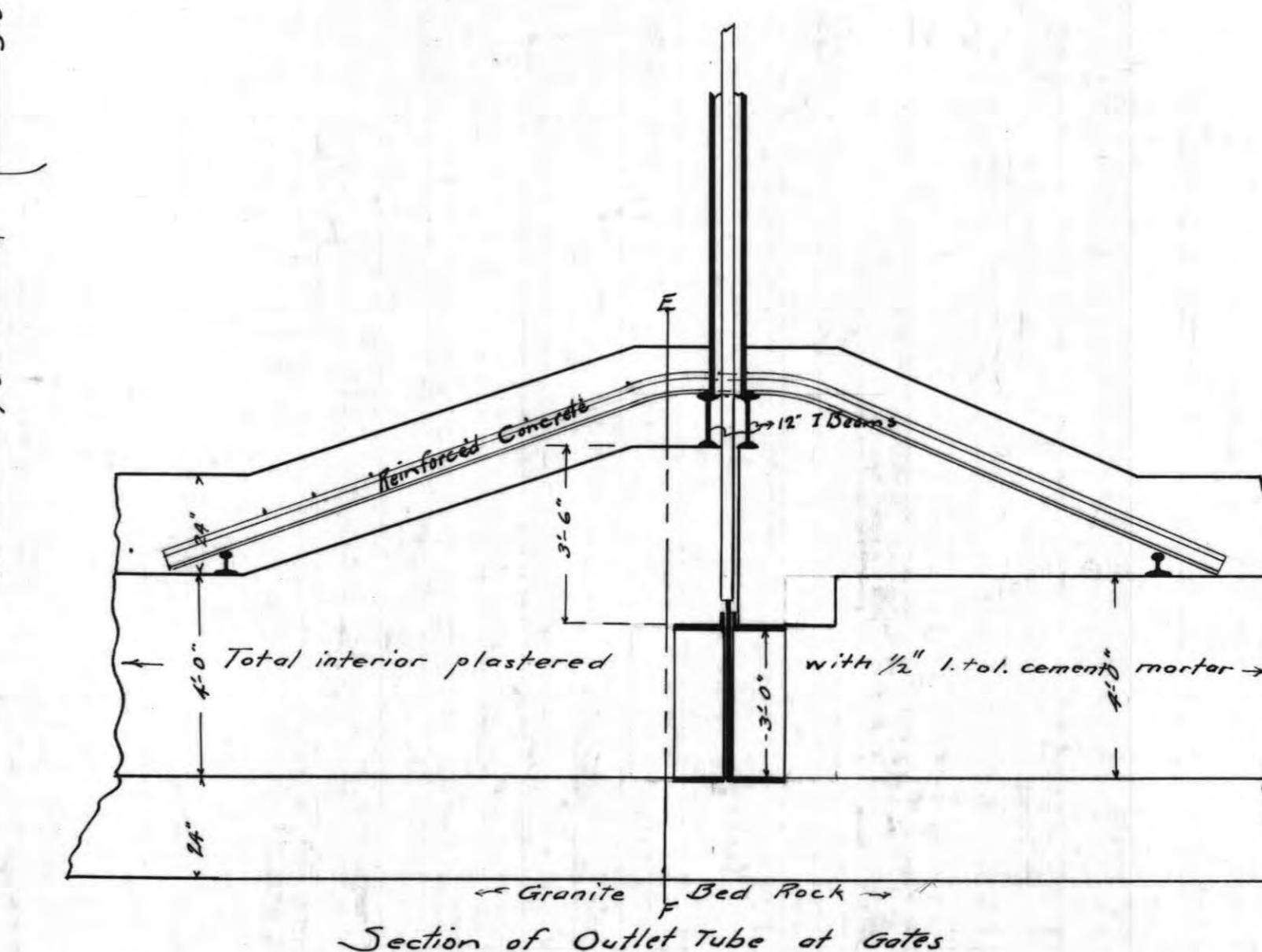
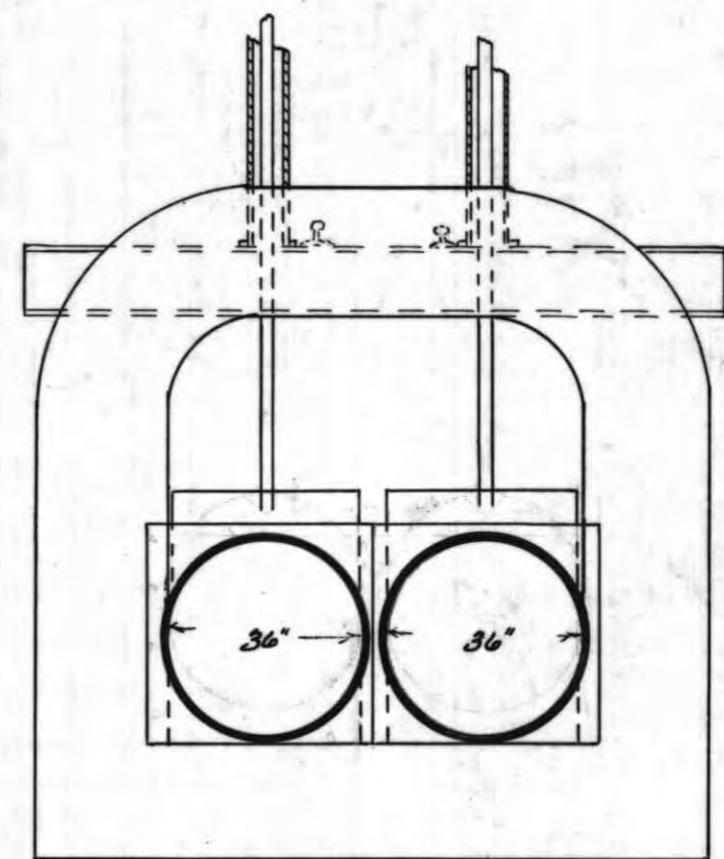
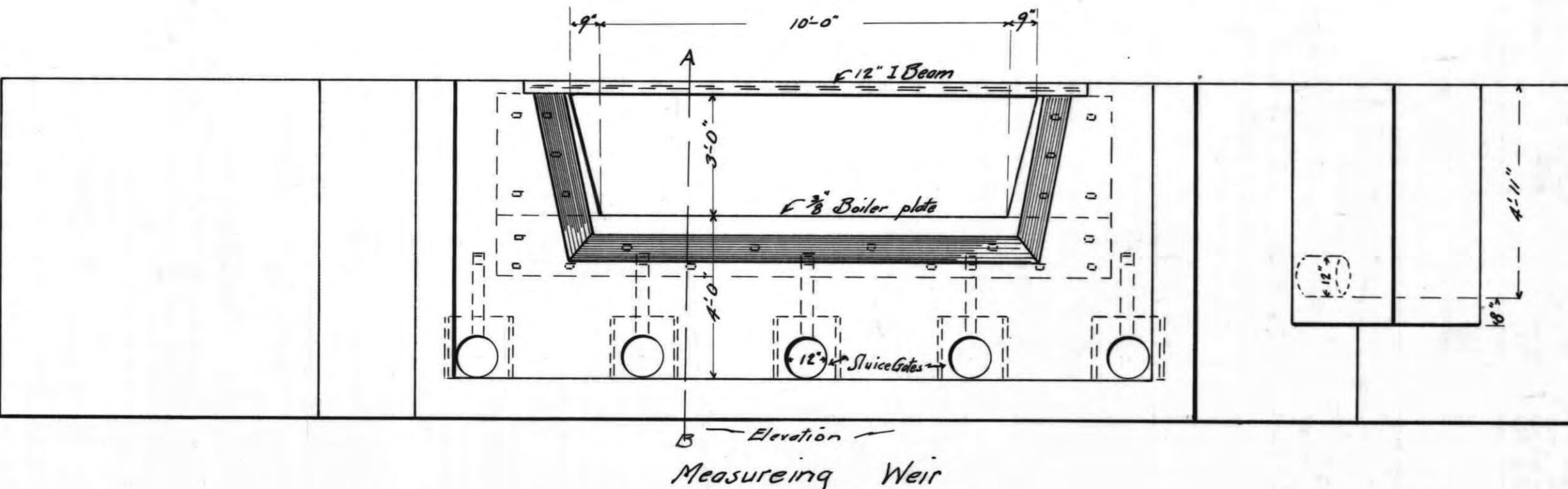
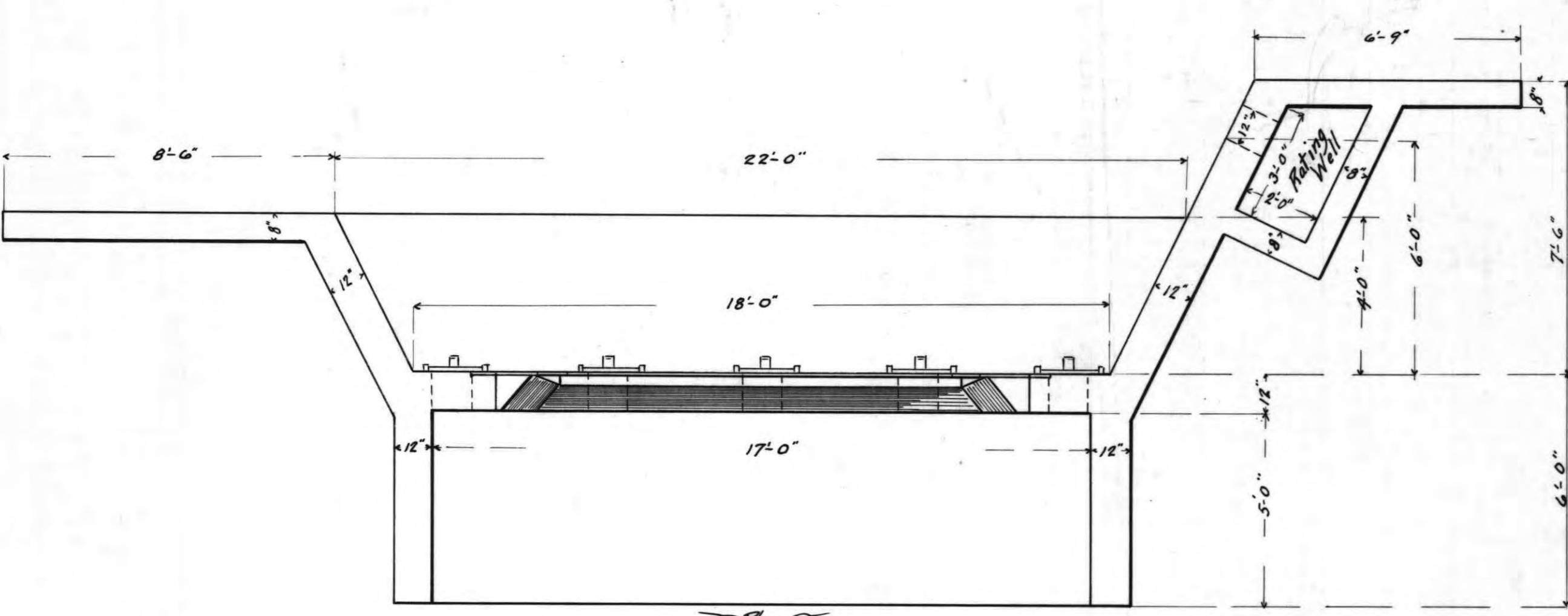
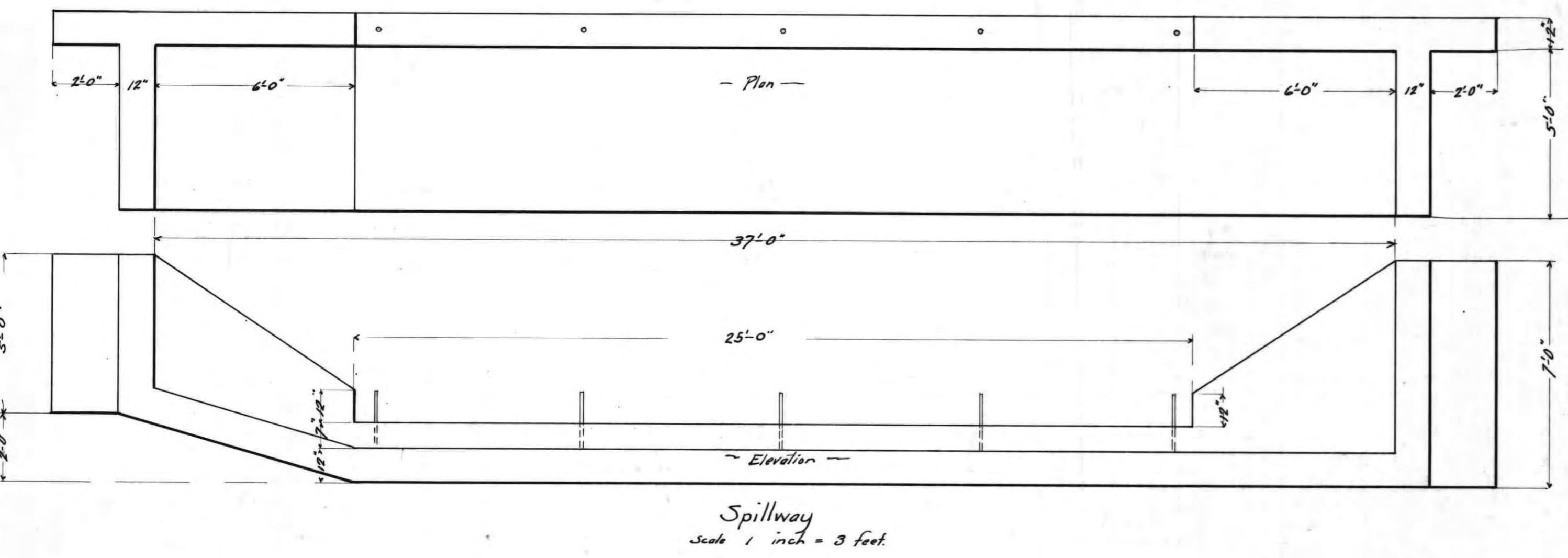
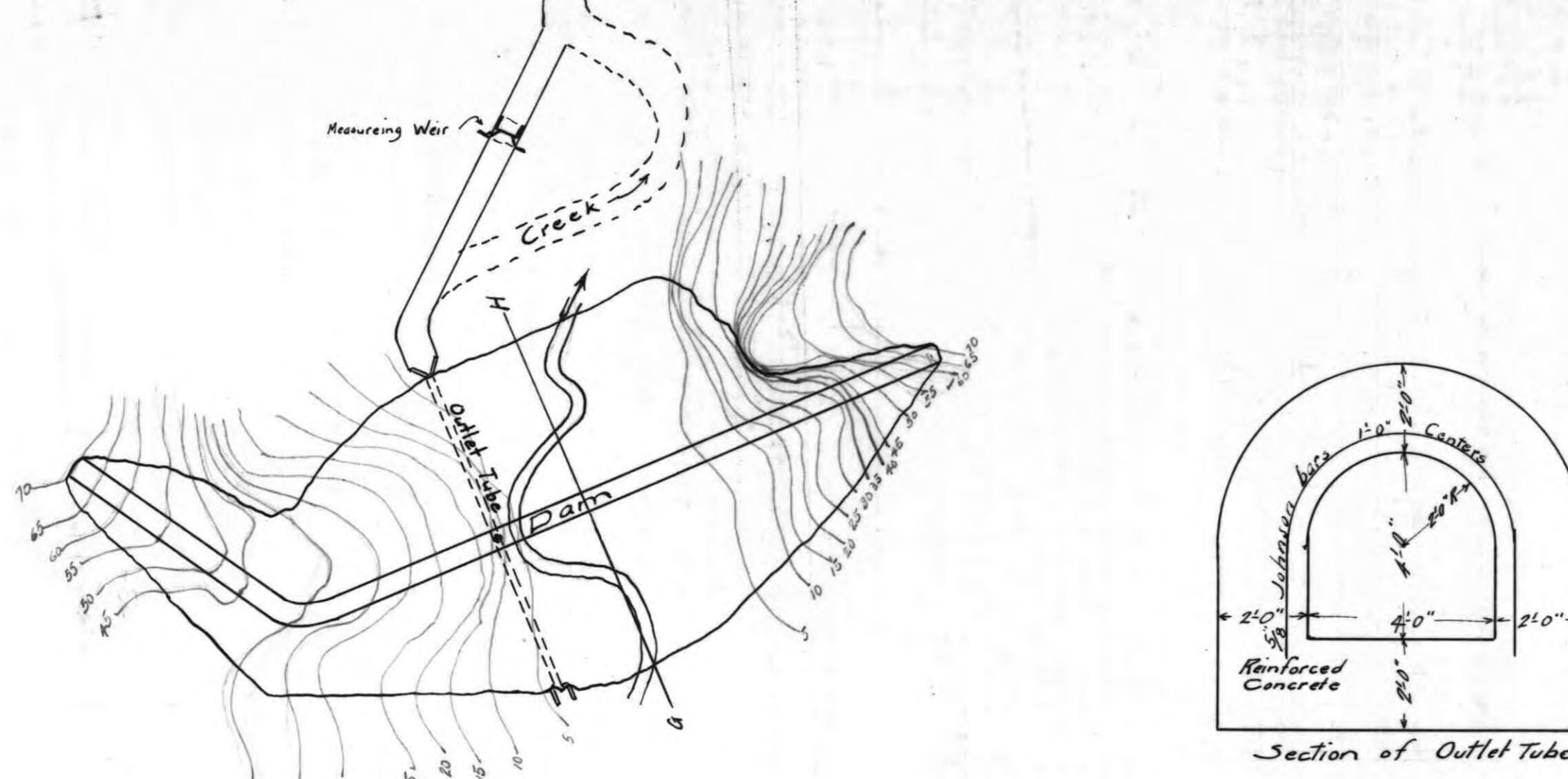
Location: Larimer County, Colorado

May, 2016

Figure 10

APPENDIX A

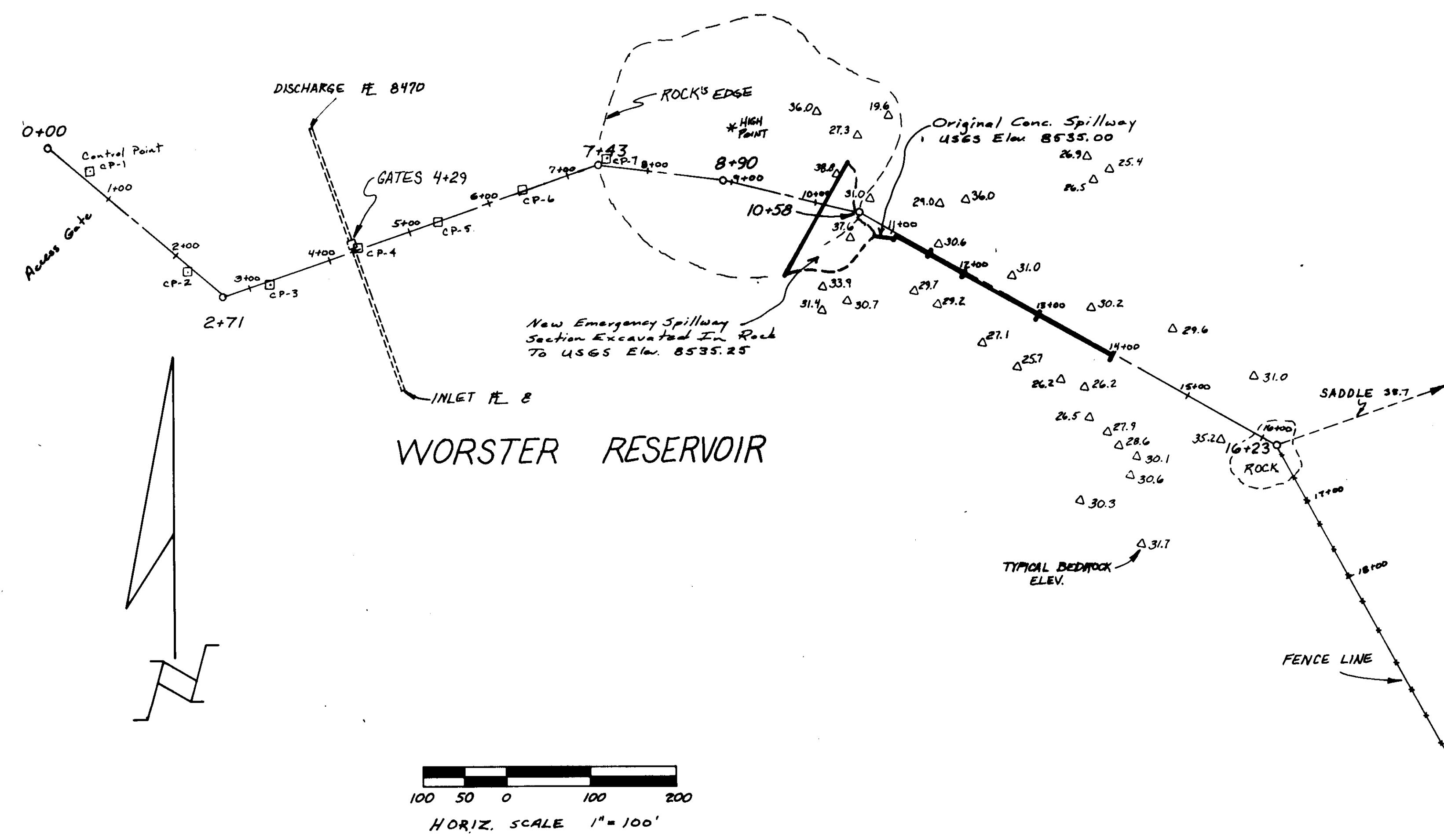
GEOMETRIC DAM INFORMATION - CONSTRUCTION PLAN SET



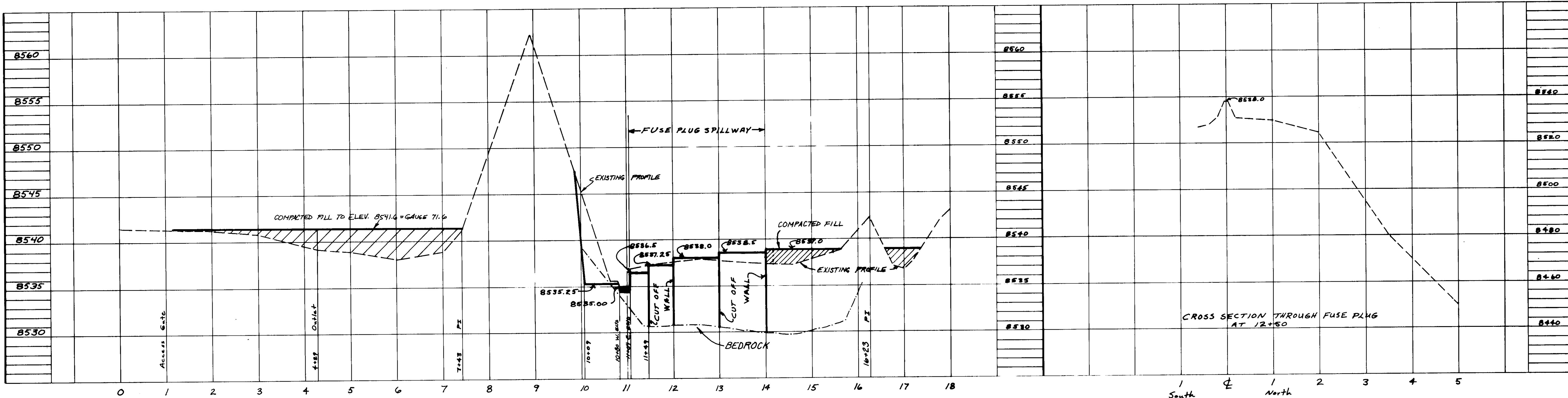
WORSTER RESERVOIR DAM
The Divide Canal & Reservoir Co.
Claimant:
Address Eaton, Colo.
Cr. C. W. Candlin,
Secy.

Filed Feb. 11th 1911, by

O.F. Shattuck,
Greeley, Colo.
C-56



A hand-drawn diagram illustrating a sheet piling wall. The wall is shown as a vertical line with horizontal segments extending from it. A bracket above the wall reads "Sheet Piling Driven To Bedrock". An arrow labeled "FLOW" points upwards next to the wall. To the right, a vertical dimension line indicates a height of "12'". Below the wall, a vertical line has a bracket indicating a distance of "6'". A curved line labeled "Steel Rail Section Deadman Anchor" extends downwards from the wall. Another label "Rebar Ties" is positioned below the anchor. At the base of the wall, a slope is shown with the label "Riprap".

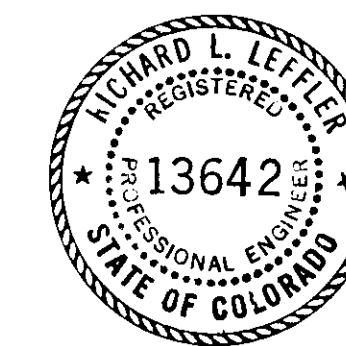


NOTE: From approx. sta. 0+50 to sta. 3+50 the upstream face of the dam was reconstructed at a slope of 3:1. From sta. 0+50 to 7+50 and from sta. 14+00 to 16+00 a 12" gravel blanket and an 18" layer of rock riprap was placed from the dam crest down to USGS Elevation 8528.

Control Points are steel pipe set in concrete with chisel marks on the caps.

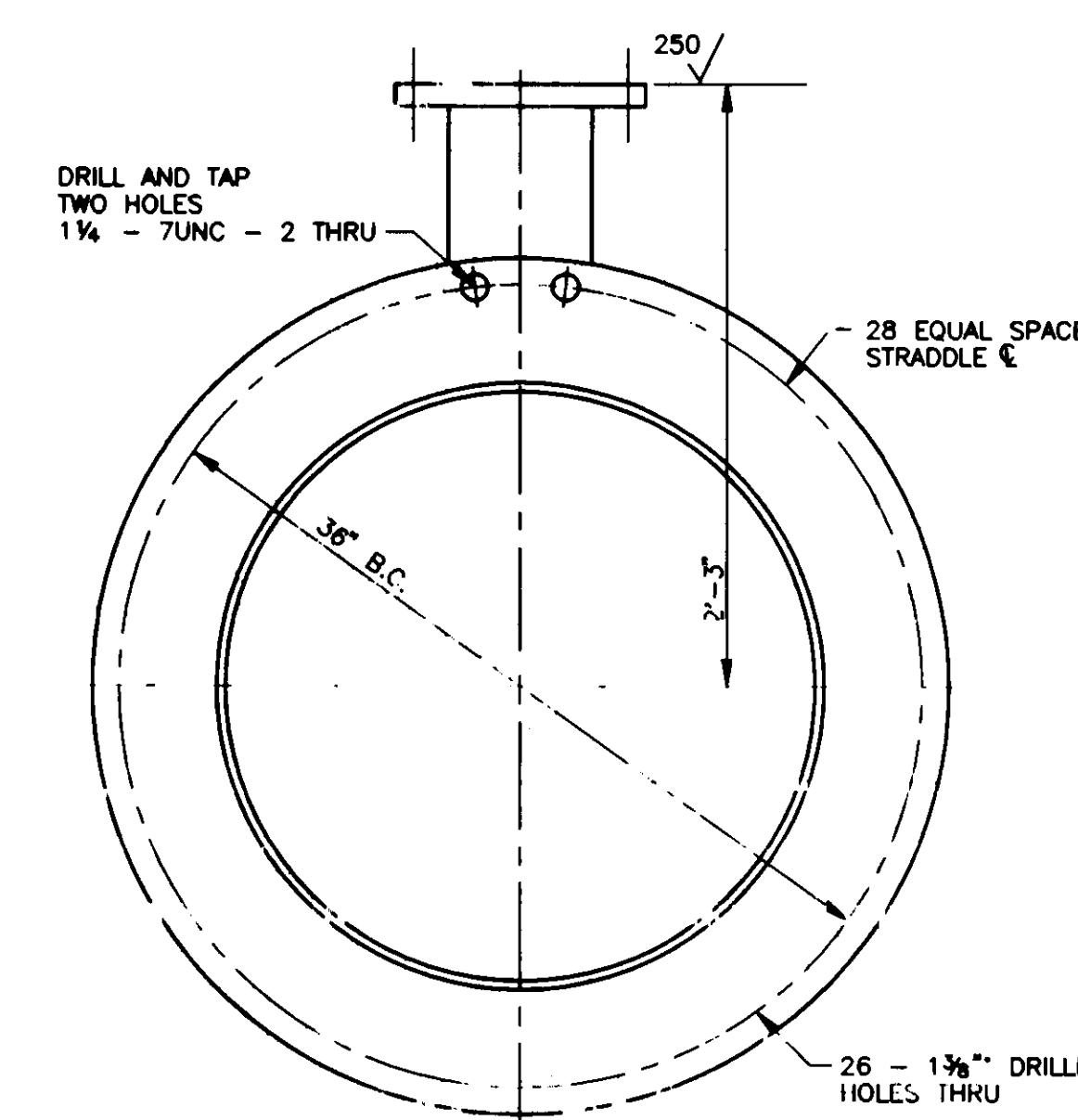
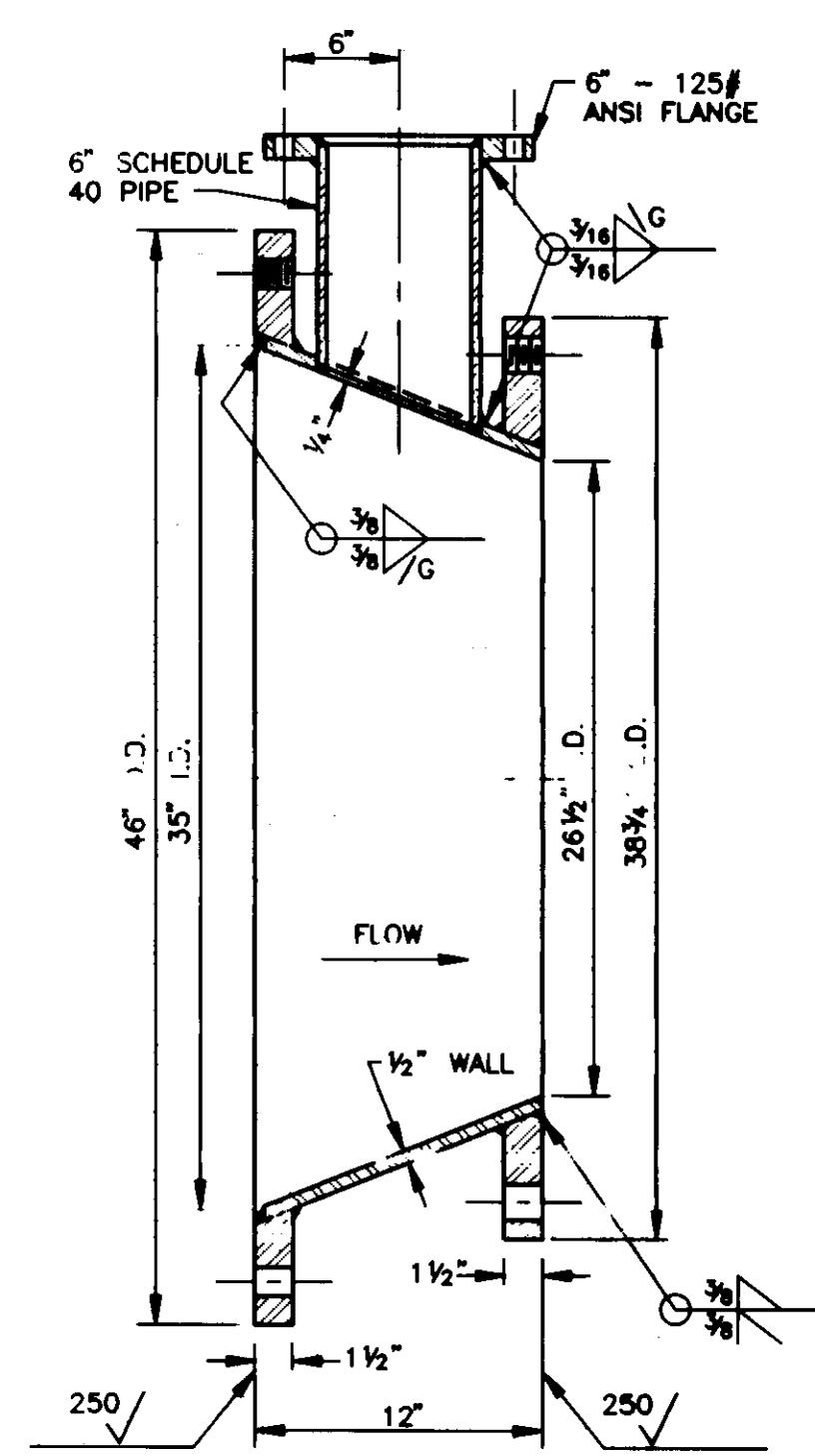
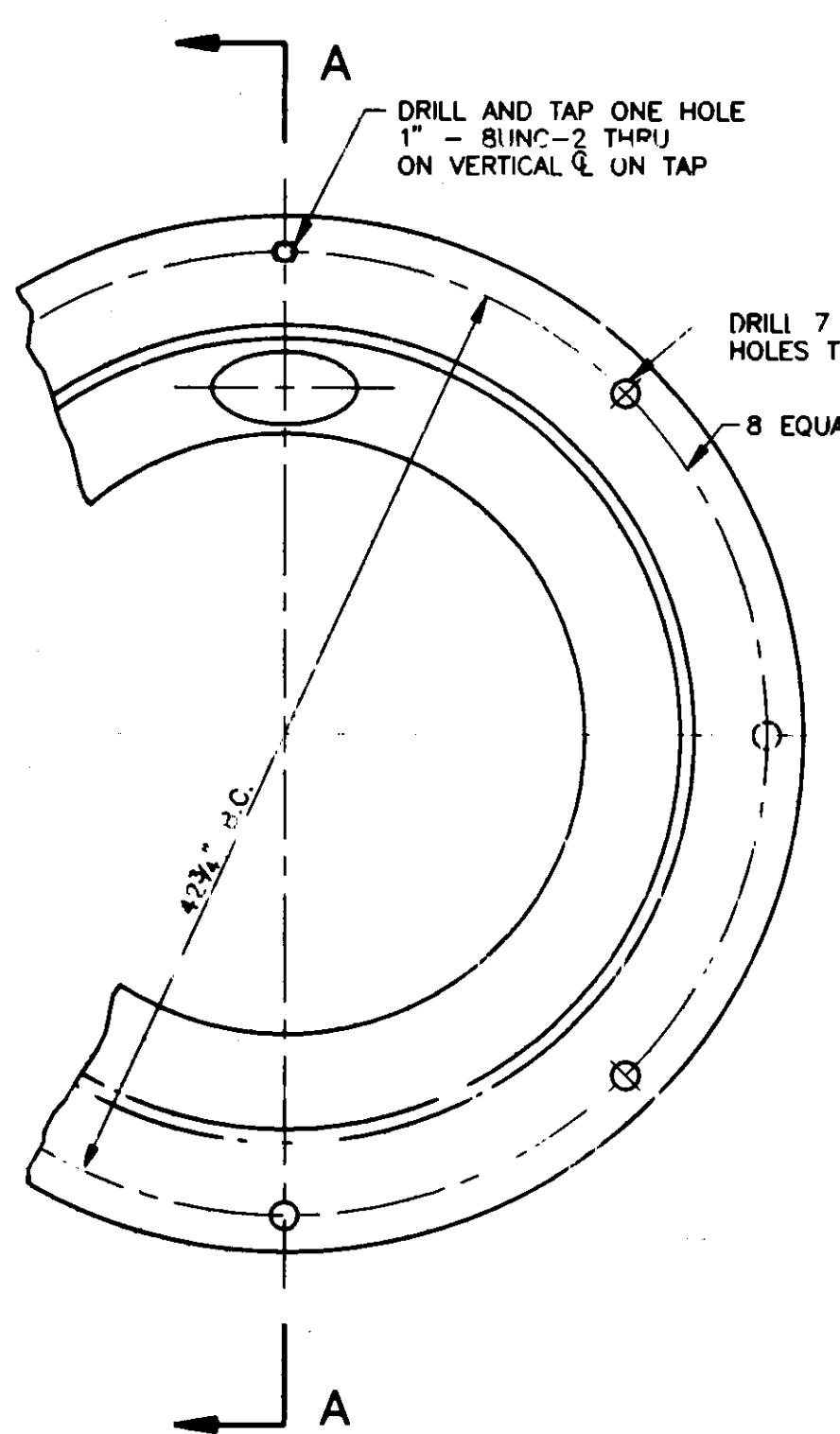
I hereby certify that these plans represent the
As-Constructed Conditions of Worster Reservoir Dam
as of the 20th day of September, 1988.

Richard L. Leffler



Revisions: AS-BUILT 10-21-86 Scale: DIVIDE CANAL AND RESERVOIR CO.
Drawn By: TMG Checked By: RLL Project No. 259.8302 Date: SEPT. 6, 1986 Sheet 1 of 1

WORSTER RESERVOIR
WATER DIVISION 1 , DISTRICT 3 **C-56A**
DAM I.D. # 030401
DIVIDE CANAL AND RESERVOIR CO.

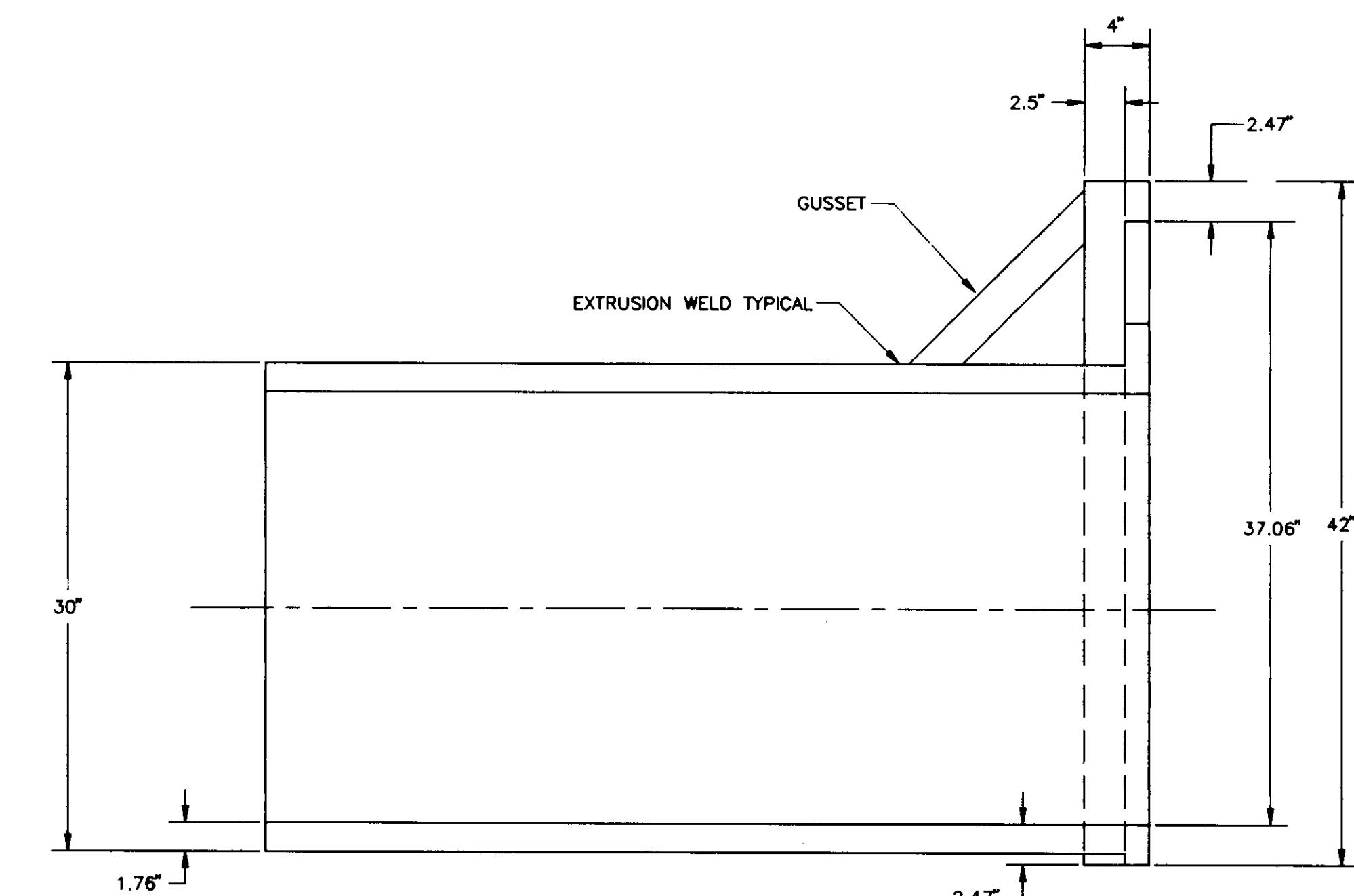


STEEL REDUCING THIMBLE SECTION AND DETAIL

SCALE: $1\frac{1}{2}" = 1'-0"$

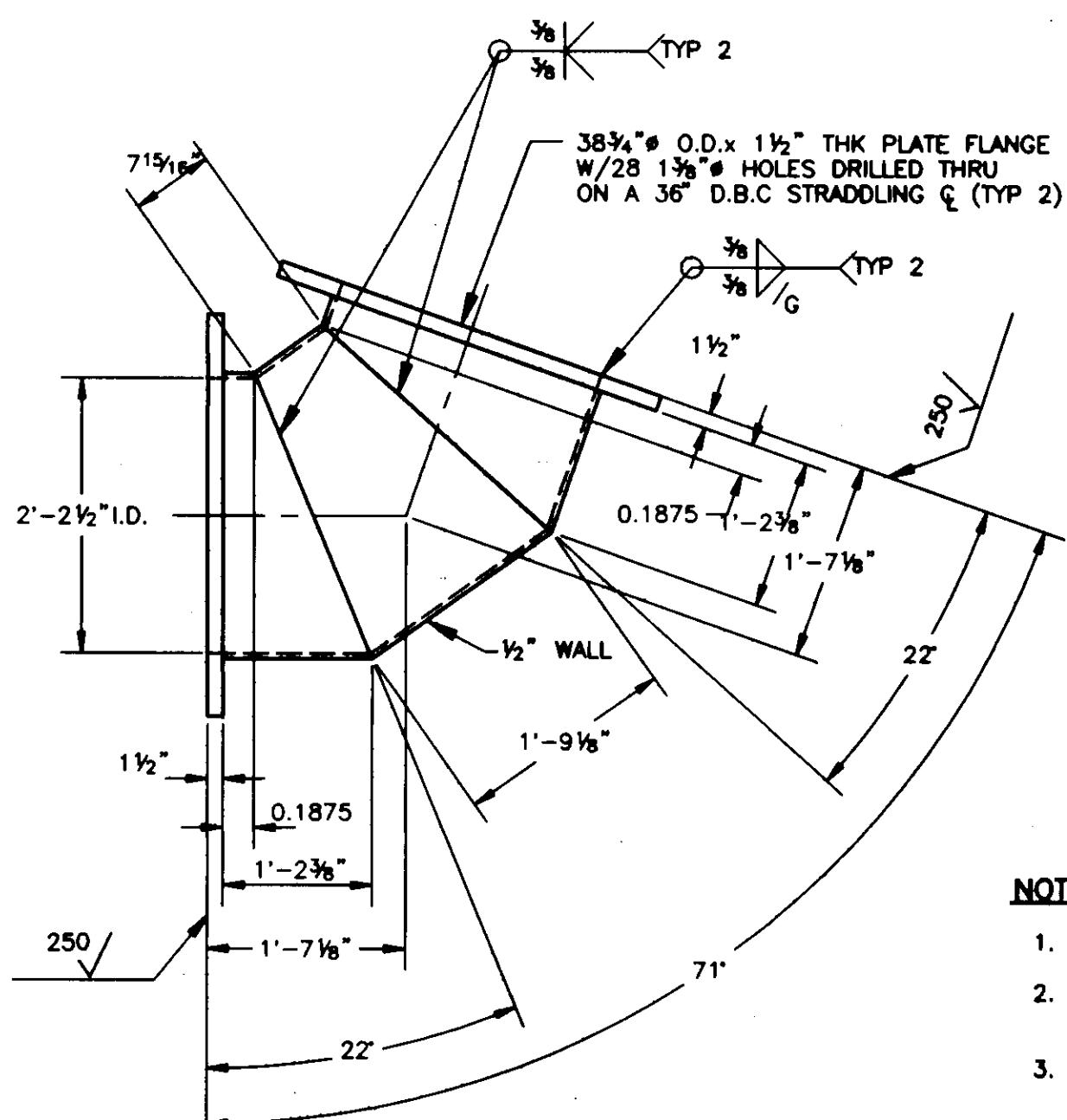
NOTES:

1. STEEL SHALL BE ASTM A36 OR EQUAL
2. BLAST CLEAN INTERIOR FLUIDWAY PER SSPC-SPI0
3. PAINT INTERIOR FLUIDWAY ONLY WITH TNEMEC 46H-413 COAL TAR EPOXY TDFT: 16 MILS COLOR: BLACK
OR AMERON-AMERLOCK 400 HIGH SOLIDS EPOXY TDFT: 16 MILS COLOR: WHITE



HDPE ECCENTRIC REDUCER SECTION

SCALE: $1\frac{1}{2}" = 1'-0"$



NOTES:

1. STEEL SHALL BE ASTM A36 OR EQUAL
2. BLAST CLEAN INTERIOR FLUIDWAY PER SSPC-SPI0
3. PAINT INTERIOR FLUIDWAY ONLY WITH TNEMEC 46H-413 COAL TAR EPOXY TDFT: 16 MILS COLOR: BLACK
OR AMERON-AMERLOCK 400 HIGH SOLIDS EPOXY TDFT: 16 MILS COLOR: WHITE

STEEL ELBOW DETAIL

SCALE: $\frac{3}{4}" = 1'-0"$

RECORD DRAWING

| DRAWN BY: G.O.J. | DATE: 7/1/93 | CHECKED BY: C.N.H. | DATE: 5/1/95 |
|--|-----------------|-----------------------|-----------------|
| AS CONSTRUCTED - CORRECTIONS COMPLETE BY C.N.H. DATE 12/28/95 | | | |
| REVISIONS | | | |
| NO. | DESCRIPTION | DATE | BY |
| | | | |
| | | | |
| | | | |
| | | | |

INLET ELBOW, REDUCING THIMBLE AND HDPE REDUCER SECTIONS AND DETAIL

Woodward-Clyde Consultants
Consulting Engineers Denver, Colorado

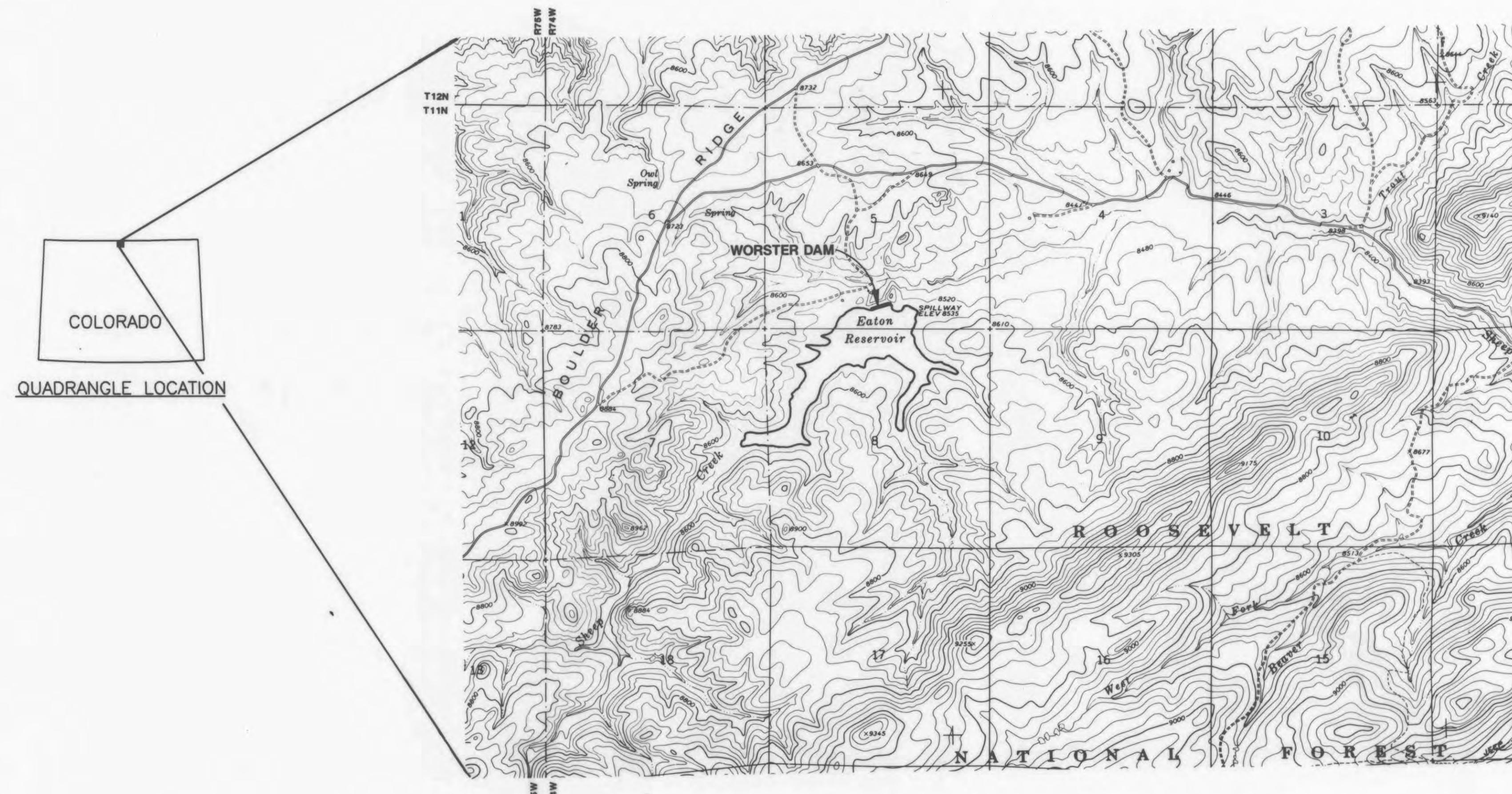
WORSTER DAM AND OUTLET REHABILITATION PROJECT NO. 23172-23112

DRAWING NO. OF

5 5

CONSTRUCTION PLANS FOR WORSTER DAM AND OUTLET REHABILITATION

SECTION 5, TOWNSHIP 11 NORTH, RANGE 74 WEST
LARIMER COUNTY, COLORADO
WATER DIVISION 1, WATER DISTRICT 3, DAM ID. 030401



INDEX OF DRAWINGS

| DRAWING NO. | TITLE |
|-------------|--|
| 1 OF 5 | COVER SHEET AND LOCATION MAP |
| 2 OF 5 | FLEXIBLE MEMBRANE LINER AND BORROW AREA PLAN VIEW AND LINER ANCHOR DETAILS |
| 3 OF 5 | OUTLET AND VALVE CHAMBER PROFILE, SECTIONS AND DETAILS |
| 4 OF 5 | INLET TRASH RACK, WEIR BOX AND STAFF GAUGE, PLAN, SECTIONS, AND DETAILS |
| 5 OF 5 | INLET ELBOW, REDUCING THIMBLE AND HDPE REDUCER SECTIONS AND DETAIL |

PREPARED FOR
CANAL DIVIDE AND RESERVOIR COMPANY
PREPARED BY
WOODWARD-CLYDE CONSULTANTS
DENVER, COLORADO



RECORD DRAWING

| DRAWN BY: | DATE: | CHECKED BY: | DATE: |
|---------------------------------------|-------------------------|-------------|--------|
| G.O.J. | 7/1/93 | C.N.H. | 5/1/95 |
| AS CONSTRUCTED - CORRECTIONS COMPLETE | | | |
| BY C.N.H. DATE 12/28/95 | | | |
| REVISIONS | | | |
| NO. | DESCRIPTION | DATE | BY |
| ▲ | ADD SHEET 5 WITH DETAIL | 12/28/95 | C.N.H. |
| | | | |
| | | | |

| COVER SHEET AND LOCATION MAP | | | |
|--|-------------|----|----------|
| Woodward-Clyde Consultants Consulting Engineers Denver, Colorado | | | |
| WORSTER DAM AND OUTLET REHABILITATION PROJECT NO. 23172-23112 | DRAWING NO. | OF | 1 5 |

Daniel L. Johnson CO. PE # 13365
DANIEL L. JOHNSON
APPROVED ON THE 24th DAY OF MAY 19 95
Hal D. Simpson
STATE ENGINEER
Robert J. Stoy
DEPUTY

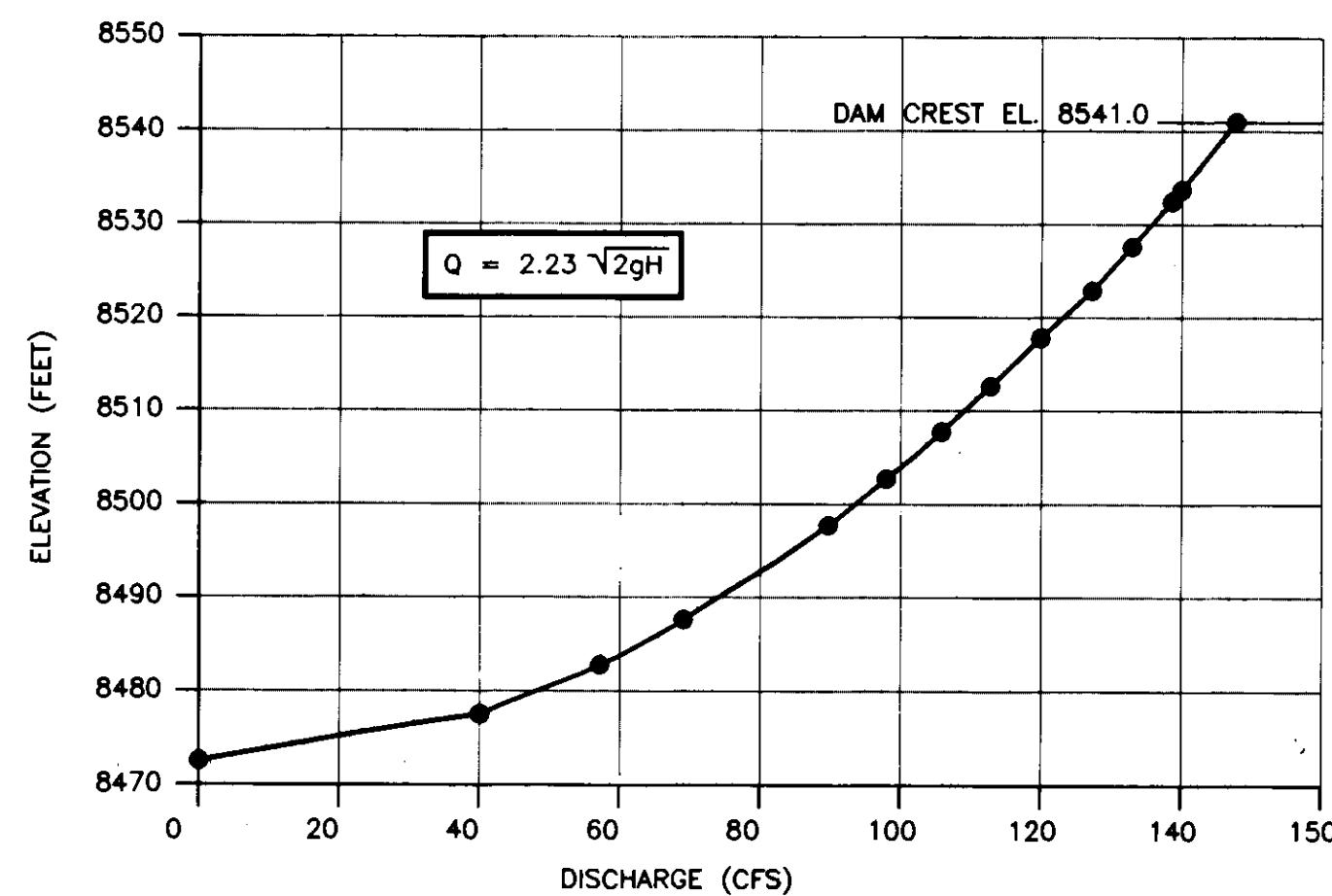
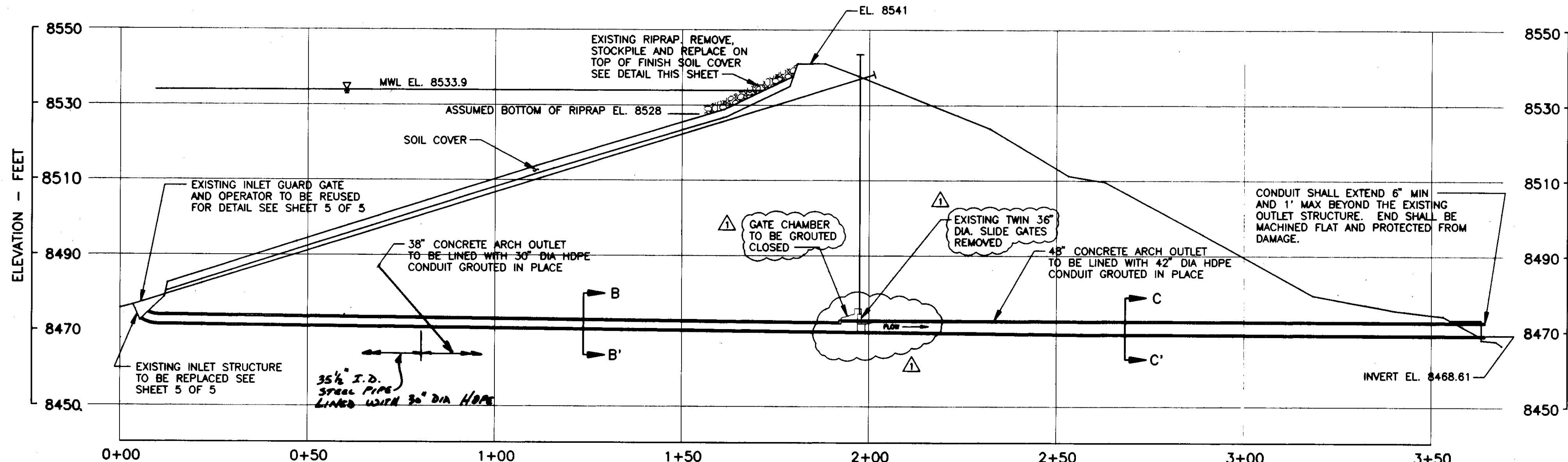


THESE PLANS REPRESENT THE AS-CONSTRUCTED CONDITIONS OF
WORSTER DAM OUTLET REHABILITATION TO THE BEST KNOWLEDGE
AND JUDGMENT, BASED IN PART ON INFORMATION FURNISHED
BY OTHERS AS OF THE 21 DAY OF JULY, 19 96.

Daniel L. Johnson CO. PE # 13365
ENGINEER'S PRINTED NAME
C-0056B

Daniel L. Johnson
SIGNATURE

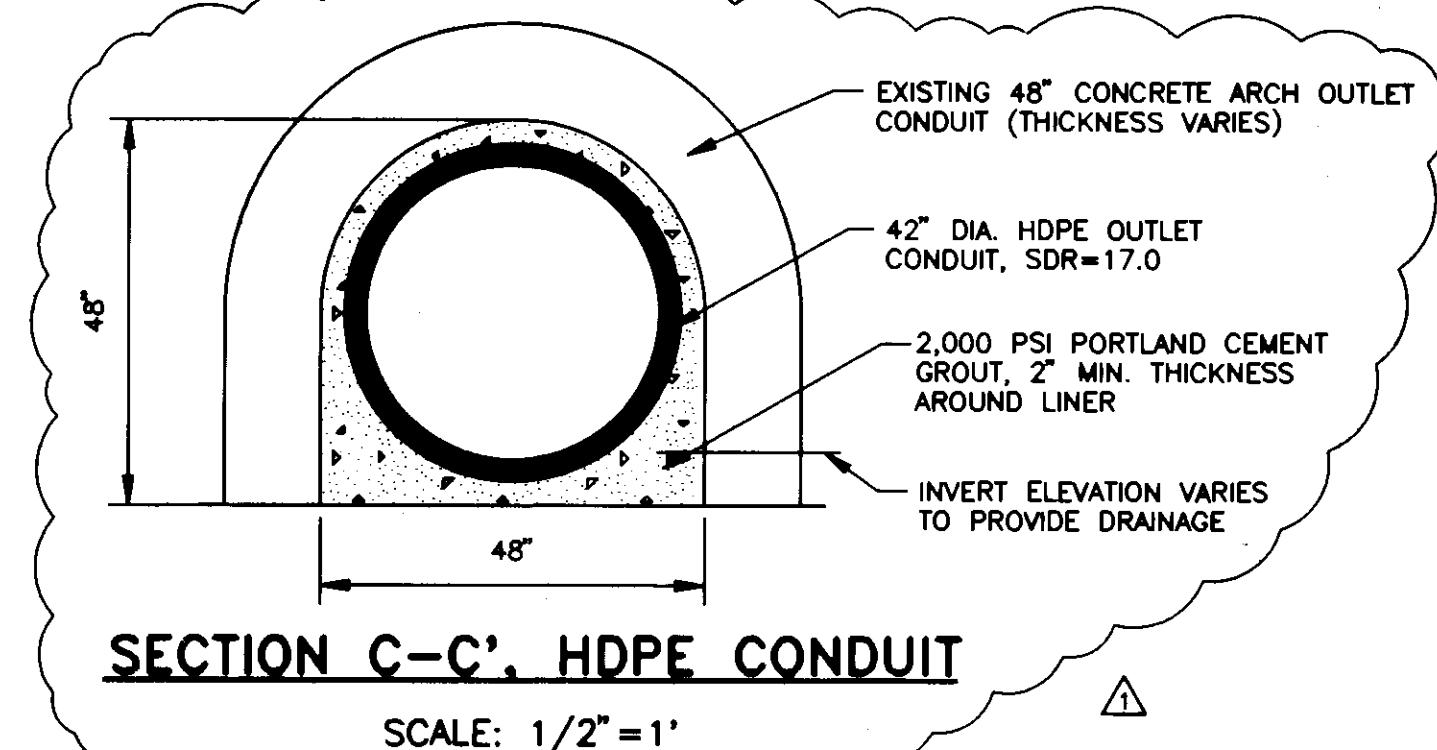
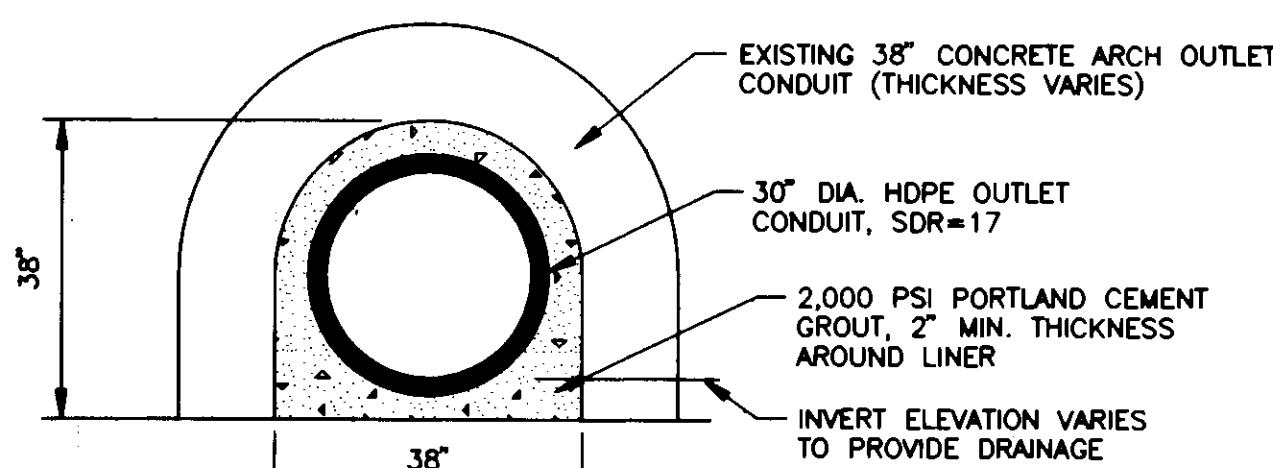
C-0056B



OUTLET WORKS RATING CURVE

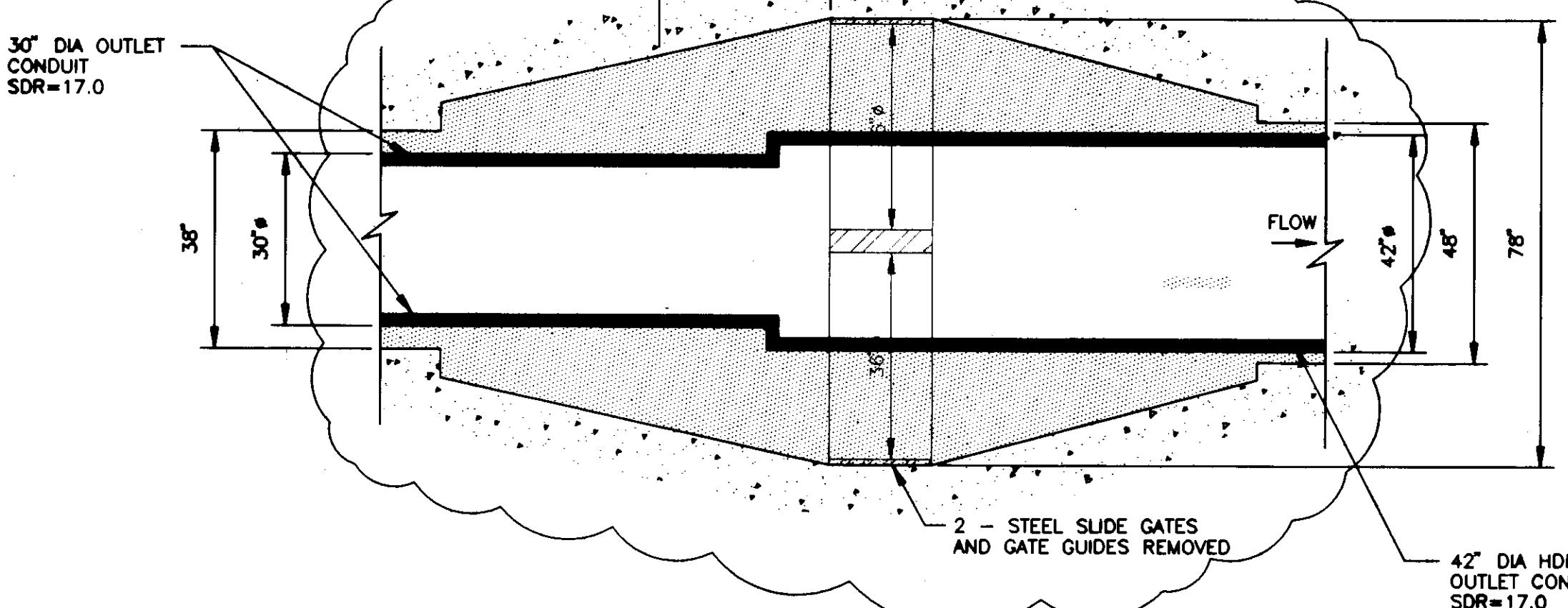
OUTLET CONDUIT PROFILE A-A'

SCALE: 1"=20'



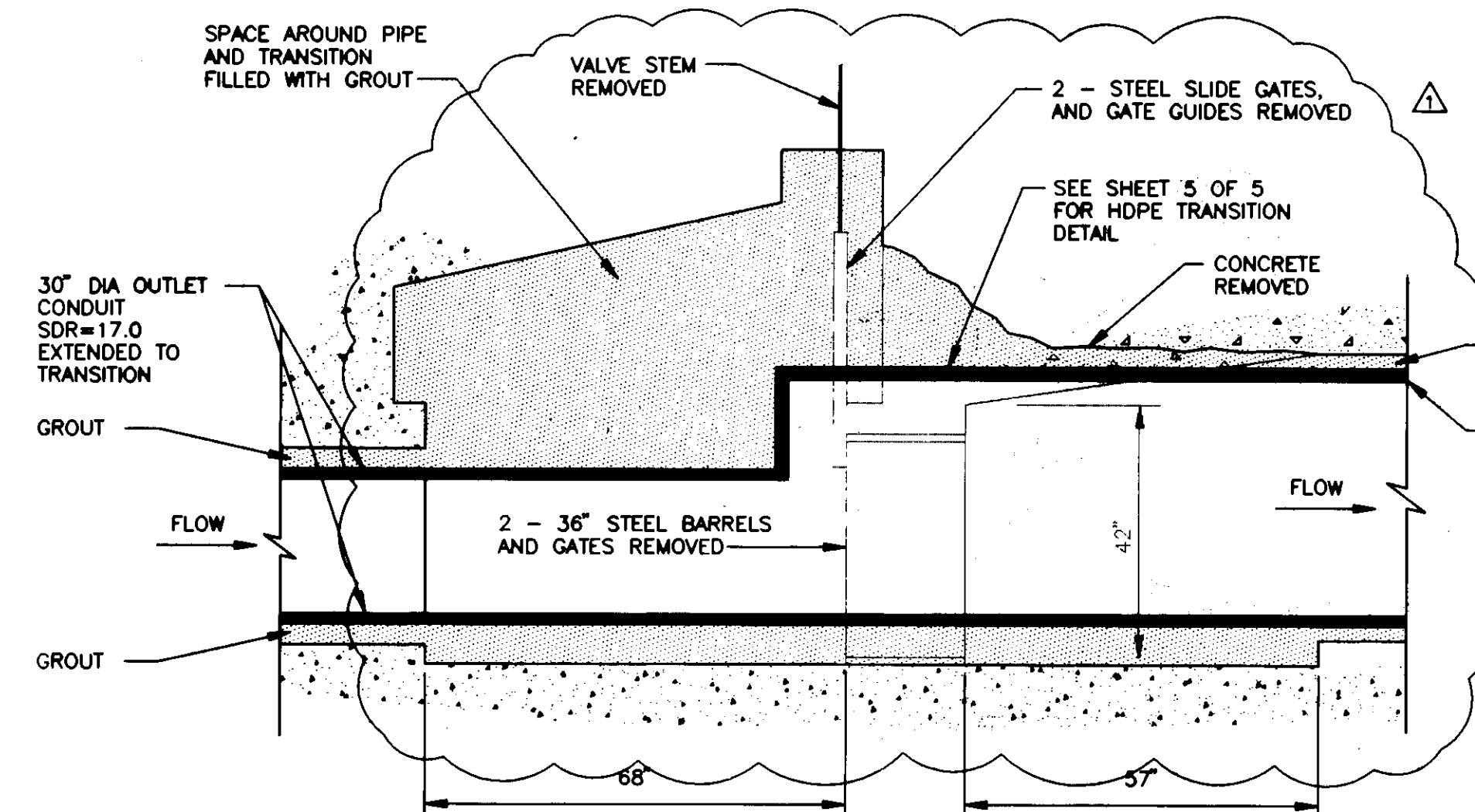
SECTION B-B'. HDPE CONDUIT

SCALE: 1/2"=1'



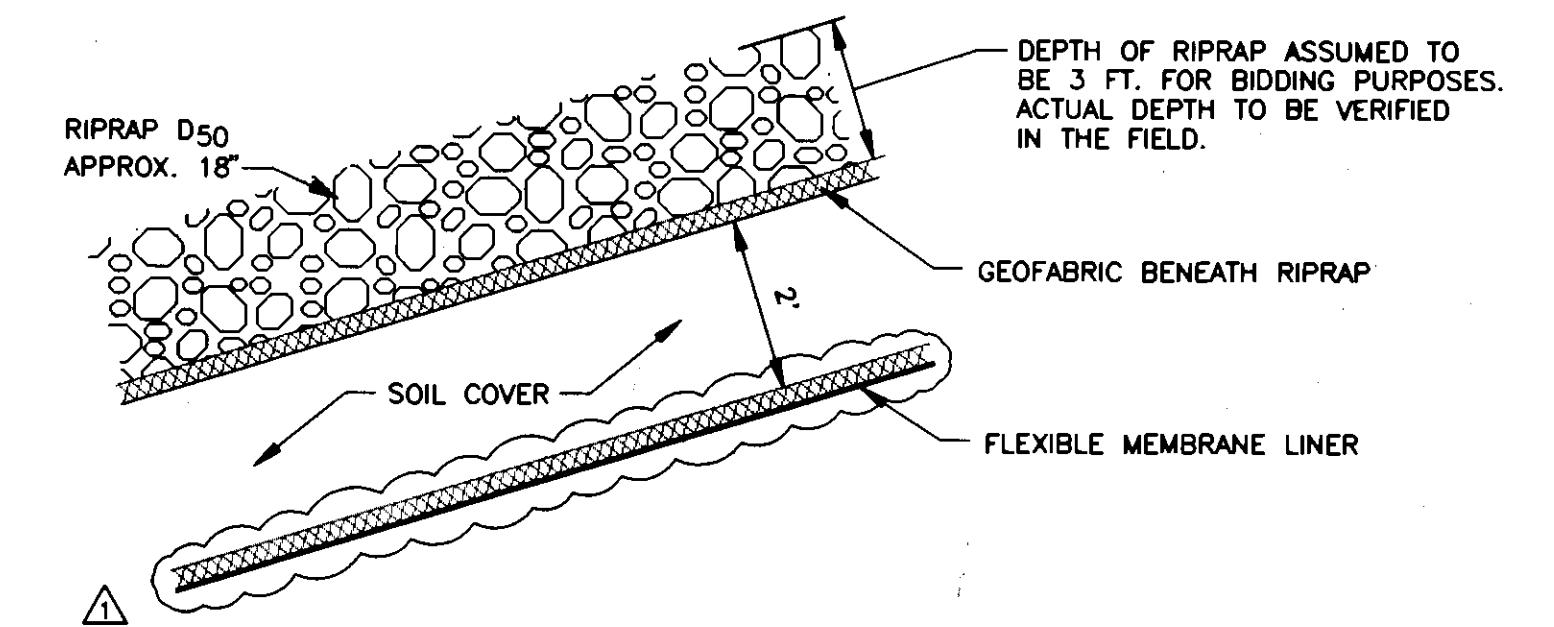
VALVE CHAMBER PLAN VIEW

NOT TO SCALE



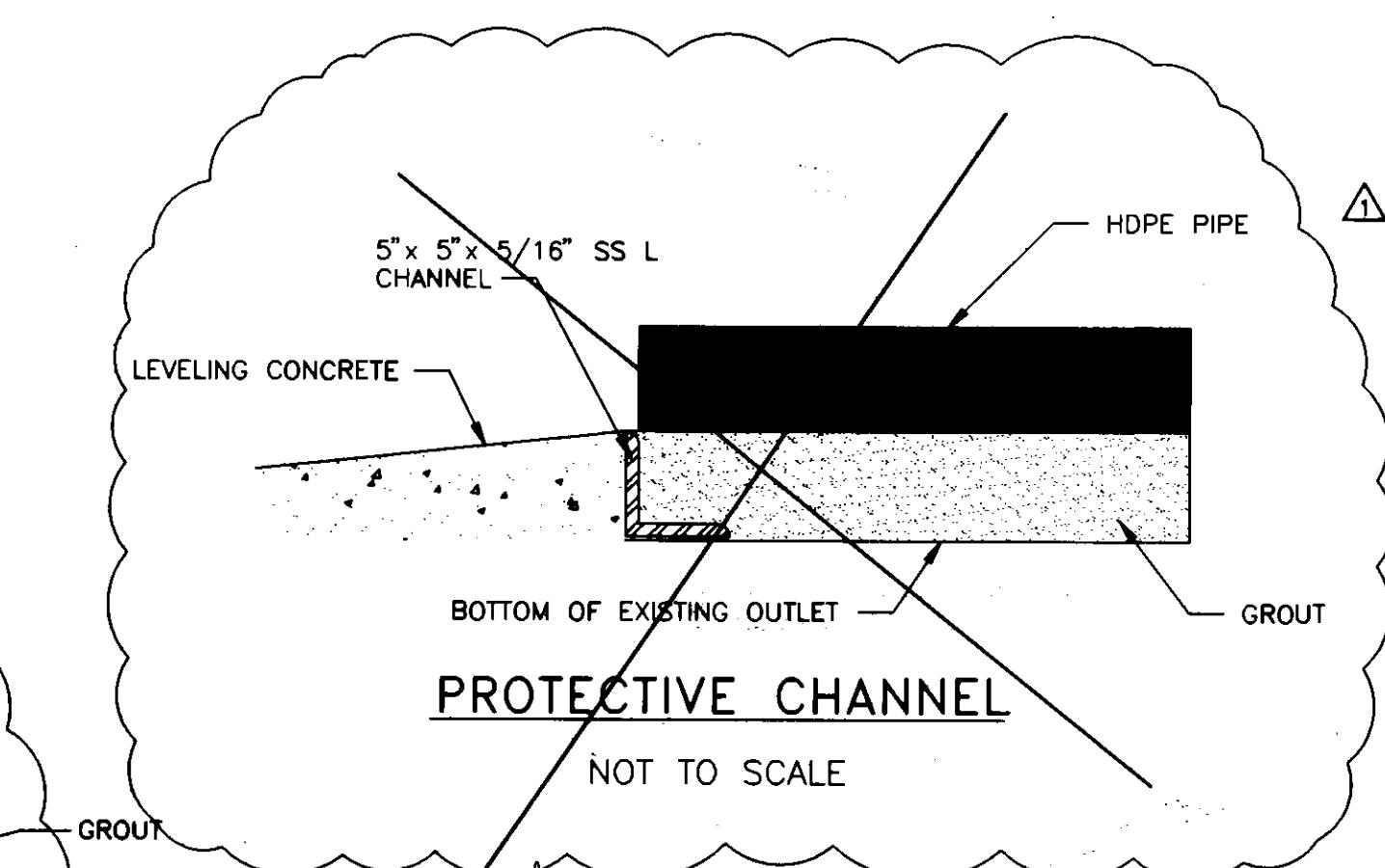
VALVE CHAMBER PROFILE

NOT TO SCALE



TYPICAL RIPRAP DETAIL

NOT TO SCALE



PROTECTIVE CHANNEL

NOT TO SCALE

NOTES:

1. CONTRACTOR SHALL SUBMIT PLANS THAT SHOW HOW THE CONDUIT WILL BE CENTERED, DISCUSS HOW THE CONDUIT WILL BE PROTECTED FROM COLLAPSE, BULK HEAD DETAILS, GROUT PORT AND SNORKEL DETAILS, GROUT RETURN TUBES, HOW THE GROUT PRESSURE WILL BE MEASURED, AND AN ESTIMATE OF THE TOTAL GROUT TAKE.
2. ALL DIMENSIONS SHOWN ARE APPROXIMATE AND SHOULD BE VERIFIED BY THE CONTRACTOR IN THE FIELD.

RECORD DRAWING

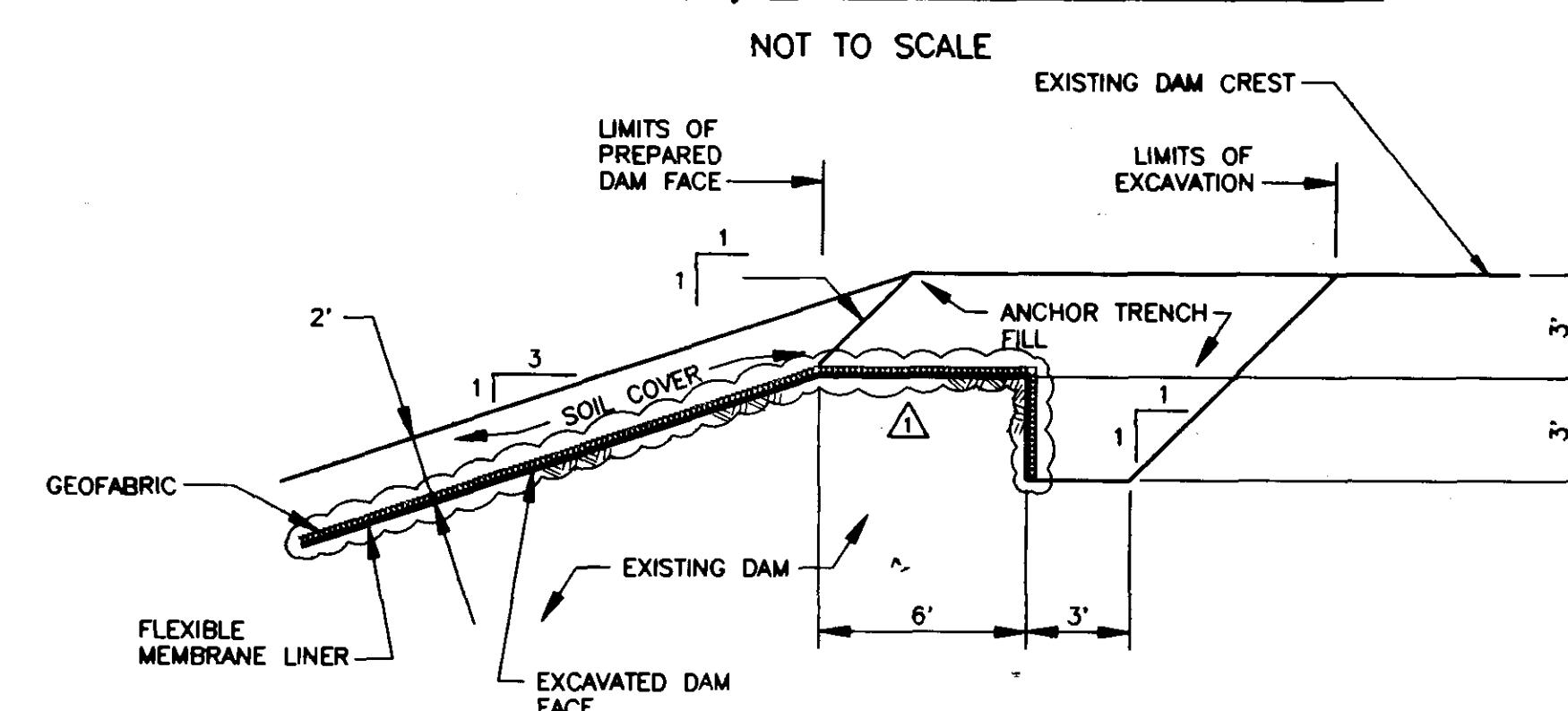
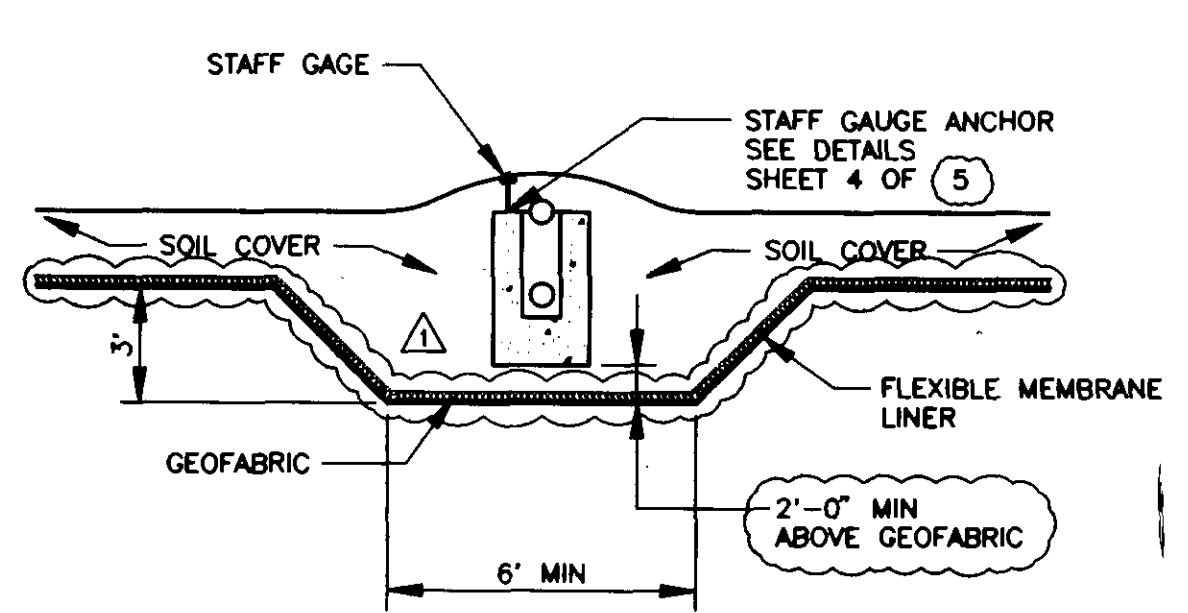
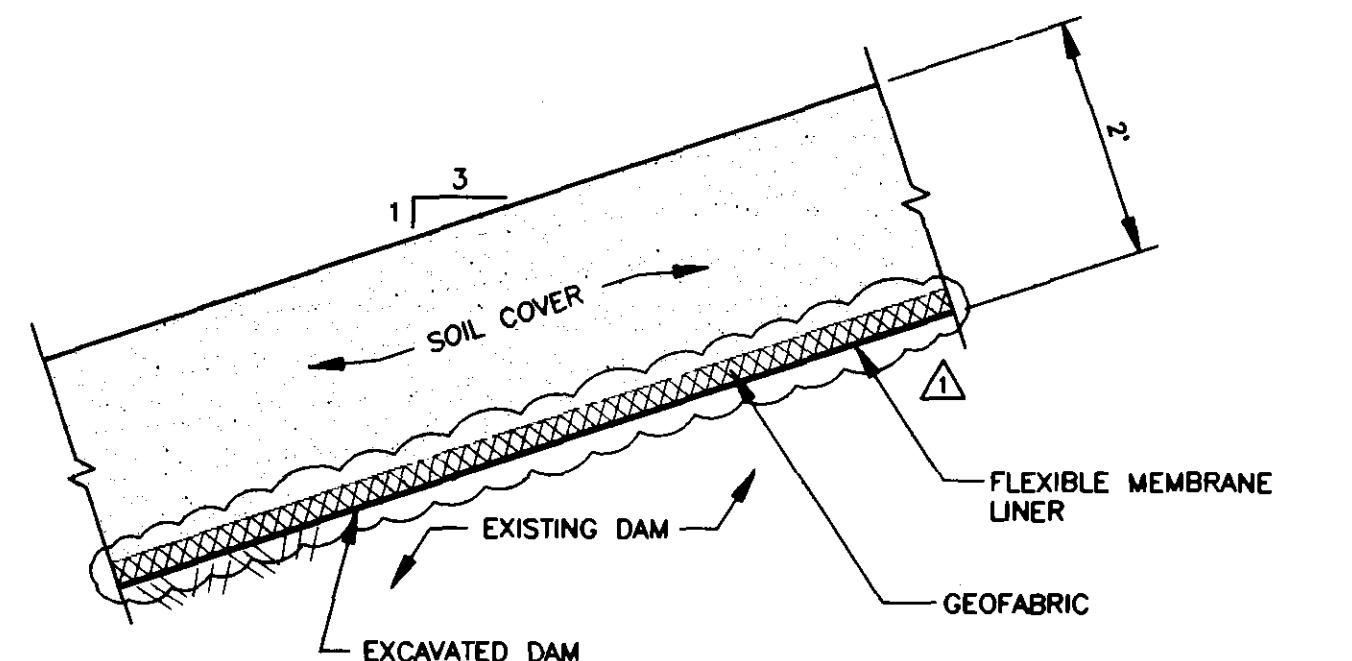
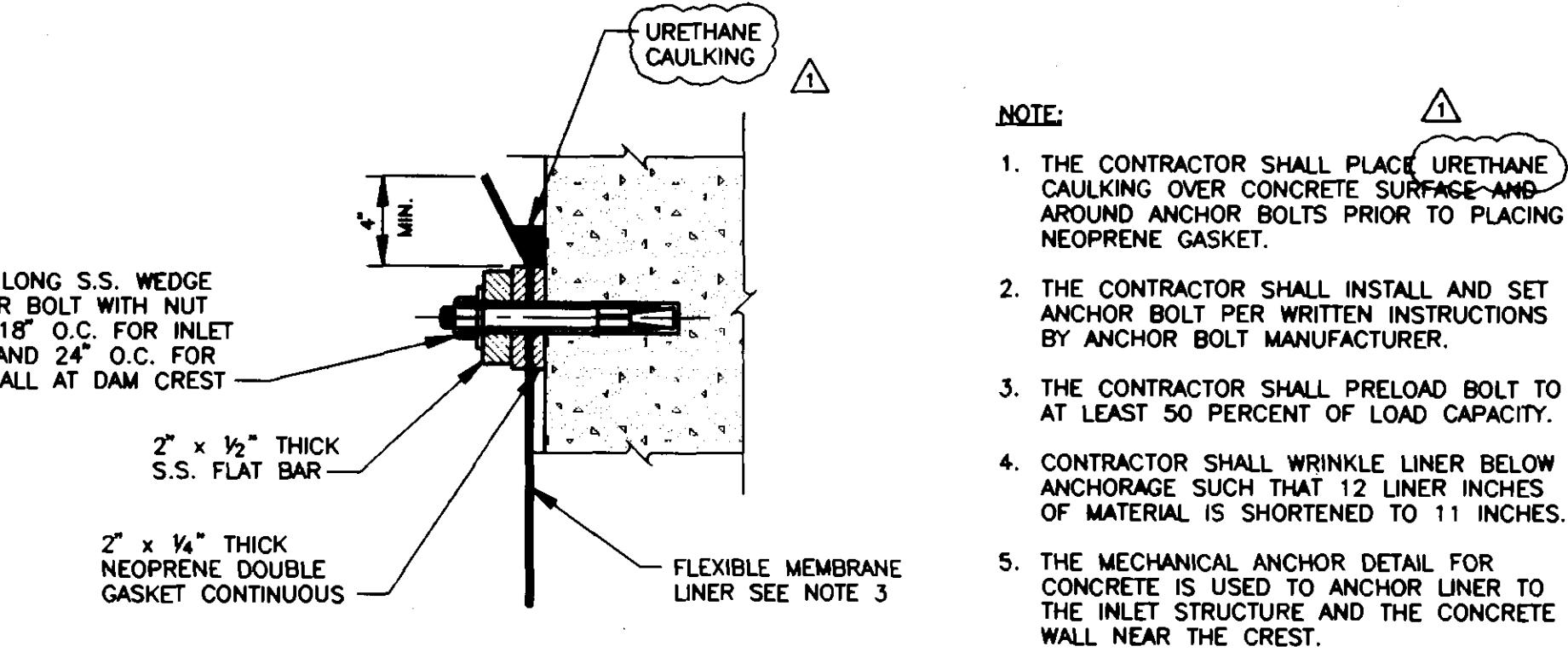
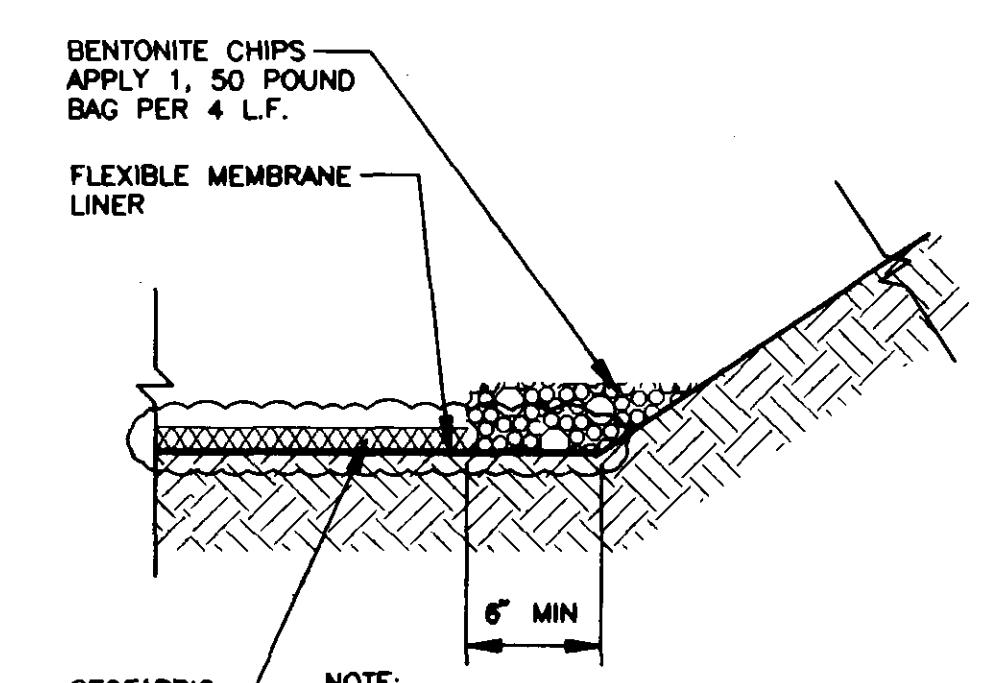
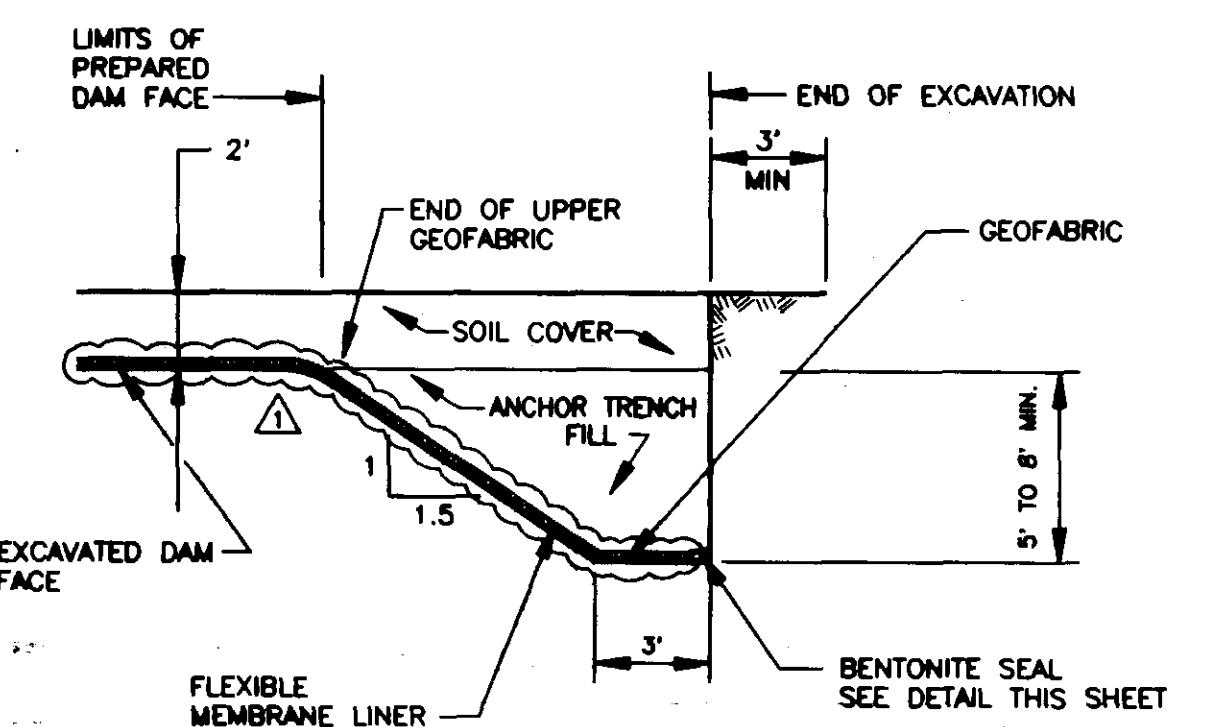
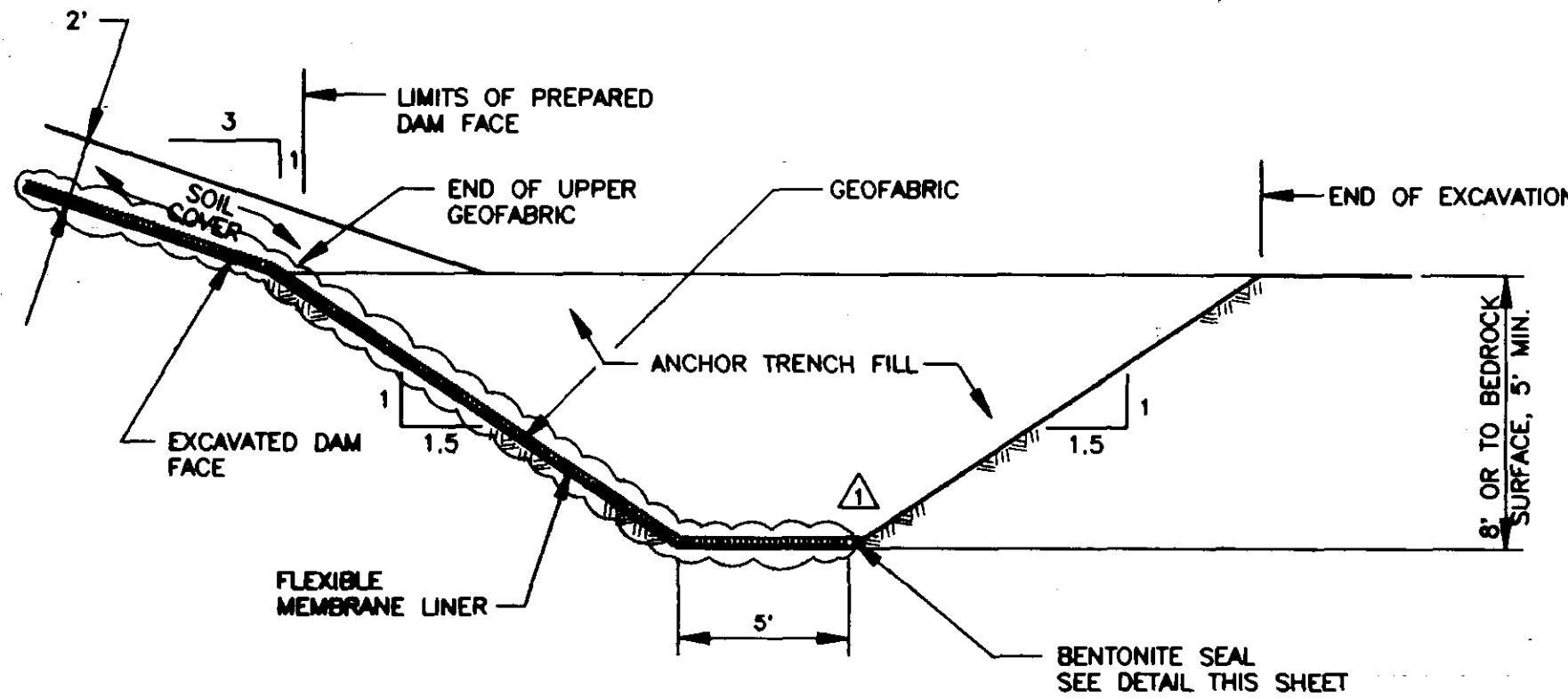
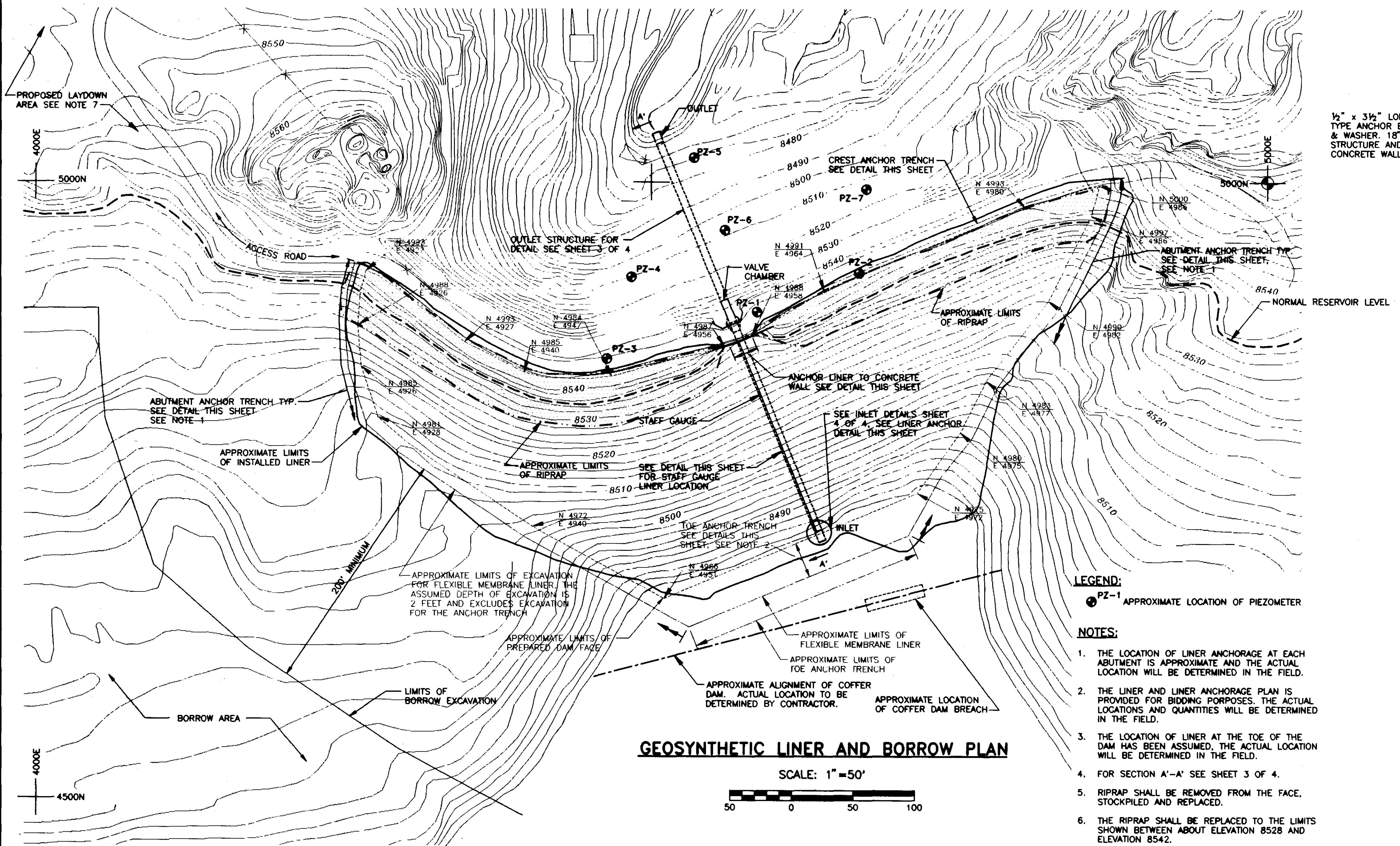
| DRAWN BY: GOJ/CNH/JAF | | DATE: 2/15/94 | CHECKED BY: C.N.H. | DATE: 5/1/95 |
|---------------------------------------|---|---------------|--------------------|--------------|
| AS CONSTRUCTED - CORRECTIONS COMPLETE | | | | |
| BY C.N.H. DATE 12/28/95 | | | | |
| REVISIONS | | | | |
| NO. | DESCRIPTION | DATE | BY | |
| △ | REVISE SECTION AND DETAILS TO SHOW CONTINUOUS OUTLET AND REMOVAL OF GATES | 12/28/95 | C.N.H. | |
| | | | | |
| | | | | |
| | | | | |

OUTLET AND VALVE CHAMBER PROFILE, SECTIONS, AND DETAILS

Woodward-Clyde Consultants

Consulting Engineers Denver, Colorado

WORSTER DAM AND OUTLET REHABILITATION PROJECT NO. 23172-23112



| | | | |
|---|--|----------|--------|
| GENERAL NOTE: | | | |
| 1. AT THE CONTRACTOR'S OPTION, (2) FEET OF MATERIAL SHALL BE REMOVED FROM THE DAM FACE OR THE FACE MAY BE PREPARED AND (2) FEET OF FILL FROM THE BORROW AREA MAY BE PLACED OVER THE LINER. SEE THE SPECIFICATION FOR SPECIFIC REQUIREMENTS FOR PREPARATION OF DAM FACE AND FILL OVER THE LINER. | | | |
| CONTRACTOR REMOVED 2 FEET OF MATERIAL FROM THE DAM FACE AND PLACED OVER THE LINER. | | | |
| DRAWN BY: C.H.JAF DATE: 2/15/94 CHECKED BY: C.N.H. DATE: 5/1/95 | | | |
| AS CONSTRUCTED - CORRECTIONS COMPLETE BY: C.N.H. DATE: 12/28/95 | | | |
| REVISIONS | | | |
| NO. | DESCRIPTION | DATE | BY |
| △ | REVISE LINER AND GEOPOLYMER INTERFACE AND LOCATION OF LINER. REVISE CAULKING | 12/28/95 | C.N.H. |
| WORSTER DAM AND OUTLET REHABILITATION PROJECT NO. 23172-23112 DRAWING NO. OF | | | |
| 2 5 | | | |

RECORD DRAWING

FLEXIBLE MEMBRANE LINER AND BORROW AREA PLAN VIEW AND LINER ANCHOR DETAILS

Woodward-Clyde Consultants Consulting Engineers Denver, Colorado

3172-2

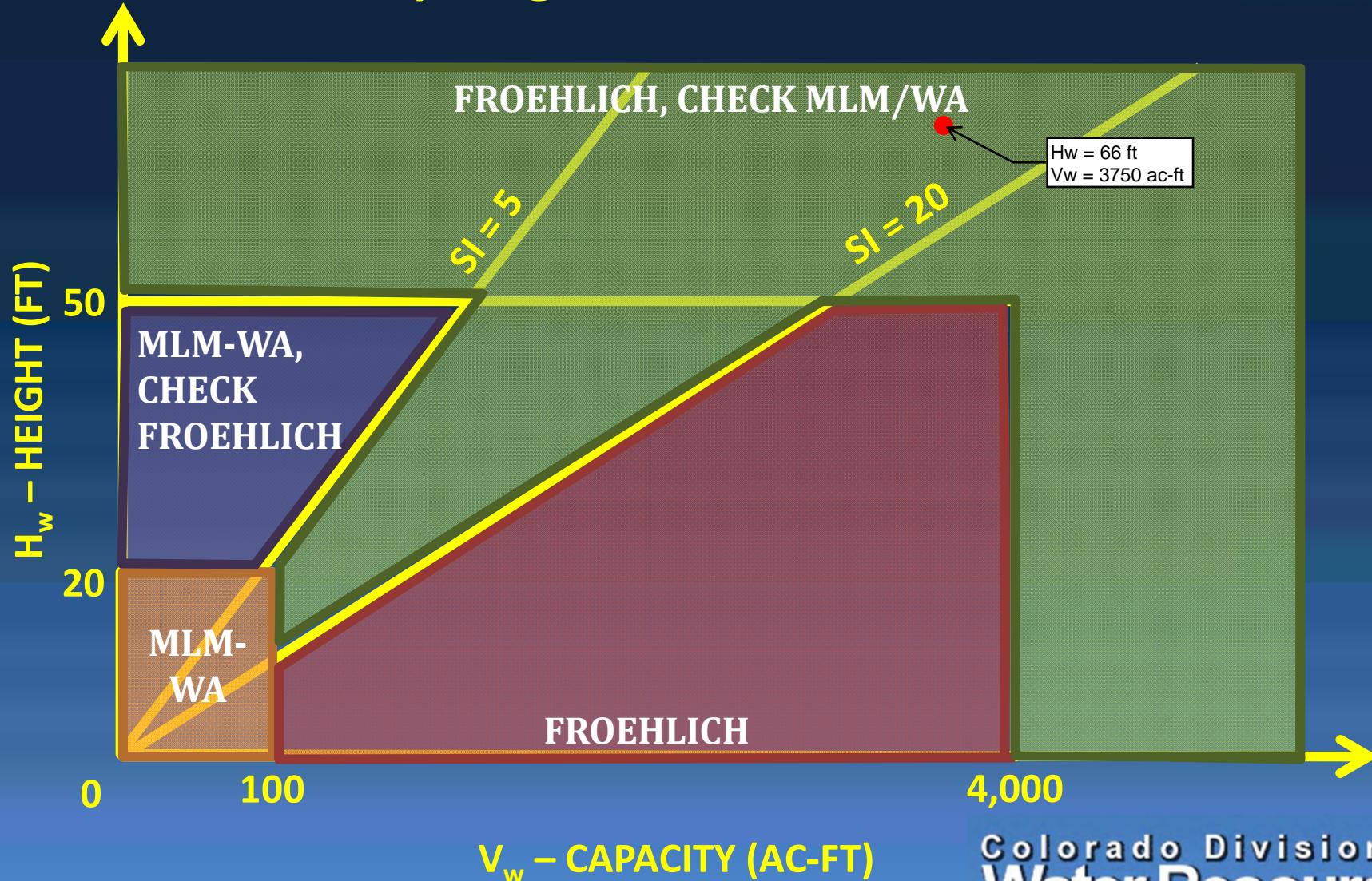
APPENDIX B

DAM BREACH ANALYSIS SPREADSHEETS

Empirical Method Guidance

Piping Failure Mode

$$SI = \frac{V_w}{H_w}$$



Adapted from Colorado Rules and Regulations for Dam Safety and Dam Construction, 2007

ESTIMATION OF DAM BREACH PARAMETERS
USING THE FROEHLICH 2008 METHOD

PROJECT: Worster Dam (Eaton Reservoir), DamID 030401

BREACH INPUT PARAMETERS:

Select Failure Mode From Drop-Down Menu: **PIPING**

| | | |
|---|---------|---|
| Height of water over base elevation of breach (H_w) = | 66.0 | Feet |
| Volume of water in the reservoir at the time of failure (V_w) = | 3,862.0 | Acre-Feet |
| Reservoir Surface Area at H_w (A_s) = | 137.5 | Acres |
| Height of breach (H_b) = | 72.0 | Feet |
| Failure Mode Factor (K_f) = | 1 | |
| Breach Side-Slope Ratio (Z_b) = | 0.7 | $Z(H):1(V)$ |
| Dam Size Class: | Large | Assumes Full Reservoir At Time of Breach. |

CALCULATED BREACH CHARACTERISTICS:

| | | |
|--------------------------------------|--------|-----------------------|
| Average Breach Width (B_{avg}) = | 137.4 | Feet |
| Bottom Width of Breach (B_b) = | 87.0 | Feet |
| Breach Formation Time (T_f) = | 0.56 | Hours |
| Storage Intensity (SI) = | 58.5 | Acre Feet/Foot |
| Predicted Peak Flow (Q_p) = | 134381 | Cubic Feet per Second |

RESULTS CHECK:

| | | |
|--|-------|---|
| Average Breach Width Divided by Height of Breach (B_{avg}/H_b) = | 1.91 | If $(B_{avg}/H_b) > 0.6$, Full Breach Development is Anticipated |
| Erosion Rate (ER), Calculated as (B_{avg}/T_f) = | 246.5 | |
| Erosion Rate Divided by Height of Water Over Base of Breach (ER/H_w) = | 3.7 | If $1.6 < (ER/H_w) < 21$, Erosion Rate is Assumed Reasonable |

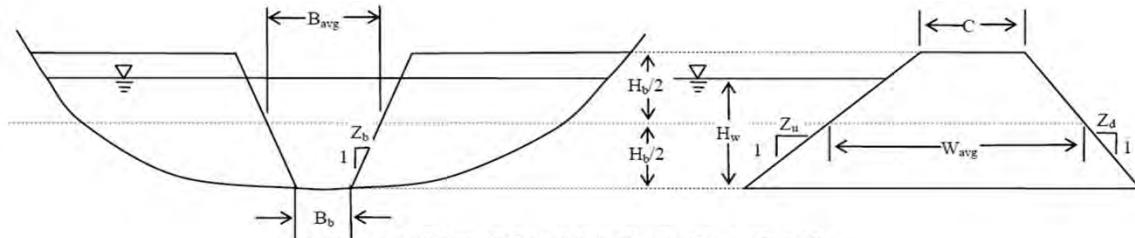


Figure 1- Breach Variable Definition Sketch

ESTIMATION OF DAM BREACH PARAMETERS
USING THE MACDONALD & LANGRIDGE-MONOPOLIS OR WASHINGTON STATE METHODS
WITH ALL FAILURE TIMES ESTIMATED BY WASHINGTON STATE METHOD

PROJECT: [Worster Dam \(Eaton Reservoir\), DamID 030401](#)

BREACH INPUT PARAMETERS:

Select Embankment Type From Drop-Down Menu: **EARTHEN (NON-COHESIVE)**

| | | |
|---|---------------|--|
| Height of water over base elevation of breach (H_w) = | 66.0 | Feet |
| Volume of water stored in reservoir at time of failure (V_w) = | 3862.0 | Acre-Feet |
| Reservoir Surface Area at H_w (S_a) = | 137.5 | Acres |
| Crest width of dam (C) = | 12.0 | Feet |
| Height of breach from dam crest to base elevation of breach (H_b) = | 72.0 | Feet |
| Slope of upstream dam face (Z_u) = | 1.5 | $Z(H):1(V)$ |
| Slope of downstream dam face (Z_d) = | 2.0 | $Z(H):1(V)$ |
| Breach side-slope ratio (Z_b) = | 2.0 | $Z(H):1(V)$ |
| Piping Orifice Coefficient (C_p) = | 0.68 | Used To Calculate Peak Discharge Through Piping Hole |
| Dam Size Class: | Large | Assumes Full Reservoir At Time of Breach |

CALCULATED BREACH CHARACTERISTICS:

| | | |
|---|----------------|-----------------------------|
| Breach Formation Factor (BFF) = | 254892 | |
| Embankment Volume Eroded (V_{er}) = | 54566.4 | Cubic Yards |
| Average Dam Width (W_{avg}) = | 138.0 | Feet (In Direction of Flow) |
| Average Breach Width (B_{avg}) = | 148.3 | Feet |
| Bottom Width of Breach (B_b) = | 4.3 | Feet |
| Breach Formation Time (T_f) = | 1.01 | Hours |
| Storage Intensity (SI) = | 58.5 | Acre Feet/Foot |
| Peak Breach Discharge (Q_p) = | 93805 | Cubic Feet per Second |

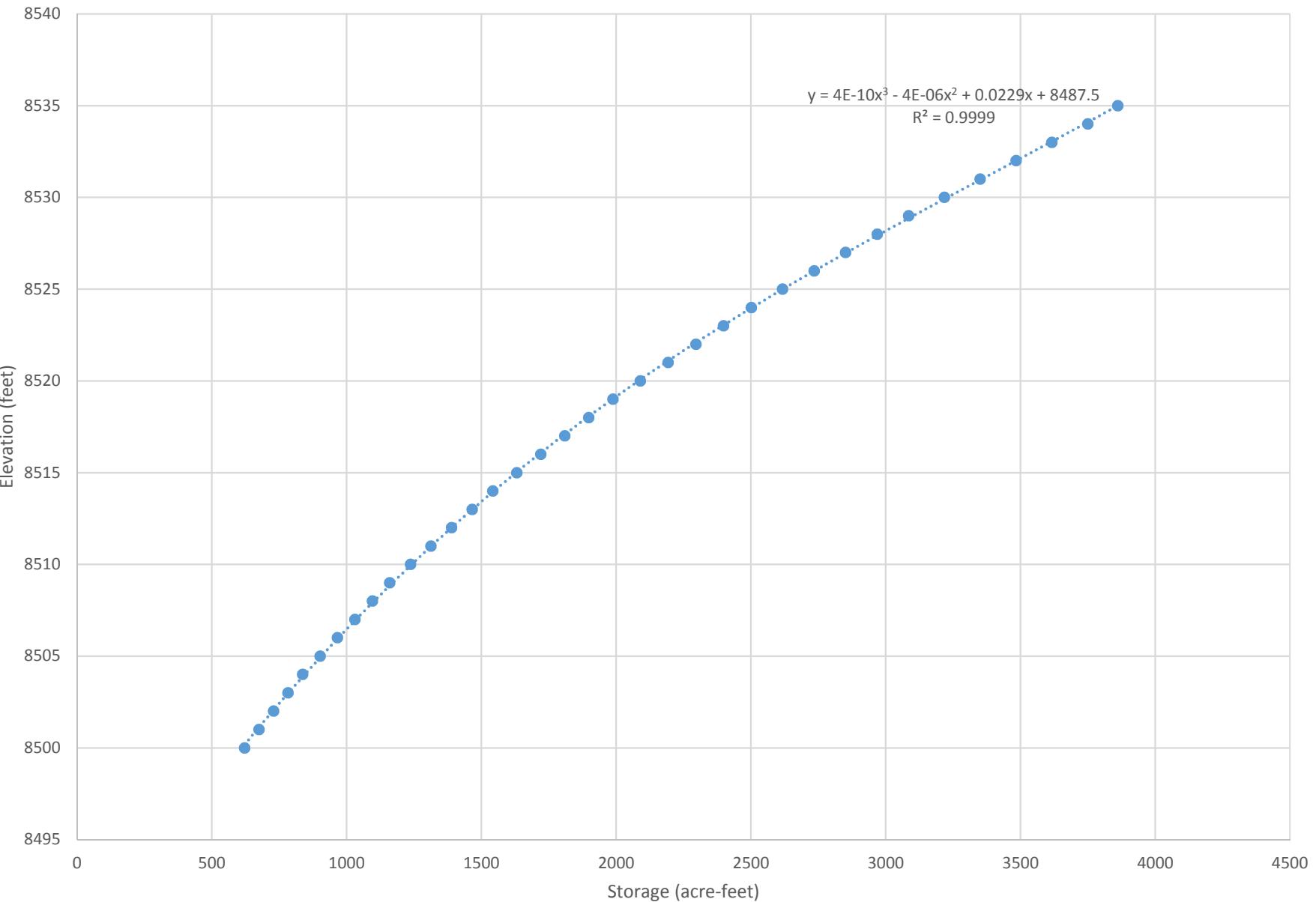
RESULTS CHECK:

| | | |
|--|--------------|---|
| Average Breach Width Divided by Height of Breach (B_{avg}/H_b) = | 2.06 | If $(B_{avg}/H_b) > 0.6$, Full Breach Development is Anticipated |
| Erosion Rate (ER), Calculated as (B_{avg}/T_f) = | 146.1 | |
| Erosion Rate Divided by Height of Water Over Base of Breach (ER/H_w) = | 2.2 | If $1.6 < (ER/H_w) < 21$, Erosion Rate is Assumed Reasonable |

APPENDIX C

STAGE-STORAGE INFORMATION FOR EATON RESERVOIR

Worster Dam Stage-Storage Curve



Eaton Reservoir Stage Storage

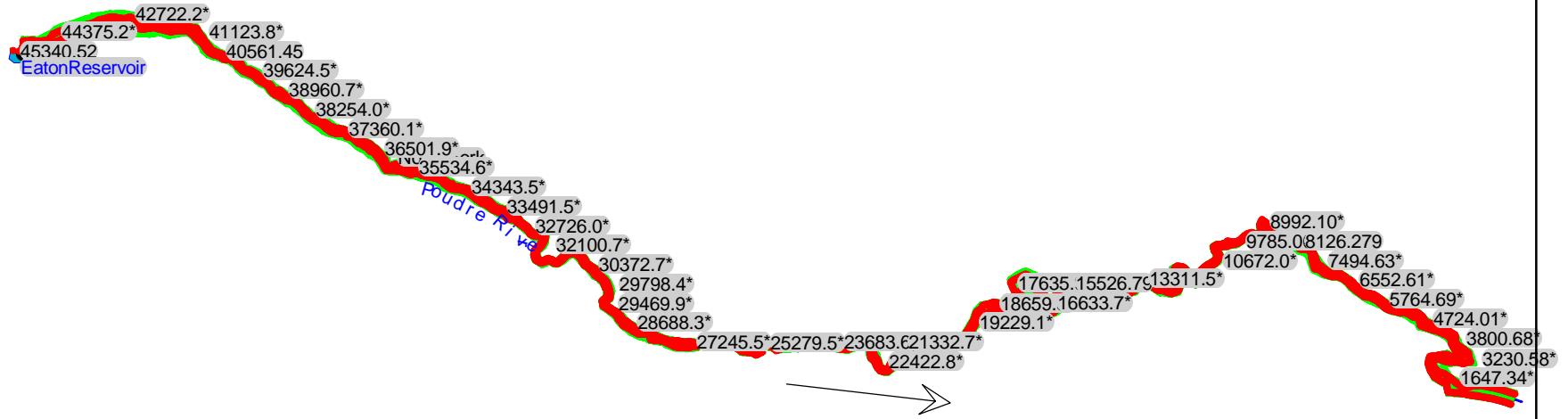
| Gage Height | Elevation | Storage | Surface Area |
|-------------|-----------|---------|--------------|
| 0 | 8469 | 0 | 0.0 |
| 5 | 8474 | 0 | 0.0 |
| 6 | 8475 | 2 | 4.0 |
| 7 | 8476 | 4 | 4.8 |
| 8 | 8477 | 6 | 5.6 |
| 9 | 8478 | 8 | 6.3 |
| 10 | 8479 | 12 | 7.1 |
| 11 | 8480 | 18 | 7.9 |
| 12 | 8481 | 25 | 8.7 |
| 13 | 8482 | 35 | 9.5 |
| 14 | 8483 | 47 | 10.2 |
| 15 | 8484 | 63 | 11.0 |
| 16 | 8485 | 81 | 11.8 |
| 17 | 8486 | 101 | 15.7 |
| 18 | 8487 | 124 | 19.5 |
| 19 | 8488 | 149 | 23.4 |
| 20 | 8489 | 175 | 27.2 |
| 21 | 8490 | 208 | 31.1 |
| 22 | 8491 | 242 | 35.0 |
| 23 | 8492 | 277 | 38.8 |
| 24 | 8493 | 313 | 42.7 |
| 25 | 8494 | 350 | 46.5 |
| 26 | 8495 | 392 | 50.4 |
| 27 | 8496 | 436 | 50.5 |
| 28 | 8497 | 480 | 50.6 |
| 29 | 8498 | 524 | 50.8 |
| 30 | 8499 | 569 | 50.9 |
| 31 | 8500 | 622 | 51.0 |
| 32 | 8501 | 675 | 51.1 |
| 33 | 8502 | 729 | 51.2 |
| 34 | 8503 | 783 | 51.4 |
| 35 | 8504 | 837 | 51.5 |
| 36 | 8505 | 902 | 51.6 |
| 37 | 8506 | 966 | 55.9 |
| 38 | 8507 | 1031 | 60.2 |
| 39 | 8508 | 1096 | 64.4 |
| 40 | 8509 | 1160 | 68.7 |
| 41 | 8510 | 1237 | 73.0 |
| 42 | 8511 | 1313 | 77.3 |
| 43 | 8512 | 1390 | 81.6 |
| 44 | 8513 | 1466 | 85.8 |
| 45 | 8514 | 1543 | 90.1 |
| 46 | 8515 | 1632 | 94.4 |
| 47 | 8516 | 1721 | 95.2 |

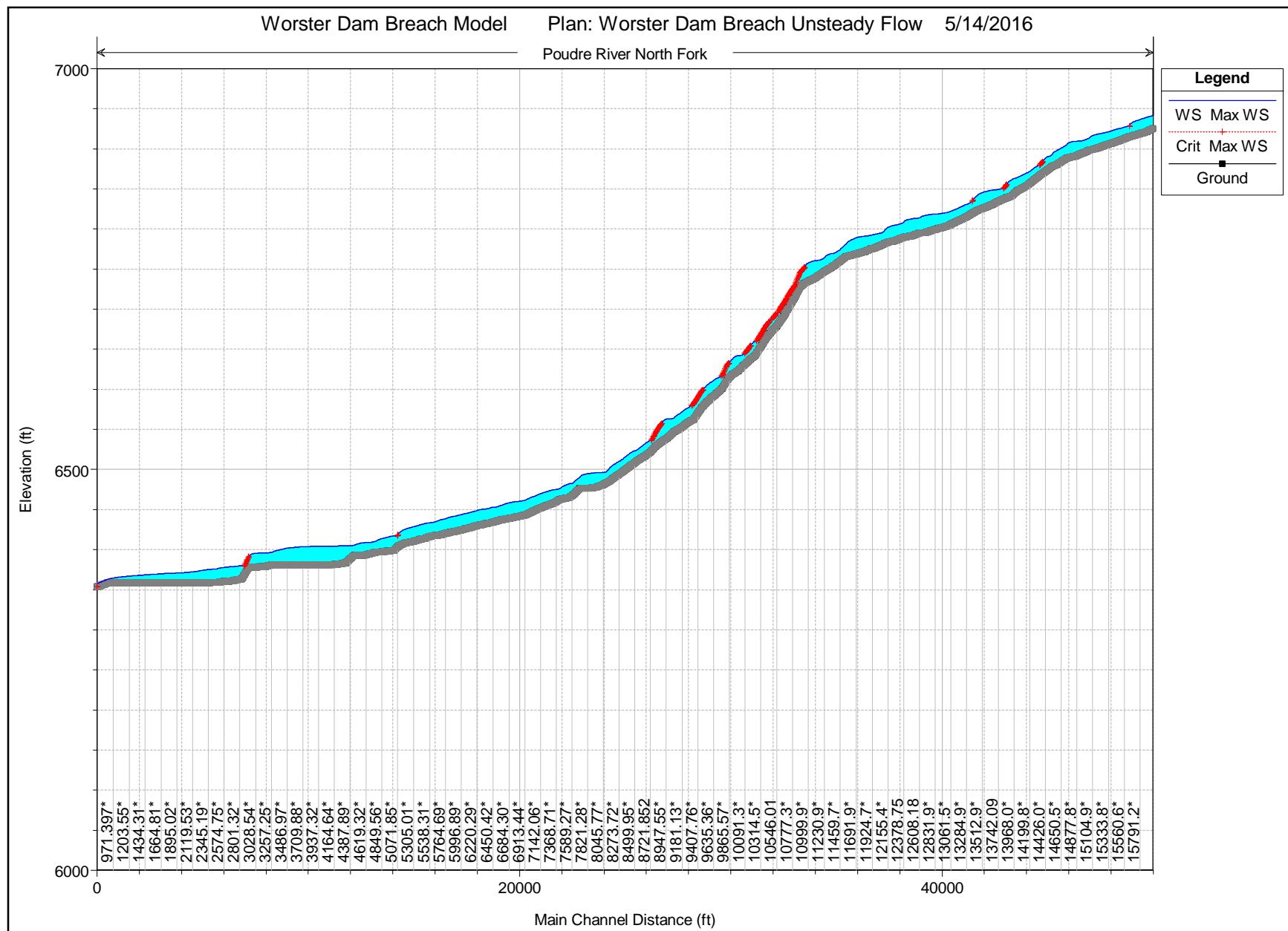
Eaton Reservoir Stage Storage

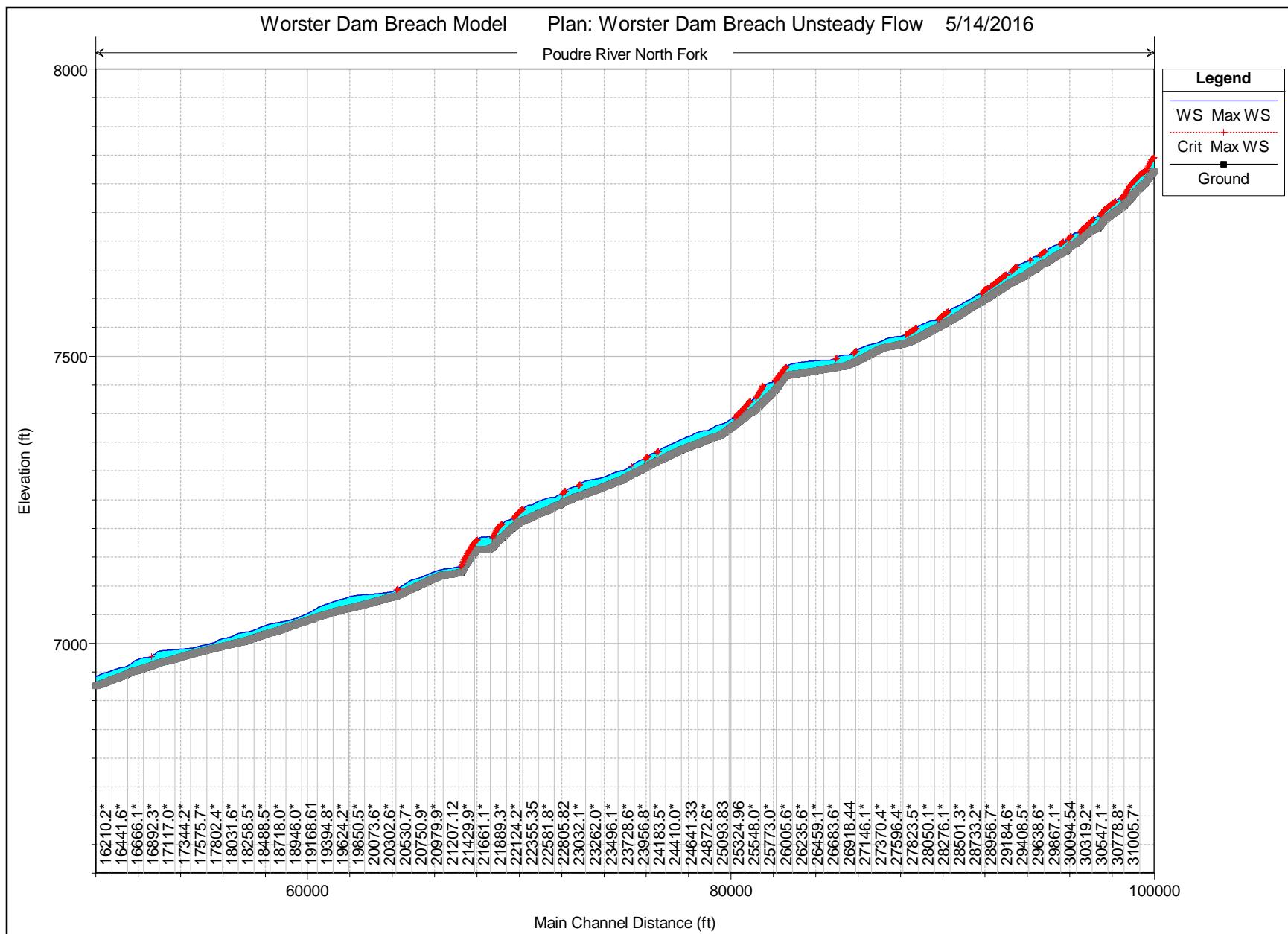
| Gage Height | Elevation | Storage | Surface Area |
|-------------|-----------|---------|--------------|
| 48 | 8517 | 1810 | 96.1 |
| 49 | 8518 | 1899 | 96.9 |
| 50 | 8519 | 1988 | 97.8 |
| 51 | 8520 | 2090 | 98.6 |
| 52 | 8521 | 2193 | 99.4 |
| 53 | 8522 | 2296 | 100.3 |
| 54 | 8523 | 2399 | 101.1 |
| 55 | 8524 | 2502 | 102.0 |
| 56 | 8525 | 2618 | 102.8 |
| 57 | 8526 | 2735 | 106.3 |
| 58 | 8527 | 2852 | 109.7 |
| 59 | 8528 | 2969 | 113.2 |
| 60 | 8529 | 3086 | 116.7 |
| 61 | 8530 | 3218 | 120.1 |
| 62 | 8531 | 3351 | 123.6 |
| 63 | 8532 | 3484 | 127.1 |
| 64 | 8533 | 3617 | 130.5 |
| 65 | 8534 | 3750 | 134.0 |
| 66 | 8535 | 3862 | 137.5 |
| 67 | 8536 | 3984 | 140.9 |
| 68 | 8537 | 4104 | 144.4 |
| 69 | 8538 | 4220 | 147.9 |
| 70 | 8539 | 4334 | 151.3 |
| 71 | 8540 | 4444 | 154.8 |
| 72 | 8541 | 4551 | 158.3 |

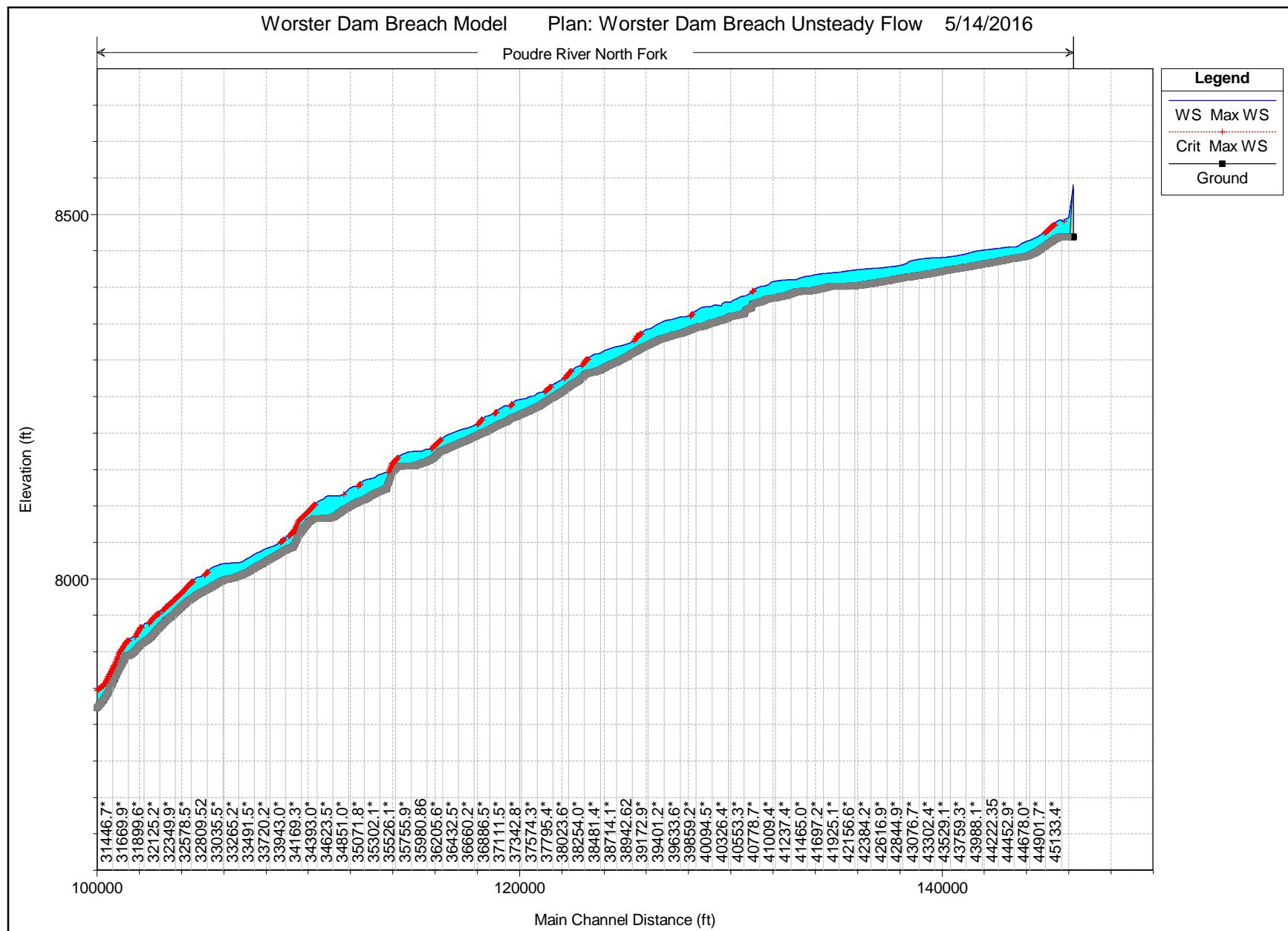
APPENDIX D

HEC-RAS MODELING RESULTS



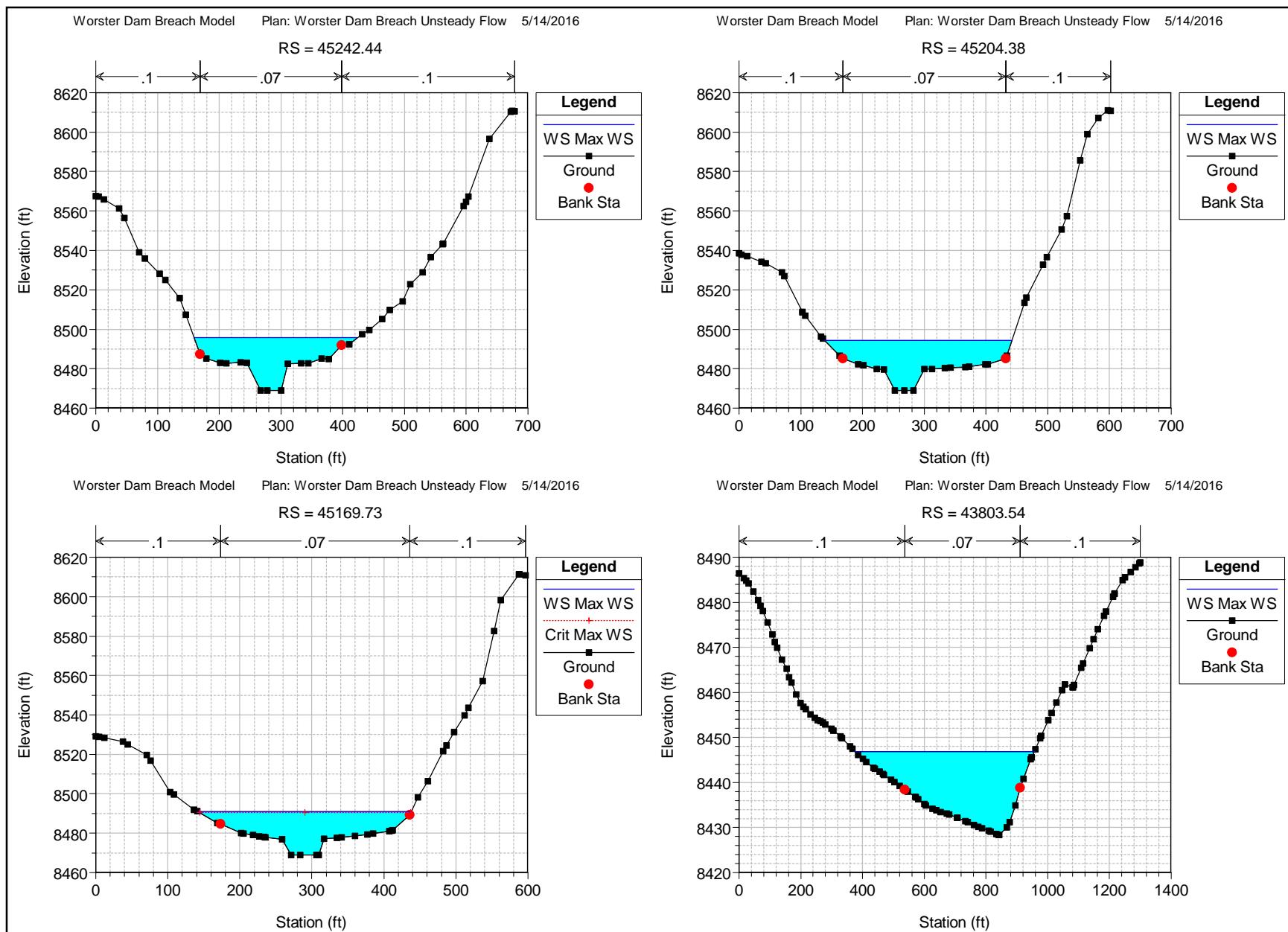


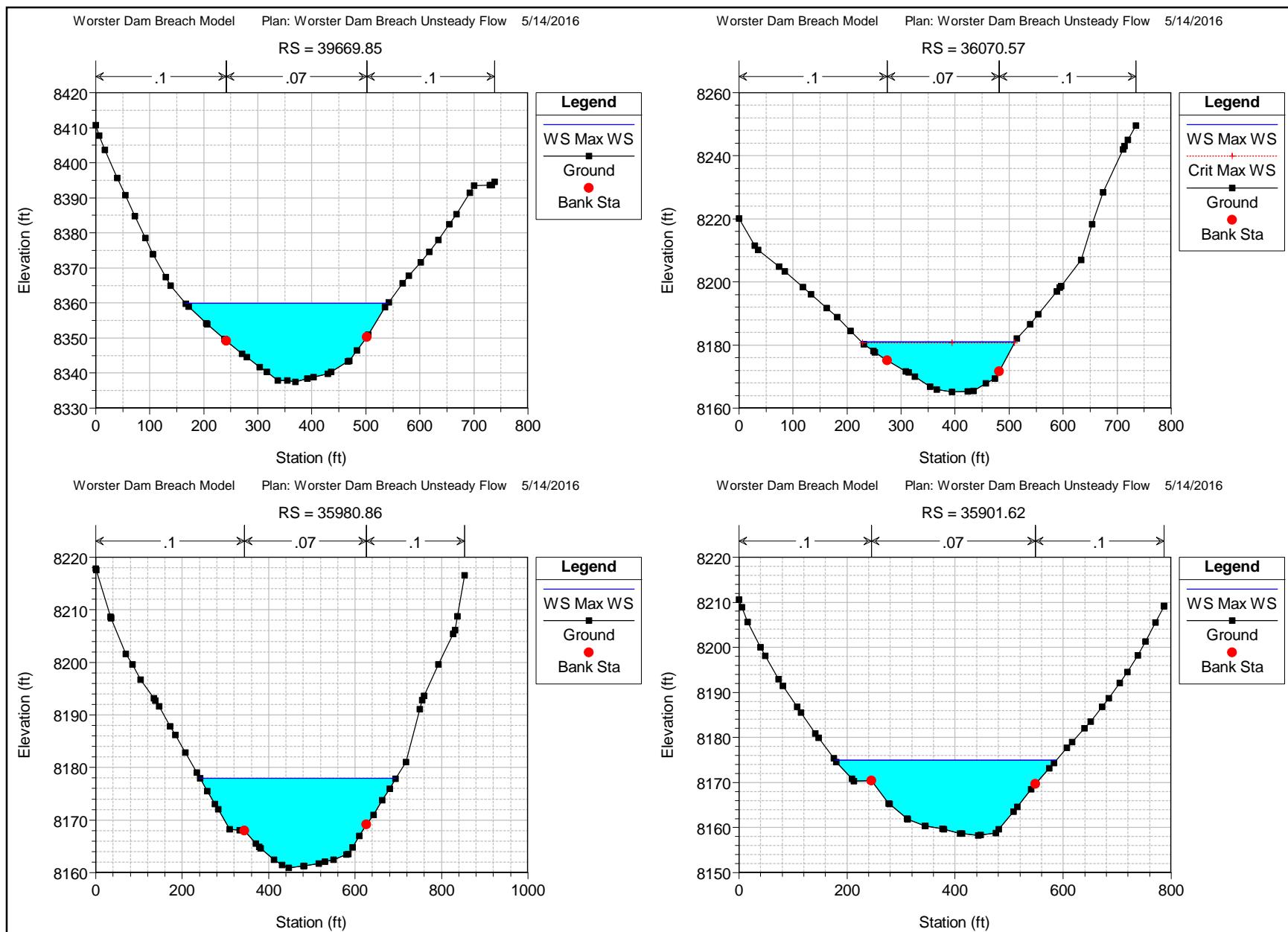


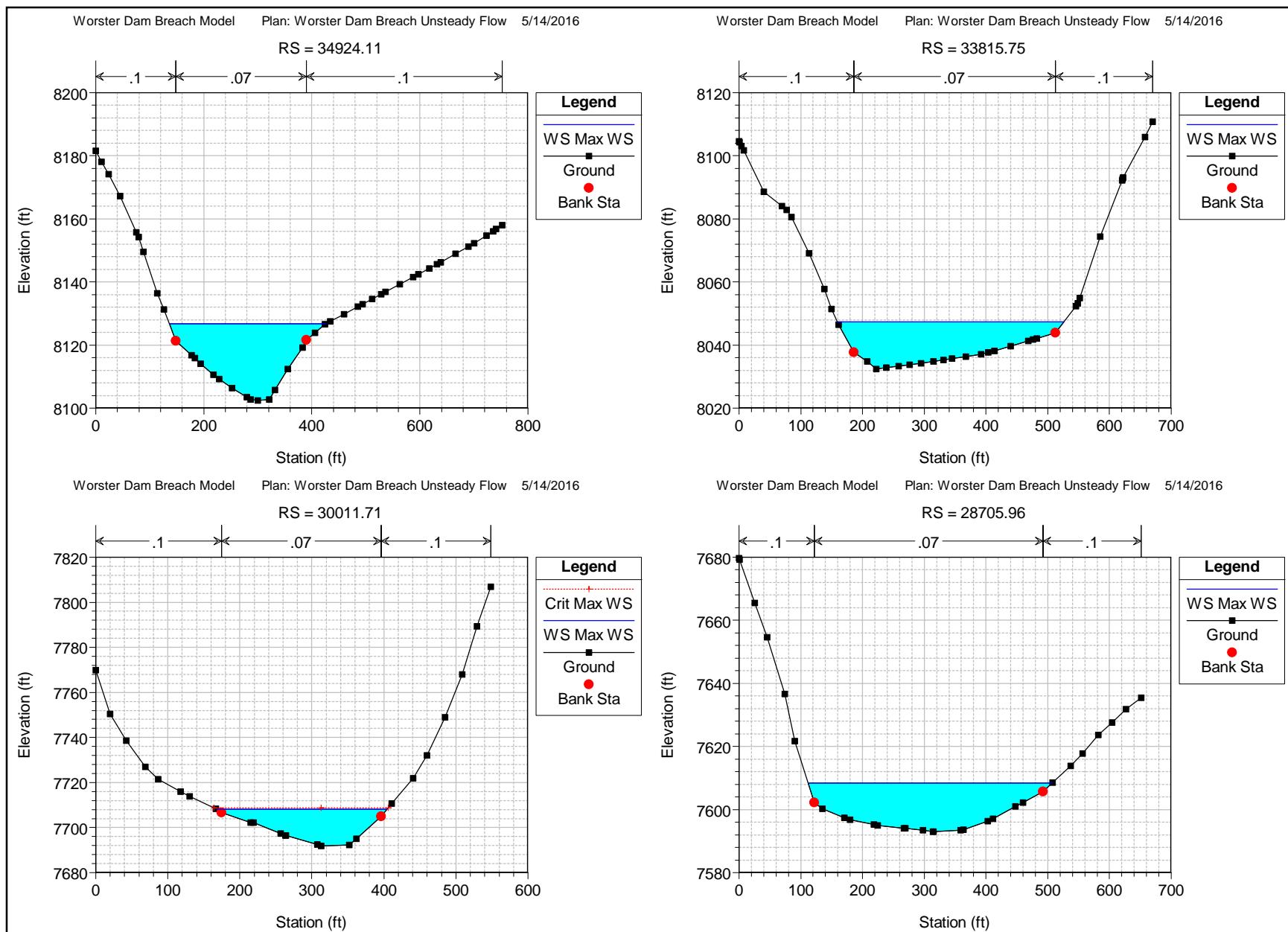


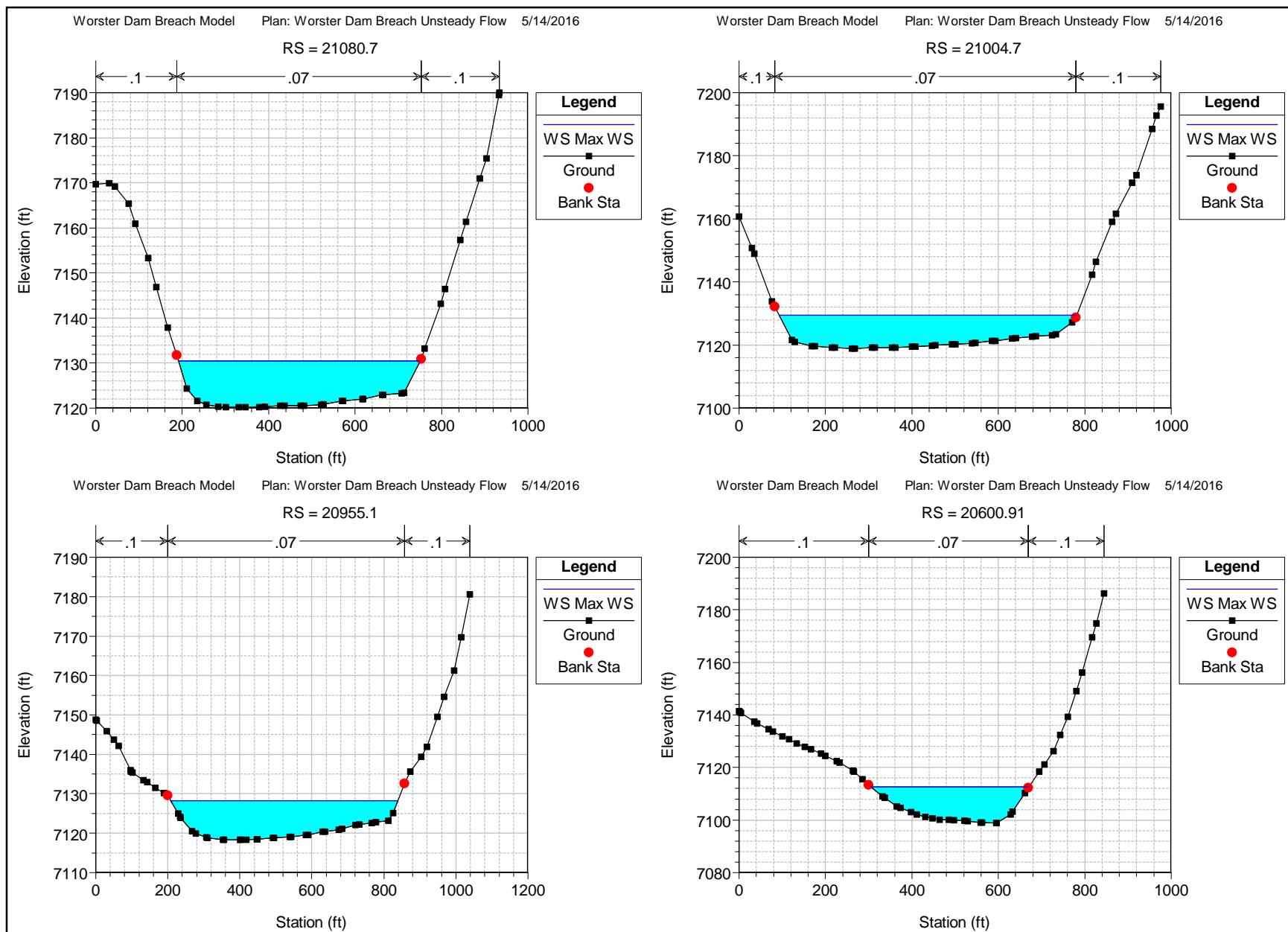
HEC-RAS Plan: WorsterUnstdy River: Poudre River Reach: North Fork Profile: Max WS (Continued)

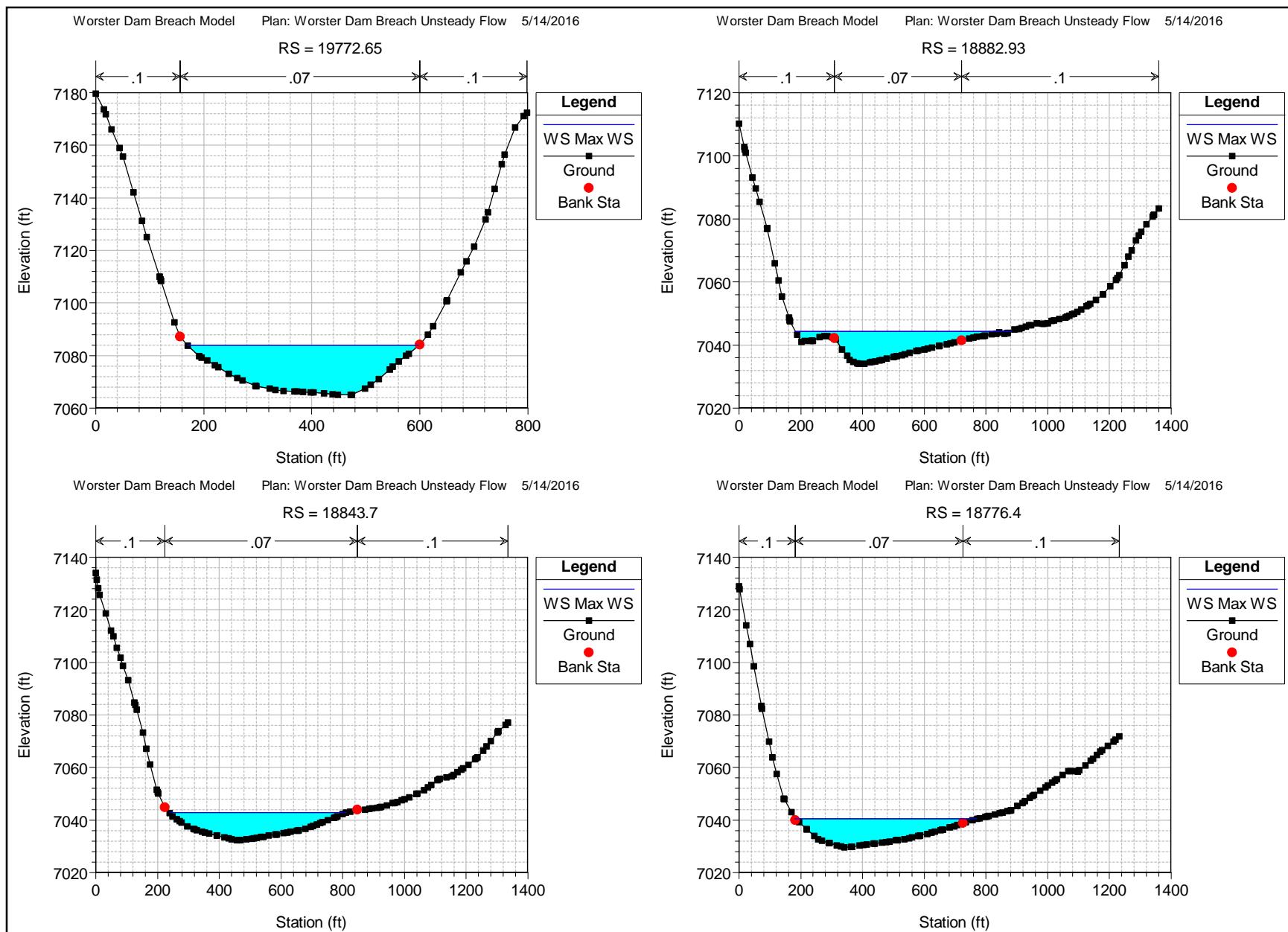
| Reach | River Sta | Profile | Q Total (cfs) | Min Ch El (ft) | W.S. Elev (ft) | Crit W.S. (ft) | E.G. Elev (ft) | E.G. Slope (ft/ft) | Vel Chnl (ft/s) | Flow Area (sq ft) | Top Width (ft) | Froude # Chl | Vel Total (ft/s) |
|------------|-----------|---------|------------------|-------------------|-------------------|-------------------|-------------------|-----------------------|--------------------|----------------------|-------------------|--------------|---------------------|
| North Fork | 892.8347 | Max WS | 15747.52 | 6358.63 | 6362.88 | | 6363.35 | 0.010104 | 5.47 | 2877.53 | 699.42 | 0.48 | 5.47 |
| North Fork | 792 | Max WS | 15746.80 | 6357.63 | 6361.88 | | 6362.34 | 0.010165 | 5.48 | 2872.18 | 699.38 | 0.48 | 5.48 |
| North Fork | 692 | Max WS | 15746.23 | 6356.63 | 6360.86 | | 6361.33 | 0.010280 | 5.50 | 2862.28 | 699.28 | 0.48 | 5.50 |
| North Fork | 592 | Max WS | 15745.21 | 6355.63 | 6359.82 | | 6360.30 | 0.010632 | 5.56 | 2832.92 | 698.99 | 0.49 | 5.56 |
| North Fork | 492 | Max WS | 15744.95 | 6354.63 | 6358.69 | | 6359.20 | 0.011867 | 5.75 | 2739.46 | 698.07 | 0.51 | 5.75 |
| North Fork | 392 | Max WS | 293.45 | 6353.63 | 6357.60 | 6353.82 | 6357.60 | 0.000004 | 0.11 | 2678.82 | 697.46 | 0.01 | 0.11 |

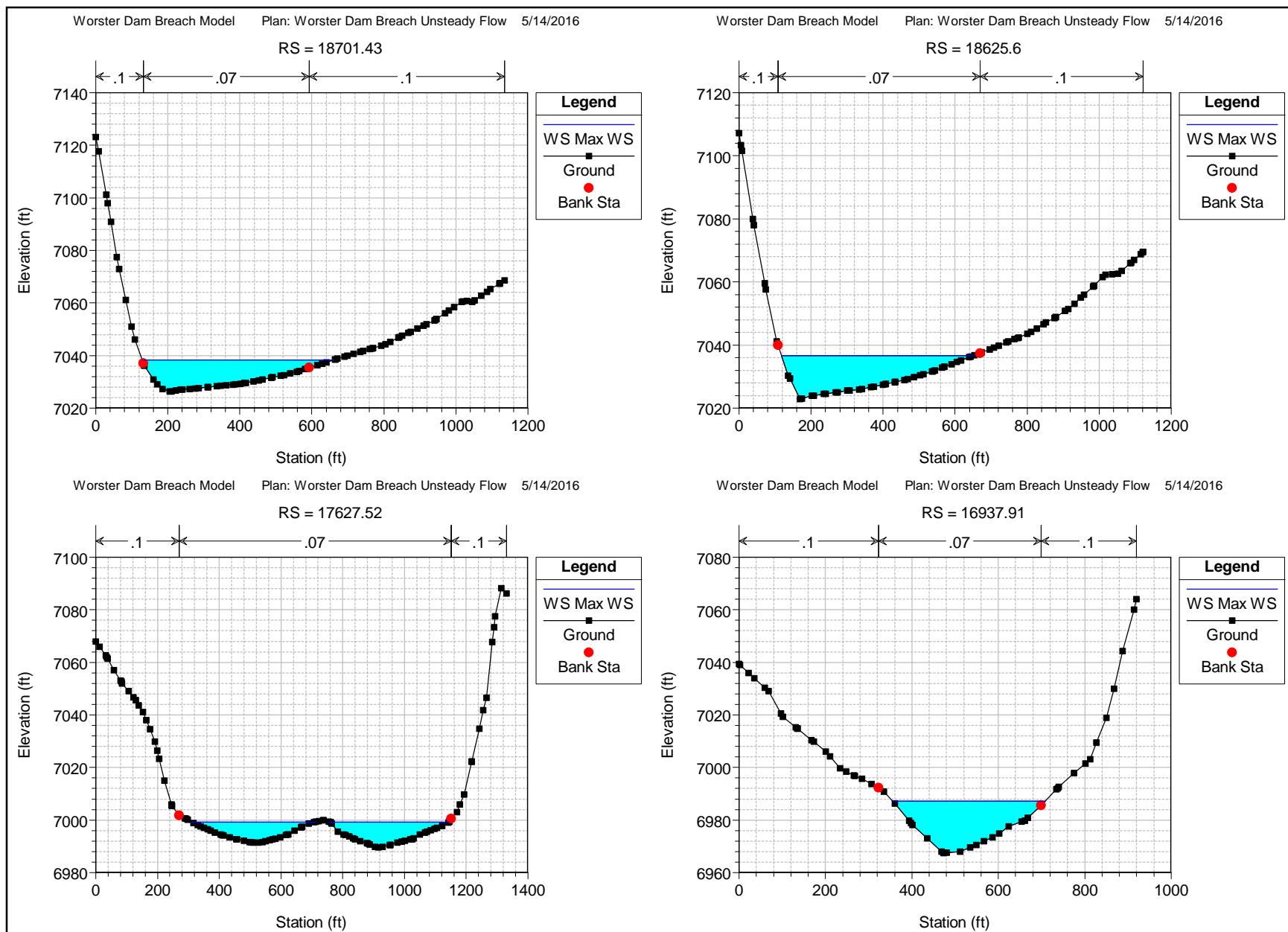


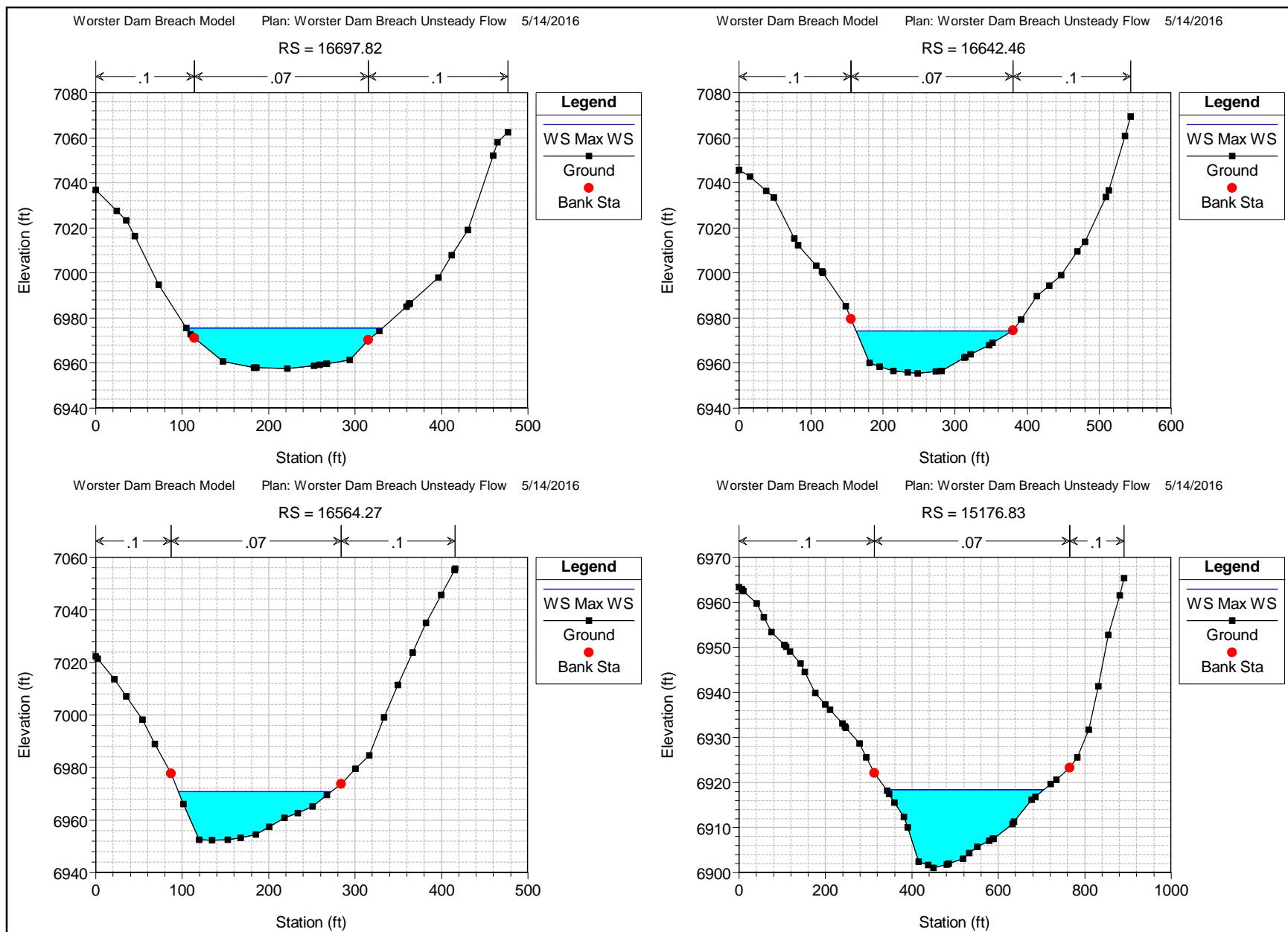








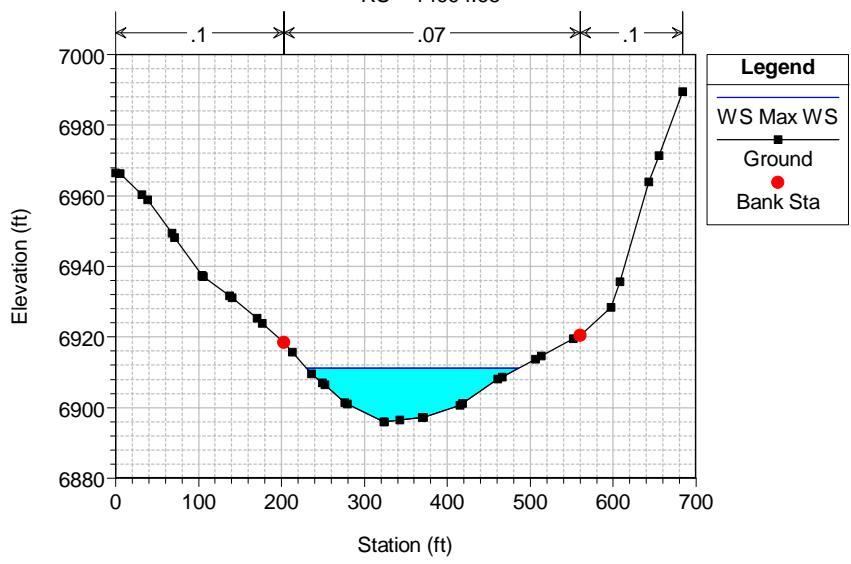


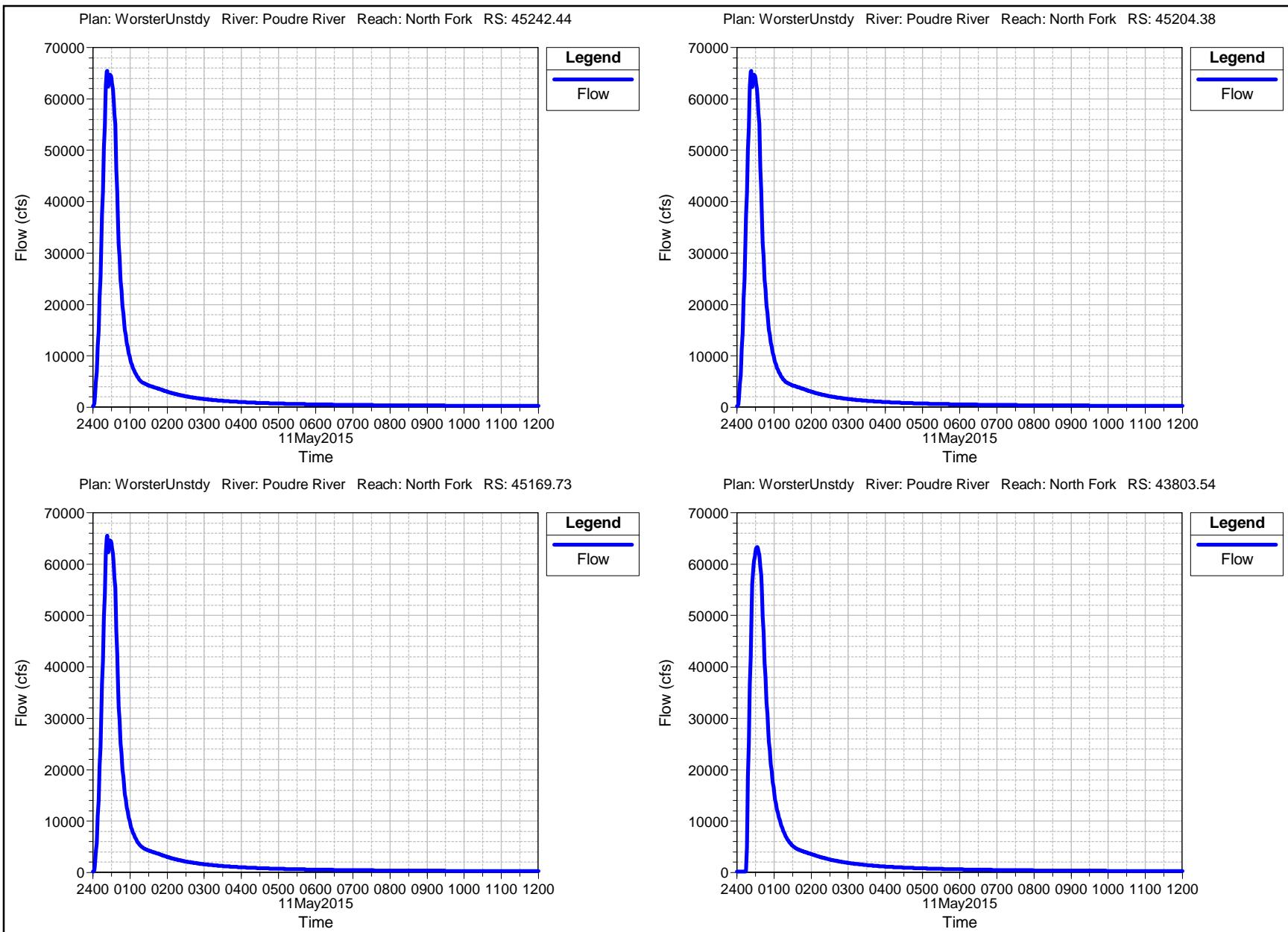


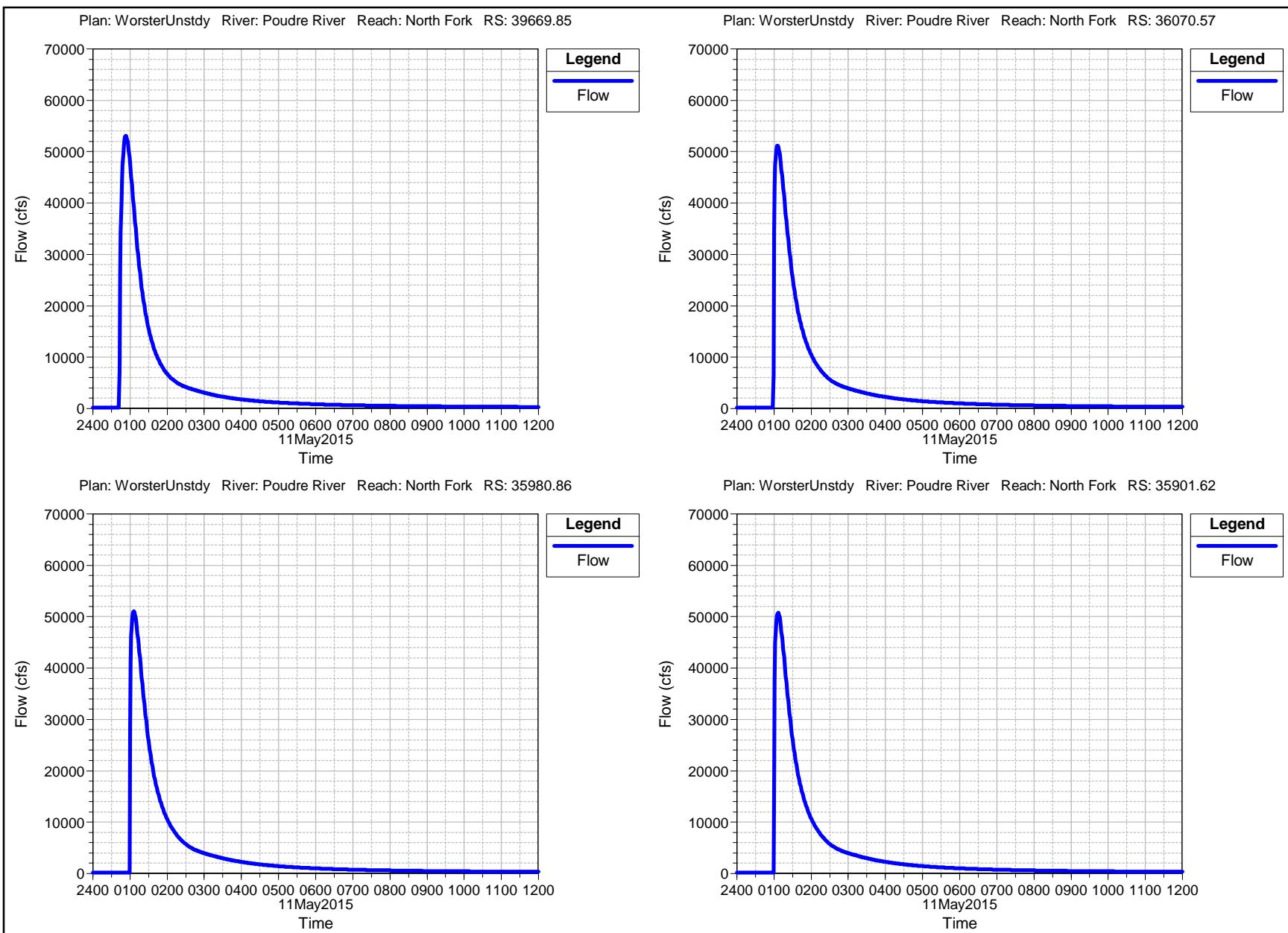
Worster Dam Breach Model

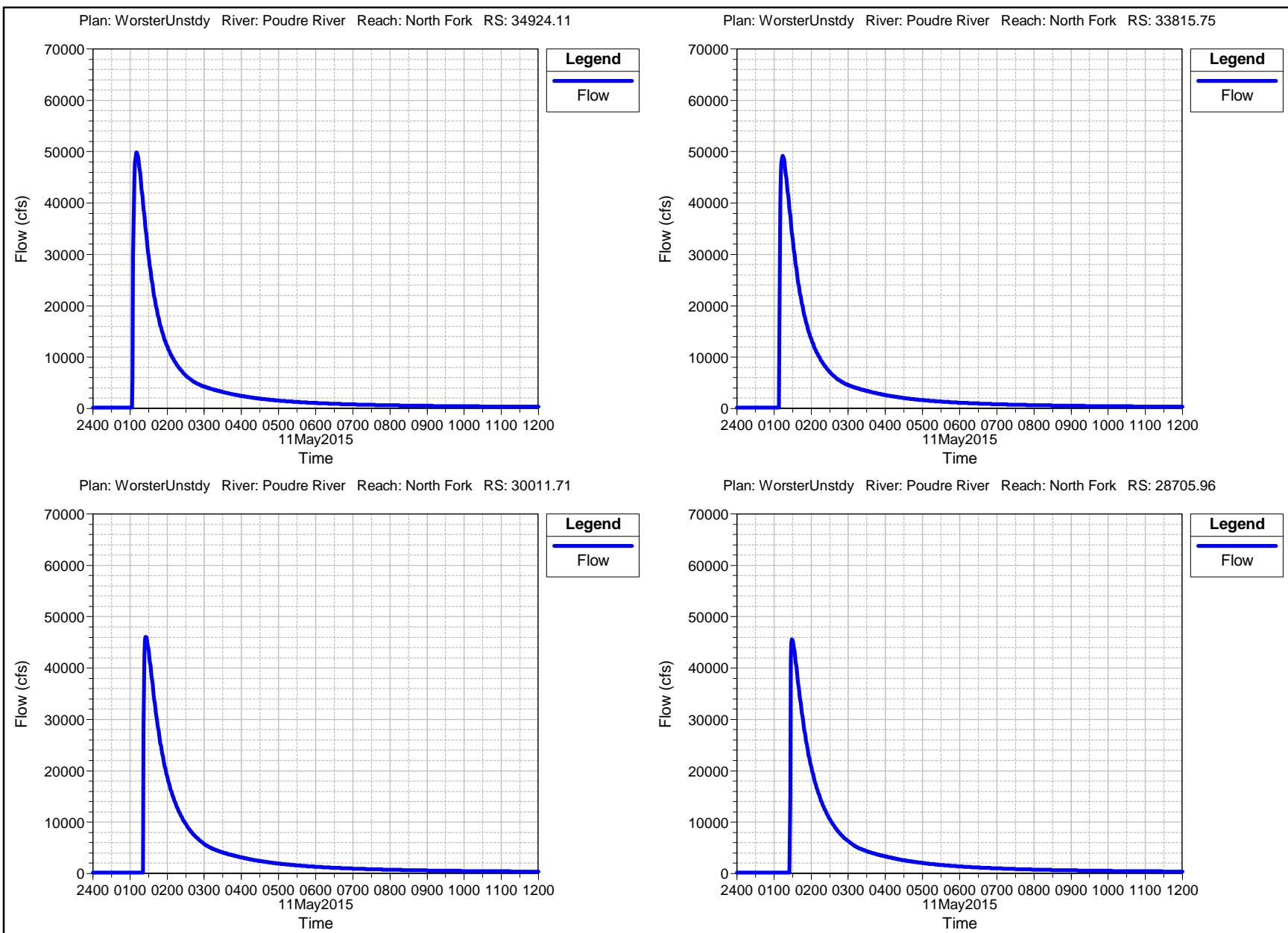
Plan: Worster Dam Breach Unsteady Flow 5/14/2016

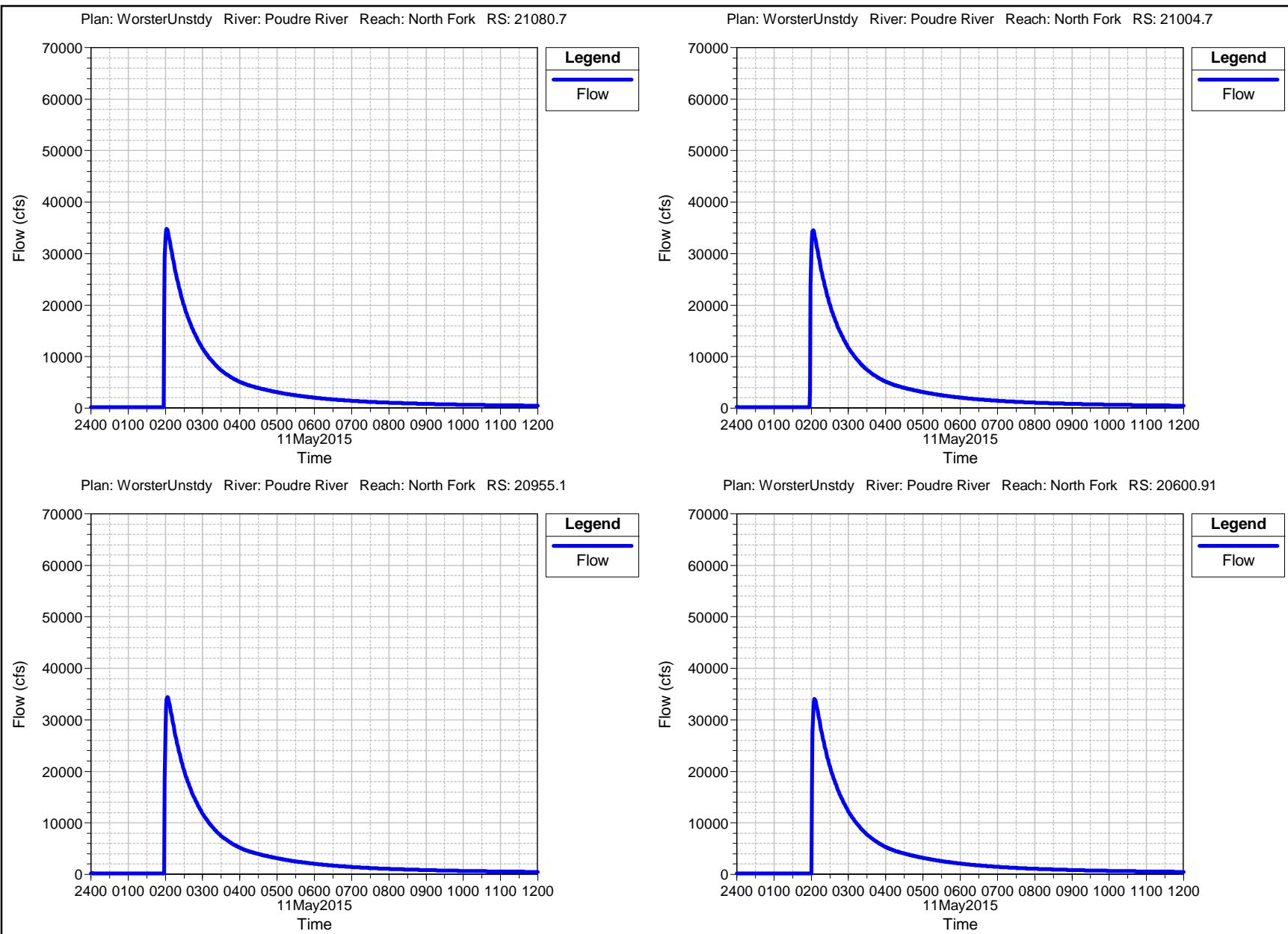
RS = 14994.68

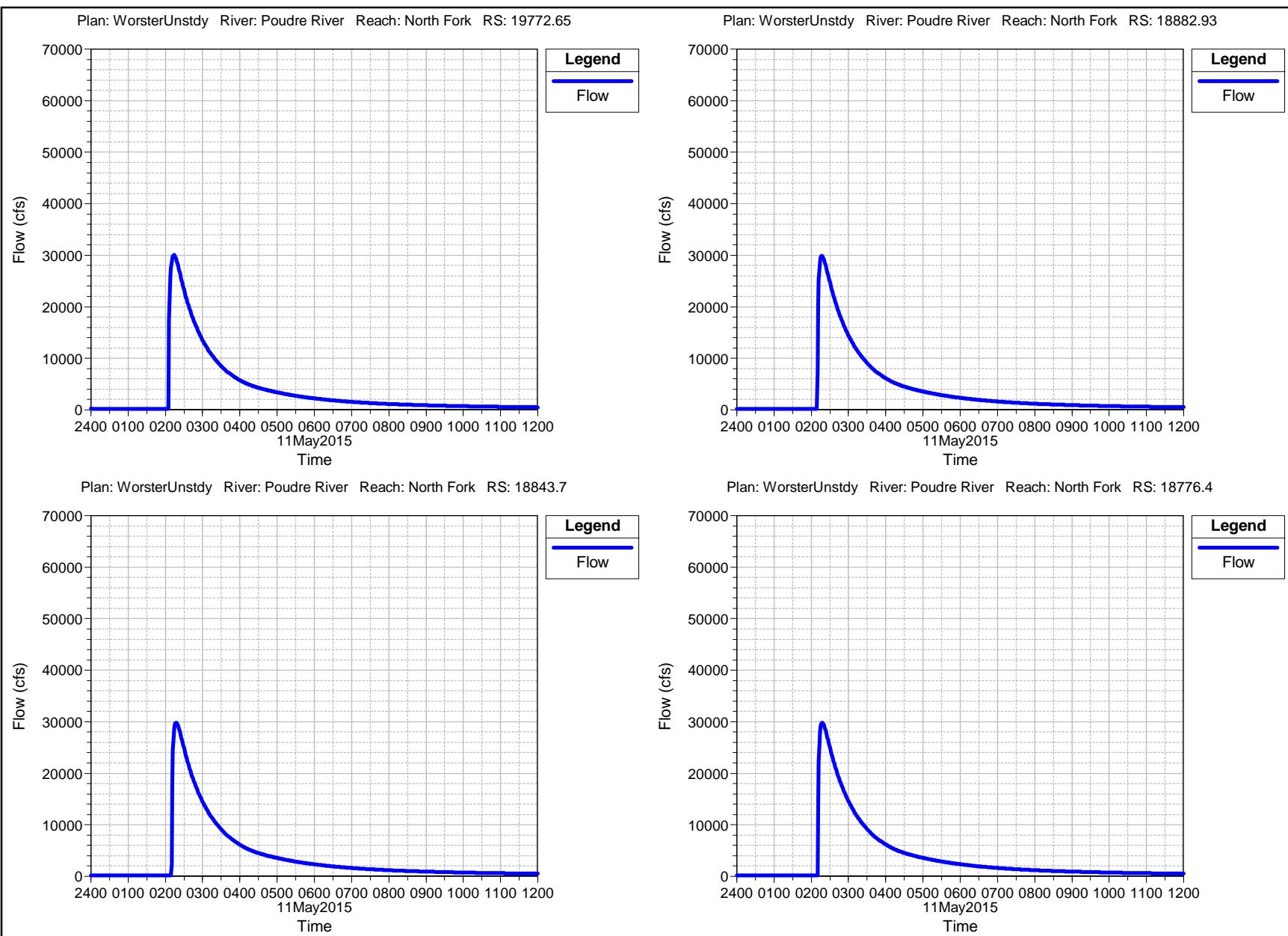


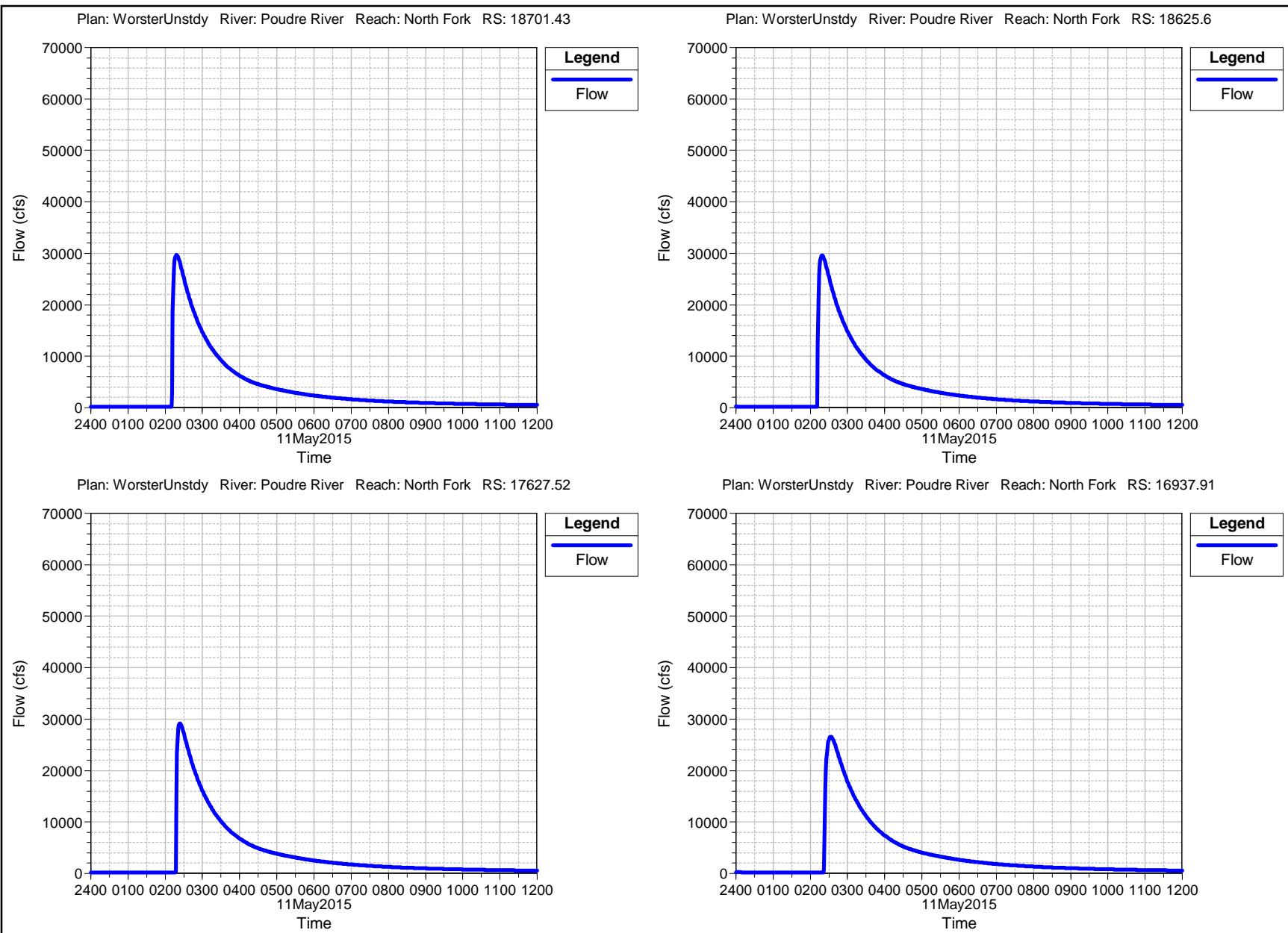


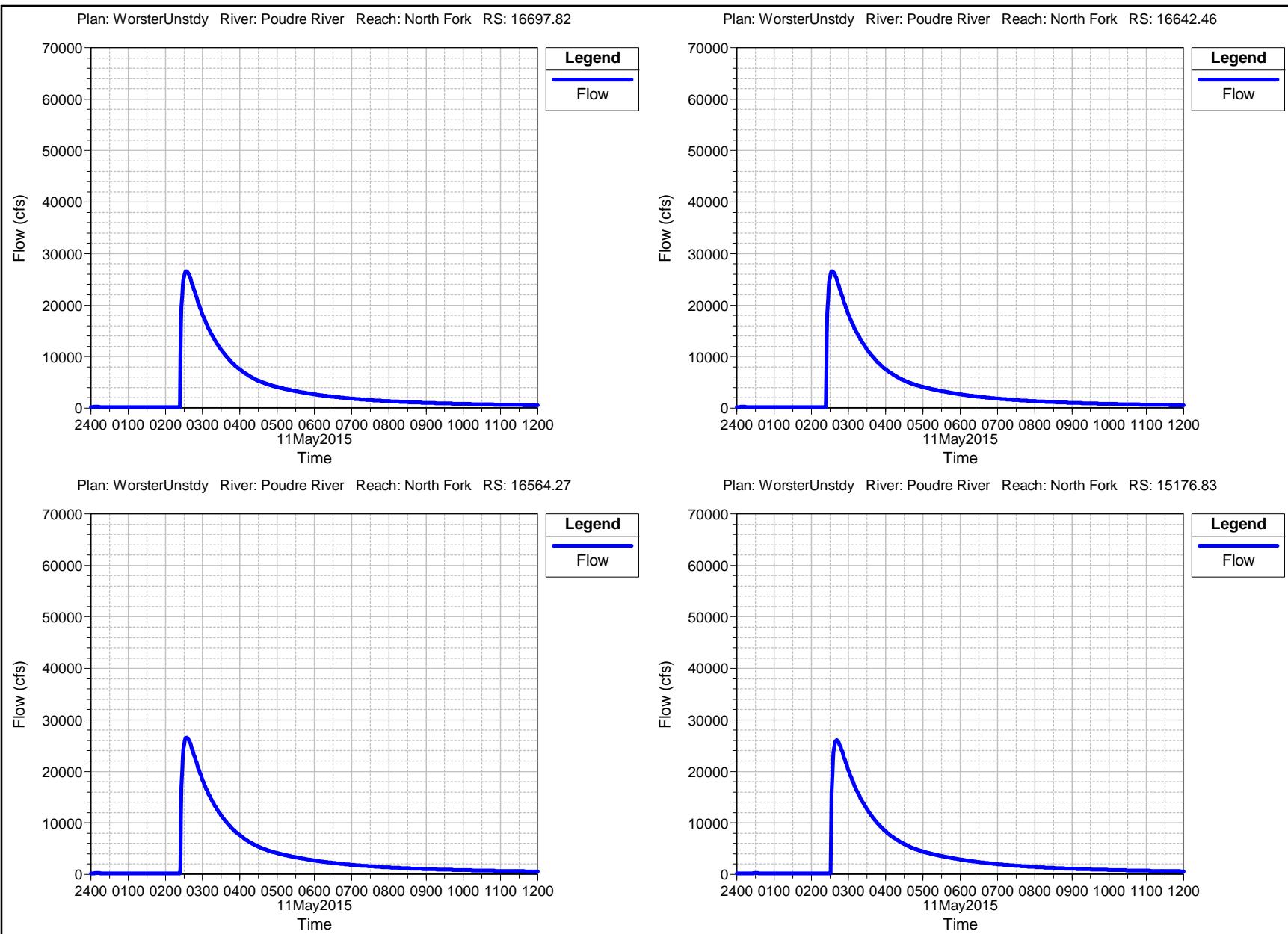


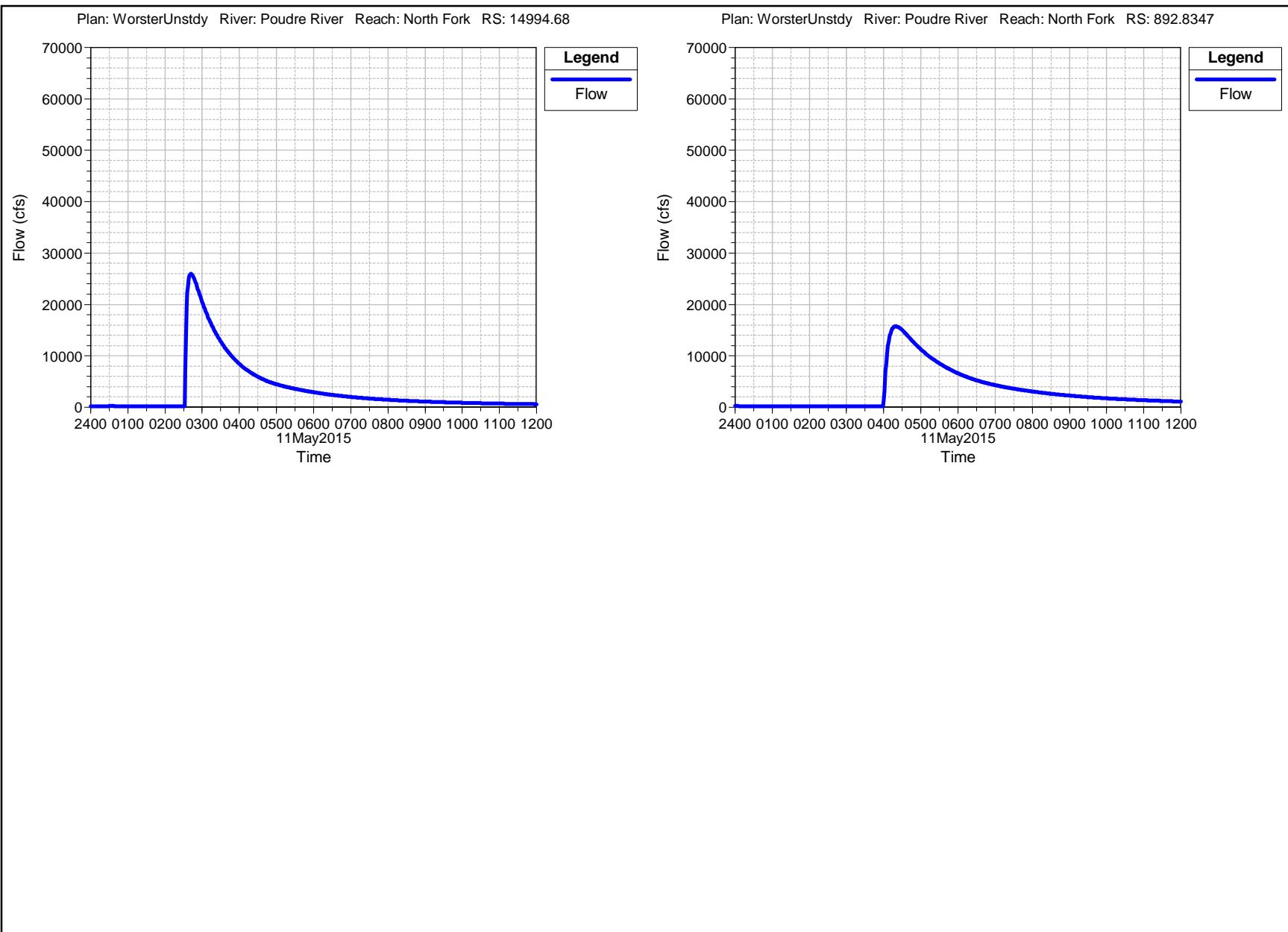












APPENDIX E
SITE PHOTOS



Photo 1 – Structure near Critical Section 16697.8



Photo 2 – Structure at Critical Section 16937.9



Photo 3 – Structures at Critical Section 17627.5



Photo 4 – Structure at Critical Section 17627.5



Photo 5 – North Fork Cache La Poudre River near Critical Section 20600.9



Photo 6 – Structure at Critical Section 20955.1



Photo 7 – Structures at Critical Section 21004.7



Photo 8 – Snow-covered access road to Worster Dam